

)ate:7/9/2018

INDEX OF SHEETS DESCRIPTION TITLE SHEET ESTIMATED QUANTITIES 2-3 4 GENERAL NOTES AND EARTHWORK SUMMARY TYPICAL SECTIONS 5 6-10 MAINTENANCE OF TRAFFIC 11-17 MISCELLANEOUS DETAILS GENERAL PLANS 18-19 20 LIMIT OF DISTURBANCE PROFILE 21-24 25-36 CROSS SECTIONS 37-91 STRUCTURAL PLANS ANDROSCOGGIN RIVER OVERPASS PLANS 92-115

2018.15 CONTRACT



ICE PRESIDENT

7/2/12

ITEM NO.	ITEM DES CRIPTION	REFERENCE QUANTITY	UNIT	COBB. STRUCTURAL QUANTITY	COBB. CIVIL QUANTITY	ANDRO. QUANTITY	TOTAL QUANTITY	ITEM NO.	ITEM DESCRIPTION	REFERENCE QUANTITY	UNIT	COBB. STRUCTURAL QUANTITY	COBB. CIVIL QUANTITY	ANDRO. QUANTITY	TOTAL QUANTITY
201.31	Removal of Debris		LS		_	1	1	507.091	A luminum Bridge Railing, 1 Bar	872 LF	LS	1			1
202.202	Removing Pavement Surface		SY		12550		12,550	508.14	High Performance Waterproofing Membrane	1820 SY	LS	1			1
202.2026	Removing Pavement Surface - Drainage Paths		SF		22		22	511.071	Cofferdam Pier 1 - NB		LS	1			1
202.206	Removing Rumble Strips		LF		100		100	511.072	Cofferdam Pier 1 - SB		LS	1			1
202.10	Removing Existing Superstructure Property of Contractor	420 CY	LS	1			1	511.073	Cofferdam Pier 2 - NB		LS	1			1
202.12	Removing Existing Structural Concrete		CY	140			140	511.074	Cofferdam Pier 2 - SB		LS	1			1
202.13	Removing Existing Railings Retained by Authority		LF	980			980	511.075	Cofferdam Pier 3 - NB		LS	1			1
203.20	Common Excavation		СҮ		931		931	511.076	Cofferdam Pier 3 - SB		LS	1			1
203.25	Granular Borrow		CY		500		500	514.06	Curing Box for Concrete Cylinders		EA	1		1	2
203.43	Geofoam Lightweight Fill		CY	280			280	515.201	Pigmented Protective Coating for Concrete Surfaces		SY	1,400			1,400
203.45	Leveling Sand		CY	69			69	515.202	Clear Protective Coating for Concrete Surfaces		SY	1,050		3,250	4,300
206.082	Structural Earth Excavation - Major Structures, Plan Quantity		CY	420			420	515.23	Anti-Graffiti Coating		SY			900	900
206.10	Structural Earth Excavation - Piers		CY	700			700	518.40	Epoxy Injection Crack Repair		LF	45		310	355
g 304.10	Aggregate Subbase Course - Gravel		CY		250		250	518.401	Epoxy Injection Crack Repair - Below Waterline		LF	15		470	485
304.14	Aggregate Base Course - Type A		CY		230		230	518.51	Repair of Upward Facing Surfaces - Below Reinforcing Steel < 8 inches		SF	24		43	67
403.207	Hot Mix Asphalt, 19.0 mm Nominal Maximum Size		Ton		700		700	518.60	Repair of Vertical Surfaces < 8 inches		SF	43		2,950	2,993
403.208	Hot Mix Asphalt, 12.5 mm Nominal Maximum Size		Ton		130		130	518.601	Repair of Vertical Surfaces < 8 inches - Below Waterline		SF	37		18	55
403.2081	Hot Mix Asphalt, 12.5 mm (Polymer Modified) - RAP		Ton	150	1,150		1,300	518.70	Repair of Overhead Surfaces < 8 inches		SF			91	91
102 200 1	Hot Mix As phalt, 12.5 mm Nominal Maximum Size (sidewalks, drives,		-					520.23	Asphaltic Plug Joint		LF	160			160
403.2084	is lands & incidentals)		Ton		80		80	520.224	Joint Armor Repair		LS	1			1
403.212	Hot Mix Asphalt, 4.75 mm Nominal Maximum Size		Ton		210		210	523.52	Bearing Installation		EA	60			60
	Hot Mix Asphalt, 12.5 mm Nominal Maximum Size (Base and							523.5401	Laminated Elastomeric Bearings, Fixed		EA	12			12
403.213	Intermediate Base Course)		Ton	150	470		620	523.54021	PTFE Elastomeric Bearings, Expansion		EA	48			48
409.15	Bituminous Tack Coat - Applied		Gal	110	1,325		1,435	523.56	Cleaning and Painting Bearing		EA			42	42
419.30	Sawing Bituminous Pavement		LF		1,980		1,980	523.561	Repair Bearing		EA			1	1
470.08	Berm Dropoff Correction - Grindings		Ton		16		16	523.562	Repair Bearing Keeper Strap		EA			10	10
470.081	Berm Correction		LF		1,350		1,350	524.301	Temporary Structural Support - Androscoggin Jacking		LS			1	1
501.231	Dynamic Loading Test		EA	4			4	524.302	Temporary Structural Support - Cobbosseecontee Girders		LS	1			1
501.38	Steel H-beam Piles 42 lb/ft, delivered		LF	510			510	524.303	Temporary Structural Support - Cobbosseecontee Braces		LS	1			1
501.381	Steel H-beam Piles 42 lb/ft, in place		LF	510			510	524.40	Protective Shielding - Steel Girders		SY	2,250			2,250
501.90	Pile Tips		EA	16			16	524.60	Temporary Access Platforms for Pier Inspection and Repair		LS	,		1	1
501.91	Pile Splices		EA	5			5	526.306	Temporary Concrete Barrier, Type I - Supplied by Authority	5210 LF	LS		0.5	0.5	1
501.92	Pile Driving Equipment Mobilization		LS	1			1	527.301	Energy Absorbing System (CAT) - New		EA		3		3
502.219	Structural Concrete, Abutments and Retaining Walls	160 CY	LS	1			1	527.341	Work Zone Crash Cushions - TL-3		Unit		1	2	3
502.239	Structural Concrete Piers	170 CY	LS	1			1	603.159	12 inch Culvert Pipe Option III		LF		148		148
502.24	Structural Concrete Piers (placed under water)		CY	310			310	603.179	18 inch Culvert Pipe Option III		LF		65		65
502.26	Structural Concrete Roadway and Sidewalk Slab on Steel Bridges	460 CY	LS	1			1	603.28	Concrete Collar		EA		3		3
502.264	Structural Concrete Parapets	110 CY	LS	1			1	604.184	Rebuild Catch Basin to Grade - Type II		EA		9		9
502.204	Structural Concrete Approach Slab	24 CY	LS	1			1	604.242	Catch Basin Type F3		EA		1		1
503.14	Epoxy-Coated Reinforcing Steel, Fabricated and Delivered		LB	243,000			243,000	604.40	Secure Catch Basin Grate		EA		2		2
503.15	Epoxy-Coated Reinforcing Steel, Placing		LB	243,000			243,000		31" W-Beam Guardrail - Mid-Way Splice (7' Steel Post, 8" Offset Blocks,						
504.702	Structural steel, fabricated and delivered, welded	80,400 LB	LD	1			1	606.13	Single Faced)		LF		1,038		1,038
504.702	Structural steel, rabilitated and derivered, welded	80,400 LB	LS	1			1	606.1723	Bridge Transition - Type III		EA	4			4
504.885	Post-Tensioning Sleeve Repair	00,700 LD	EA	1		13	1	606.1723	Bridge Transition - Type III Bridge Transition - Type III, Modified		EA	2			2
505.08	Shear Connectors	7,548 EA	LS	1		15	1.5	606.1351	Terminal End - Anchored End - 31" W-Beam Guardrail		EA	2	1		
506.9102	Zine Rich Coating System (Shop Applied)	80,400 LB	LS	1			1	606.279	Terminal End - Anchored End, Thrie Beam		EA		1		1
506.9102	Field Touch-Up of Existing Steel	33,000 LB	LS	1			1	000.279	rommariant and - menored and, inte Deam		EA		1		I
1200.9102	riew rough-op of Existing Steer	55,000 LD		1 1	1	1	1 1	1							

lgn	Scale:	Designed by:		
102_Quantities.c	No. Revision By Date		HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155	THE GOLD STAR MEMORIAL HIGHWAY
		CONSULTANT PROJECT MANAGER: Craig R. Morin, P.E.	FAX (207) 228-0909	
me		By Date By Date		
ena		Designed EDD 07\18 Checked CDH 07\18		
File		Drawn SLS 07\18 In Charge of RAL 07\18		MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

ESTIMATED QUANTITIES

CONTRACT:2018.15

SHEET NUMBER: EQ-01

ITEM NO.	TIEM DESCRIPTION	REFERENCE QUANTITY	UNIT	COBB. STRUCTURAL QUANTITY	COBB. CIVIL QUANTITY	ANDRO. QUANTITY	TOTAL QUANTITY
606.353	Reflectorized Flexible Guardrail Marker		EA		3		3
606.354	Remove and Reset Reflectorized Flexible Guardrail Marker		EA		3		3
606.64	Guardrail Thrie Beam - Double Rail		LF		1,225		1,225
606.65	Guardrail Thrie Beam - Single Rail		LF		115		115
606.791	Guardrail - Flared Terminal - 31" W-Beam Guardrail		EA		1		1
607.09	Woven Wire Fence - Metal Posts		LF		120		120
607.33	Bracing Assembly, Type II - Metal Post		EA		2		2
607.4311	Snow Drift Fence		LF		280		280
609.15	Sloped Curb Type 1		LF	970			970
610.08	Plain Riprap		СҮ	1,100	8		1,108
610.18	Stone Ditch Protection		CY		17		17
613.319	Eros ion Control Blanket		SY		160		160
615.07	Loam		CY		310		310
618.141	Seeding Method Number 3		Unit		25		25
619.1201	Mulch - Plan Quantity		Unit		25		25
619.1202	Temporary Mulch		LS		1		1
620.58	Erosion Control Geotextile		SY		85		85
620.70	HDPE Geomembrane		SY	700			700
627.712	White or Yellow Pavement Marking Line		LF		21,600		21,600
627.73	Temporary 6 Inch Pavement Marking Tape		LF		4,950	9,700	14,650
627.731	Temporary 6 Inch Black Pavement Marking Tape		LF		3,850	2,800	6,650
627.77	Removing Existing Pavement Marking		SF		7,850		7,850
627.94	Pavement Marking Tape		LF		140		140
629.05	Hand Labor, Straight Time		HR		20	20	40
631.10	Air Compressor (including operator)		HR		20	10	30
631.11	Air Tool (including operator)		HR		20	10	30
631.12	All Purpose Excavator (including operator)		HR		20		20
631.172	Truck - Large (including operator)		HR		20	20	40
631.36	Foreman		HR		10	10	20
644.10	Glare Screen - Supplied by Authority		LF		1,100		1,100
652.30	Flashing Arrow		EA		2	2	4
652.312	Type III Barricade		EA		1		1
652.33	Drum		EA		120	86	206
652.34	Cone		EA		120	50	170
652.35	Construction Signs		SF		680	830	1,510
652.361	Maintenance of Traffic Control Devices		LS		0.9	0.1	1
652.38	Flaggers		HR			10	10
652.41	Portable-Changeable Message Sign		EA		2	2	4
652.45	Truck Mounted Attenuator		CD		28	20	48
652.451	Automated Trailer Mounted Speed Limit Sign		CD		28	20	48
652.46	Temporary Portable Rumble Strip		Unit		28	20	48
656.50	Baled Hay, in place		EA		20		20
656.632	30 inch Temporary Silt Fence		LF		960		960
656.64	Boom Supported Floating Silt Fence		LF	116		302	418
659.10	Mobilization		LS	0.6	0.2	0.2	1

COMMON EXCAVATION FOR ESTIMATE

COMMON EXCAVATION (FROM CROSS SE GRUBBING IN FILL FULL DEPTH PAVEMENT REMOVAL AT AB TOTAL COMMON EXCAVATION (for estin

FILL FOR BORROW CALCULATIONS

COMMON FILL (FROM CROSS SECTIONS) GRUBBING IN FILL TOTAL FILL

AVAILABLE COMMON EXCAVATION FOR BO

 (1) TOTAL COMMON EXCAVATION DEDUCTIONS: GRUBBING IN CUT GRUBBING IN FILL PAVEMENT SALVAGE (CUT & FILL)
 (2) TOTAL DEDUCTIONS

TOTAL AVAILABLE COMMON EXCAVAT TOTAL AVAILABLE NON-ROCK EXCA

COMPUTATION FOR COMMON BORROW FO

(3)TOTAL FILL

TOTAL AVAIL. NON-ROCK EXCAV. (4)TOTAL AVAILABLE EXCAVATION BORROW NEEDED = TOTAL FILL MIN

IF NO BORROW IS NEEDED, SURPLUS MATE TOTAL FILL, PLUS TOTAL WASTE MATERIAL COMMON BORROW (for estimate) =

03_Quantities.dgn	Scale:	Revision	By Do	Designed b	y:	HN	ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092	MAINE TURNPIKE	THE GOLD STAR MEMORIAL HIGHWAY
name: 0(CONSULTANT Designed	PROJEC By EDD	T MANAGER: Date 07\18	Craig R. Mori	n, P.E. By CDH	Date 07∖18	TEL (207) 774-5155 FAX (207) 228-0909		
File				Drawn	SLS		In Charge of		07\18		MTA PROJECT MANA	AGER: Kristi Van Ooyen, P.E.

ECTIONS) BUTMENTS timate)	832 43 56	931
	<u>99</u> 43	142_
BORROW CALCULATION	NS	
	184	931
	43	308
TION (1) MINUS (2) AVATION		<u> 623</u> <u> 623</u>
OR ESTIMATE		
		142
623 x 0.90 =	=	<u> </u>
ERIAL = AVAILABLE EXC _ TO BE WASTED =	CAVATION MINUS	419

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

ESTIMATED QUANTITIES

CONTRACT:2018.15

SHEET NUMBER: EQ-02

GENERAL NOTES:

I. ALL DETAILS SHALL BE IN CONFORMANCE WITH MAINE DEPARTMENT OF TRANSPORTATION (MDOT) STANDARD DETAILS HIGHWAYS AND BRIDGES LATEST REVISION AND MOOT BEST MANAGEMENT PRACTICES FOR EROSION AND SEDIMENT CONTROL LATEST REVISION UNLESS OTHERWISE INCLUDED IN THESE PLANS

2. ALL EXISTING DELINEATOR AND MILE MARKER POSTS SHALL BE REMOVED AND RESET UPON COMPLETION OF THE CONTRACT. PA GUARDRAIL DELINEATOR POSTS WILL BE MADE UNDE DELINEATOR POSTS SUPPLIED BY THE CONTRACTOR SHALL BE PAID F UNDER ITEM 606.353.

3. THE EXISTING WIRE FENCE AT THE ABUTMENTS, SHALL BE REMOVED AND DISPOSED. REPLACEMENT WOVEN WIRE FENCE AND POSTS SHALL BE INSTALLED BY THE CONTRACTOR. REMOVAL OF THE EXISTING FENCE AND POSTS SHALL BE INCIDENTAL TO THE SECTION 203. EARTHWORK PAY ITEMS.

4. THE CONTRACTOR SHALL SUBMIT THE PROPOSED STAGING AREA(S) AND FIELD TRAILER LOCATION TO THE RESIDENT FOR APPROVAL PRIOR TO STARTING WORK.

5. GEOTECHNICAL INFORMATION FURNISHED OR REFERRED TO IN THIS PLAN SET IS FOR THE BIDDER'S AND CONTRACTOR'S USE. NO ASSURANCE IS GIVEN THAT THE INFORMATION OR INTERPRETATIONS WILL BE REPRESENTATIVE OF ACTUAL SUBSURFACE CONDITIONS AT THE TIME OF CONSTRUCTION. THE AUTHORITY SHALL NOT BE RESPONSIBLE FOR THE BIDDER'S AND CONTRACTOR'S INTERPRETATIONS OF. OR CONCLUSIONS DRAWN FROM THE GEOTECHNICAL INFORMATION. THE BORING LOGS CONTAINED IN THE PLAN SET PRESENT FACTUAL AND INTERPRETIVE SUBSURFACE INFORMATION COLLECTED AT DISCRETE LOCATIONS. DATA PROVIDED MAY NOT BE REPRESENTATIVE OF THE SUBSURFACE CONDITIONS BETWEEN BORING LOCATIONS.

6. THE CONTRACTOR SHALL TAKE NOTE THAT THE COBBOSSEECONTEE STREAM WATER ELEVATION FLUCTUATE BY SEVERAL FEET DUE TO THE RELEASE OF WATER FROM UPSTREAM DAMS, AS WELL AS THE SENSITIVITY OF THE RIVER ELEVATIONS DUE TO RAINFALL. THE CONTRACTOR SHALL PLAN AND COMPLETE THE WORK IN A MANNER WHICH ACCOUNTS FOR THESE VARIATIONS.HYDROLOGIC DATA IS SHOWN ON SHEFT S-02.

7. CLEARING LIMITS SHOWN ON THE PLANS ARE APPROXIMATE. CLEARING COMPLETED BY OTHERS PRIOR TO THE START OF THIS CONTRACT.

8. EXISTING CROSSOVER AT MM 98.0 SHALL BE OPENED FROM NOV. ITO APRIL 15 AND TEMPORARILY CLOSED ALL OTHER TIMES DURING CONSTRUCTION. EXISTING CROSSOVER AT MM 98.0 SHALL BE PERMANENTLY CLOSED AT PROJECT COMPLETION. PAYMENT SHALL BE MADE UNDER 202.202, 403.2081, 409.15, 527.301, 527.306.606.354 AND 606.3606 ITEMS.

9. THE U.S. ARMY CORPS OF ENGINEERS AND THE STATE OF MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION PERMIT FOR THE COBBOSSEECONTEE STREAM OVERPASS PROJECT ARE INCLUDED AS PART OF THIS CONTRACT (SEE SPECIFICATIONS). THE CONTRACTOR SHALL COMPLY WITH ALL CONDITIONS OF THESE PERMITS.

IO. RIGHT OF WAY AND PROPERTY LINES ARE SHOWN FOR INFORMATIONAL PURPOSES ONLY. ADDITIONAL INFORMATION IS AVAILABLE FROM THE MAINE TURNPIKE AUTHORITY UPON REQUEST.

II. THE EXISTING PAVED MEDIAN SOUTH OF THE COBBOSSEECONTEE STREAM OVERPASS, USED FOR MOT SHALL BE $1/_2$ " MILL AND PAVED TO BE OPENED TO TRAFFIC DURING CONSTRUCTION. PAYMENT SHALL BE MADE UNDER 202.202, 403.2081, AND 409.15.

12. THE EXISTING PAVED MEDIAN, NORTH OF THE COBBOSSEECONTEE STREAM OVERPASS, USED FOR MOT, SHALL BE RECONSTRUCTED AS SHOWN ON SHEET MOT-OI.

EARTHWORK NOTES:

I. THE NORMAL GRUBBING WIDTH IN THE FILLS WHEN SUBGRADE IS LESS THAN 5 FEET ABOVE EXISTING GROUND SHALL BE VARIABLE LEFT OR RIGHT. THE GRUBBING DEPTH HAS BEEN ESTIMATED AS 6 INCHES.

2. WASTE MATERIALS SHALL BE DISPOSED OF OFF THE PROJECT SITE AND IN ACCORDANCE WITH ALL ENVIRONMENTAL REGULATIONS.

3. EXCAVATIONS ACCOMPLISHED AS PART OF THIS PROJECT SHALL BE CONSTRUCTED IN ACCORDANCE WITH OSHA SUBPART P OF 29 CFR PART f92.6.65D-652 (CONSTRUCTION STANDARDS FOR EXCAVATION).

4. REMOVAL OF EXISTING PAVEMENT. WITHIN THE AREAS OF FULL DEPTH PAVEMENT AND FULL DEPTH RECONSTRUCTION, SHALL BE PAID FOR AS COMMON EXCAVATION. EXISTING PAVEMENT THICKNESS HAS BEEN ESTIMATED TO BE 10 INCHES.

5. FILL/BORROW SHALL BE COMPACTED TO 90% OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE MODIFIED PROCTOR, EXCEPT AS AMENDED BY SPECIAL PROVISION 203. GRANULAR BORROW AND AGGREGATE SHALL BE COMPACTED TO 95% OF THEIR MAXIMUM DRY DENSITY AS DETERMINED BY THE MODIFIED PROCTOR, EXCEPT AS AMENDED BY SPECIAL PROVISION 203.

USIS SHALL BE REMOVED	
PAYMENT FOR RESETTING	3. ALL DITCH ELEVATIONS AND OFFSETS SHOWN ON THE CROSS SECTIONS ARE FOR THE FINISHED DITCH FLOW LINE.
ER ITEM 606.354.	
SHALL BE PAID FOR	4. ANY NECESSARY CORING OF EXISTING CATCH BASINS TO TAKE A PROPOSED PIPE WILL NOT BE PAID FOR

SEPARATELY AND SHALL BE INCIDENTAL TO THE PROPOSED CULVERT ITEMS. UTILITY NOTES:

I. EXISTING UTILITIES ON THESE PLANS WERE COMPILED FROM FIELD SURVEY AND VARIOUS OTHER SOURCES. LOCATIONS ARE NOT GUARANTEED TO BE ACCURATE NOR IS IT GUARANTEED THAT ALL UTILITIES ARE SHOWN. NO SEPARATE OR ADDITIONAL COMPENSATION WILL BE ALLOWED TO THE CONTRACTOR DUE TO ANY VARIANCE BETWEEN THE DATA SHOWN ON THE PLANS AND THE ACTUAL FIELD CONDITIONS ENCOUNTERED. NO WORK SHALL BE STARTED UNTIL THE OWNERS OF THE VARIOUS UTILITIES ARE NOTIFIED BY THE CONTRACTOR OF THE PROPOSED CONSTRUCTION. THE CONTRACTOR IS REQUIRED TO CALL DIG SAFE AT I-888-344-7233 PRIOR TO THE START OF THE WORK. THE CONTRACTOR SHALL NOTIFY THE RESIDENT 10 DAYS PRIOR TO CONSTRUCTION SO THE RESIDENT MAY COORDINATE WITH DIG SMART.

I. NO EXISTING DRAINAGE SHALL BE ABANDONED, REMOVED OR PLUGGED WITHOUT PRIOR APPROVAL OF THE RESIDENT. ABANDONED STRUCTURES TO REMAIN SHALL BE PLUGGED WITH BRICK AND MORTAR. THIS WORK SHALL BE INCIDENTAL

2. INLETS AND OUTLETS OF ALL CULVERTS SHALL BE RIPRAPPED UNLESS OTHERWISE NOTED ON THE PLANS OR

2. SEE SPECIFICATIONS FOR REQUIRED UTILITY COORDINATION.

GUARDRAIL NOTES:

DRAINAGE NOTES:

TO SECTION 604 ITEMS.

DIRECTED BY THE RESIDENT.

I. AT THE END OF EACH DAY, THE CONTRACTOR IS REQUIRED TO HAVE AN APPROVED CRASHWORTHY END TREATMENT ON ALL GUARDRAIL WITHIN ALL WORK AREAS THAT ARE ACCESSIBLE TO TRAFFIC.

2. CONNECTIONS FOR PROPOSED GUARDRAIL TO EXISTING GUARDRAIL SHALL BE INCIDENTAL TO THE PROPOSED GUARDRAIL ITEMS, UNLESS NOTED OTHERWISE.

3. ALL GUARDRAIL SHALL BE INSTALLED IN A MANNER TO AVOID DRAINAGE STRUCTURES.

4. SEE DOWNSPOUT DETAILS, SHEET MD-OI FOR THE SHOULDER PAVEMENT LIMITS FROM THE BRIDGE JOINT TO THE STONE DOWNSPOUT.

5. GUARDRAIL REMOVED AND NOT RESET OR STACKED SHALL BE INCIDENTAL TO CONTRACT ITEMS AND INCLUDE ALL REMOVAL, DISPOSAL, EQUIPTMENT, AND LABOR NECESARY TO SATISFACTORILY COMPLETE THE WORK.

6. HOLES CREATED BY GUARDRAIL REMOVAL WILL BE FILLED AND COMPACTED WITH APPROVED MATERIALS AS DIRECTED BY THE RESIDENT. PAYMENT TO BE CONSIDERED INCIDENTAL TO THE GUARDRAIL ITEMS.

SIGN NOTES:

I. BRIDGE NO. SIGNS SHALL BE SUPPLIED BY THE MAINE TURNPIKE AUTHORITY. THE CONTRACTOR SHALL SUPPLY THE U-CHANNEL POST AND SHALL INSTALL THE SIGNS. PAYMENT SHALL BE UNDER ITEM 645.272.

2. EXISTING E-ZPASS GUIDE SIGN MOUNTED ON LUNTS HILL ROAD UNDERPASS BRIDGE SHALL BE FULLY COVERED FOR THE DURATION OF THE PROJECT AND UNCOVERED AT PROJECT COMPLETION, CARE SHALL BE TAKEN TO AVOID DAMAGING THE SIGN WHILE COVERING, UNCOVERING, AND THROUGHOUT CONSTRUCTION. THIS WORK SHALL BE CONSIDERED INCIDENTAL TO THE CONSTRUCTION SIGNS ITEM.

3. EXISTING PAY TOLL I MILE GUIDE SIGN MOUNTED ON LUNTS HILL ROAD UNDERPASS BRIDGE SHALL HAVE THE ARROW COVERED BY RIVETED TEMPORARY PANELS FOR THE DURATION OF THE PROJECT AND UNCOVERED AT PROJECT COMPLETION. CARE SHALL BE TAKEN TO AVOID DAMAGING THE SIGN WHILE COVERING, UNCOVERING, AND THROUGHOUT CONSTRUCTION THIS WORK SHALL BE CONSIDERED INCIDENTAL TO THE CONSTRUCTION SIGNS ITEM.

LIST OF ABBREVIATIONS

ABUT ABUTMENT ADDL ADDITIONAL ALT ALTERNATE APPROX APPROXIMATELY BOT BOTTOM BRG BEARING CL CLEAR Q - CENTERLINE CONSTR CONSTRUCTION C.Y CUBIC YARD DEMO DEMOLITION DIA DIAMETER EA EACH EB - EASTBOUND E.F EACH FACE EL ELEVATION	EQ EQUAL EXIST EXISTING EXP EXPANSION F.F FAR FACE JT JOINT MAX MAXIMUM MEDOT - MAINE DEPARTMENT OF TRANSPORTATION MIN MINIMUM MTA - MAINE TURNPIKE AUTHORITY NB - NORTHBOUND N.F NEAR FACE N.T.S NOT TO SCALE PED PEDESTAL PGL - PROFILE GRADE LINE & -PLATE PROP PROPOSED	P.S.I POUNDS per SQUARE INC RDWY ROADWAY SHLDR SHOULDER SB - SOUTHBOUND SF - SQUARE FEET SP SPACES STA STATION T.&B TOP & BOTT TPKE TURNPIKE TYP TYPICAL U.O.N UNLESS OTHERWISE VERT VERTICAL WB - WESTBOUND W.P WORKING POIN WW - WINGWALL
	THOP: THOP USED	

INCH /AY LDER DUND FEET BOTTOM IKE

NISE NOTED CAL JND POINT

otes.	Scal	e:			Designed by:					
General Notes								TD		
Gen	No.	Revision	Bv	Date			HN			
- 11	INO.	Revision	Бу	Date						
004.								0 · D H · I		
					CONSULTANT F	ROJE	CT MANAGER:	Craig R. Morin, I	² .E.	
m						By	Date		By	Date
Filename					Designed	LZD	07\18	Checked	CDH	07\18
File					Drawn	SLS	07\18	In Charge of	RAL	07\18

HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155 FAX (207) 228-0909



EROSION CONTROL NOTES:

I. THE ANTICIPATED EROSION CONTROL DEVICES ARE SHOWN ON THE PLANS. THE CONTRACTOR SHALL PROPOSE ACTUAL TYPE AND LOCATION OF DEVICES FOR APPROVAL BY THE RESIDENT. ADDITIONAL MEASURES MAY BE PROPOSED BY THE CONTRACTOR DUE TO SITE OR WEATHER CONDITIONS. THE RESIDENT MAY DIRECT THE CONTRACTOR TO IMPLEMENT ADDITIONAL MEASURES. ANY ADDITIONAL MEASURES APPROVED BY THE RESIDENT WILL BE MEASURED FOR PAYMENT UNDER THE APPROPRIATE BID ITEMS.

2.4" LOAM HAS BEEN ESTIMATED FOR 100% OF THE DISTURBED SLOPE AREA UNLESS OTHERWISE SPECIFIED ON THE PLANS. ACTUAL PLACEMENT OF THE LOAM SHALL BE AS DESIGNATED BY THE RESIDENT.

3. ALL SLOPES SHALL BE SEEDED WITH SEEDING METHOD NO. 1, 2, OR 3. UNLESS OTHERWISE NOTED, SEEDING METHOD NO. I SHALL BE UTILIZED ON ALL LAWNS AND DEVELOPED AREAS, SEEDING METHOD NO. 2 SHALL BE UTILIZED ON ALL NON-GUARDRAIL FORESLOPES. FROM THE EDGE OF FORESLOPE TO THE DITCH LINE OR TOE OF FILL: SEEDING METHOD NO. 3 SHALL BE UTILIZED ON ALL BACKSLOPES AND ON ALL GUARDRAIL FORESLOPES.

4. MULCH SHALL BE APPLIED IN AREAS SEEDED BY SEEDING METHODS NO. I, 2, AND 3 EXCEPT WHERE EROSION CONTROL BLANKET IS SPECIFIED.

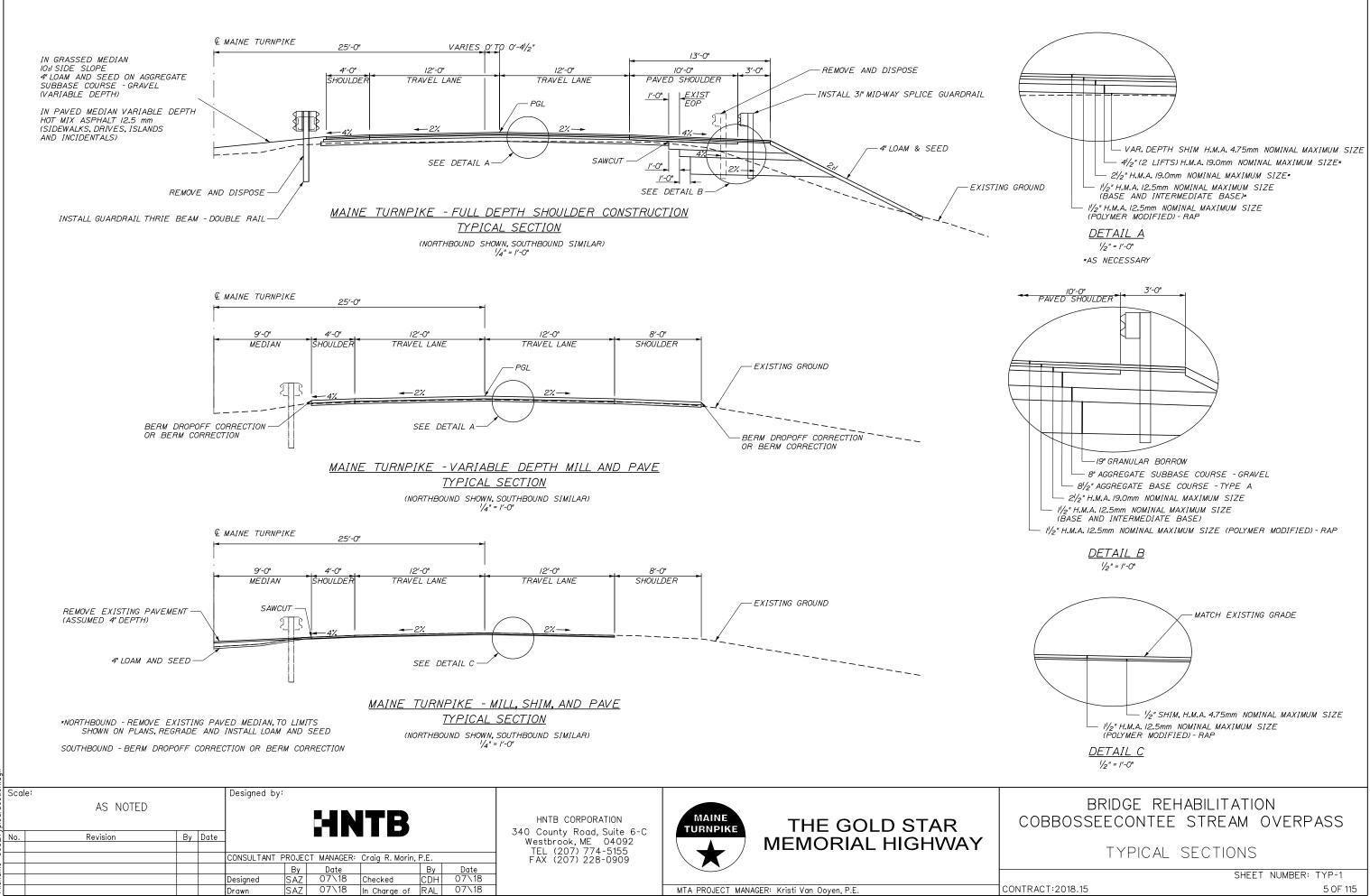
6. TEMPORARY BERMS AND TEMPORARY SLOPE DRAINS ARE ANTICIPATED AT ALL STONE DOWNSPOUT LOCATIONS WHILE GROWTH IS BEING ESTABLISHED ON SIDE SLOPES.

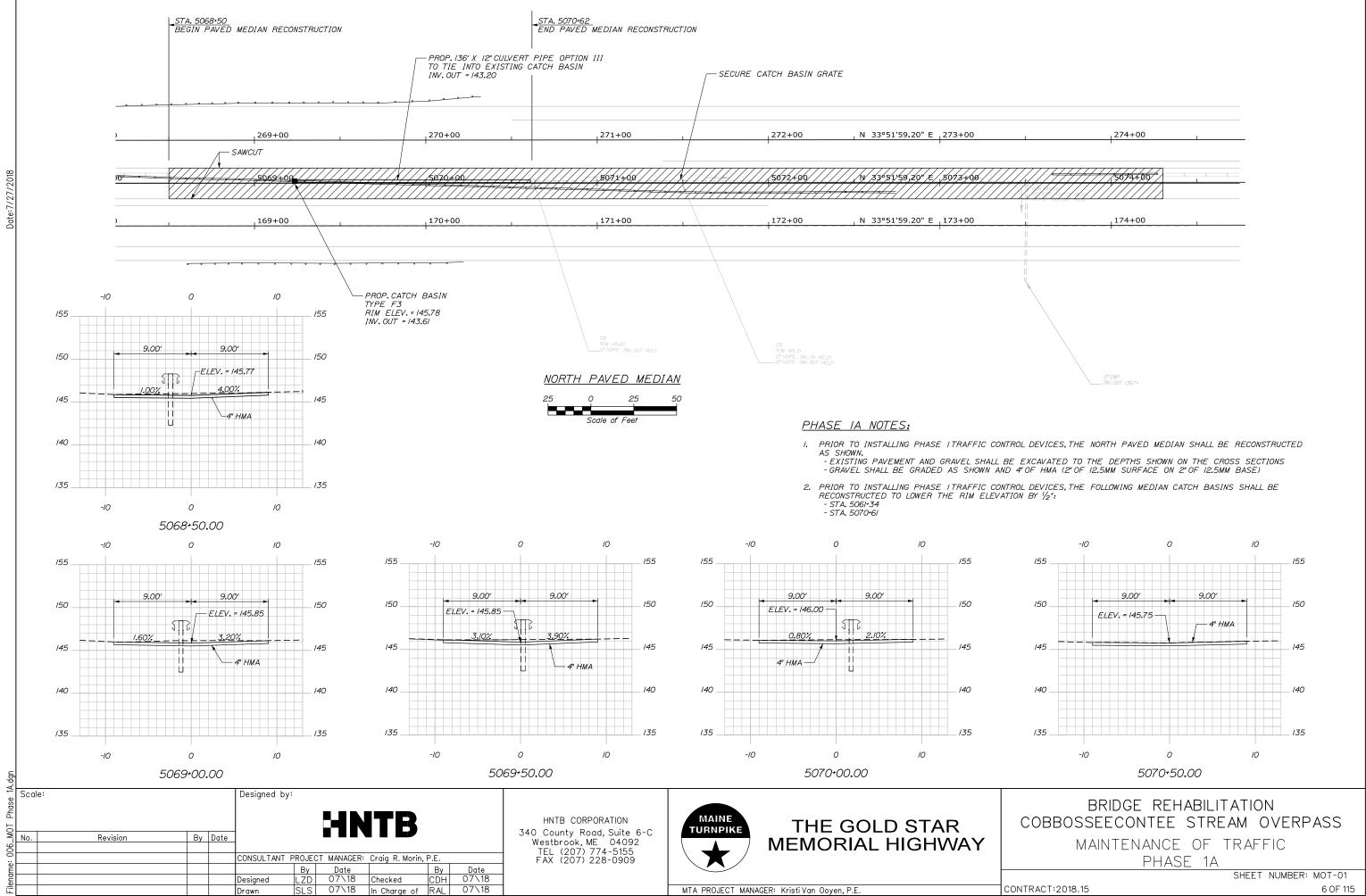
7. TEMPORARY EROSION CONTROL BLANKET. ITEM 613.319 SHALL BE INSTALLED IN ALL DITCHES AND 2: SLOPES FROM TOP TO TOE OF SLOPE. LOAM AND SEED SHALL BE PLACED PRIOR TO THE INSTALLATION OF THE EROSION CONTROL BLANKET. LIMITS OF THE EROSION CONTROL BLANKET IN DITCHES SHALL BE 6'WIDE OR AS DESIGNATED BY THE RESIDENT.

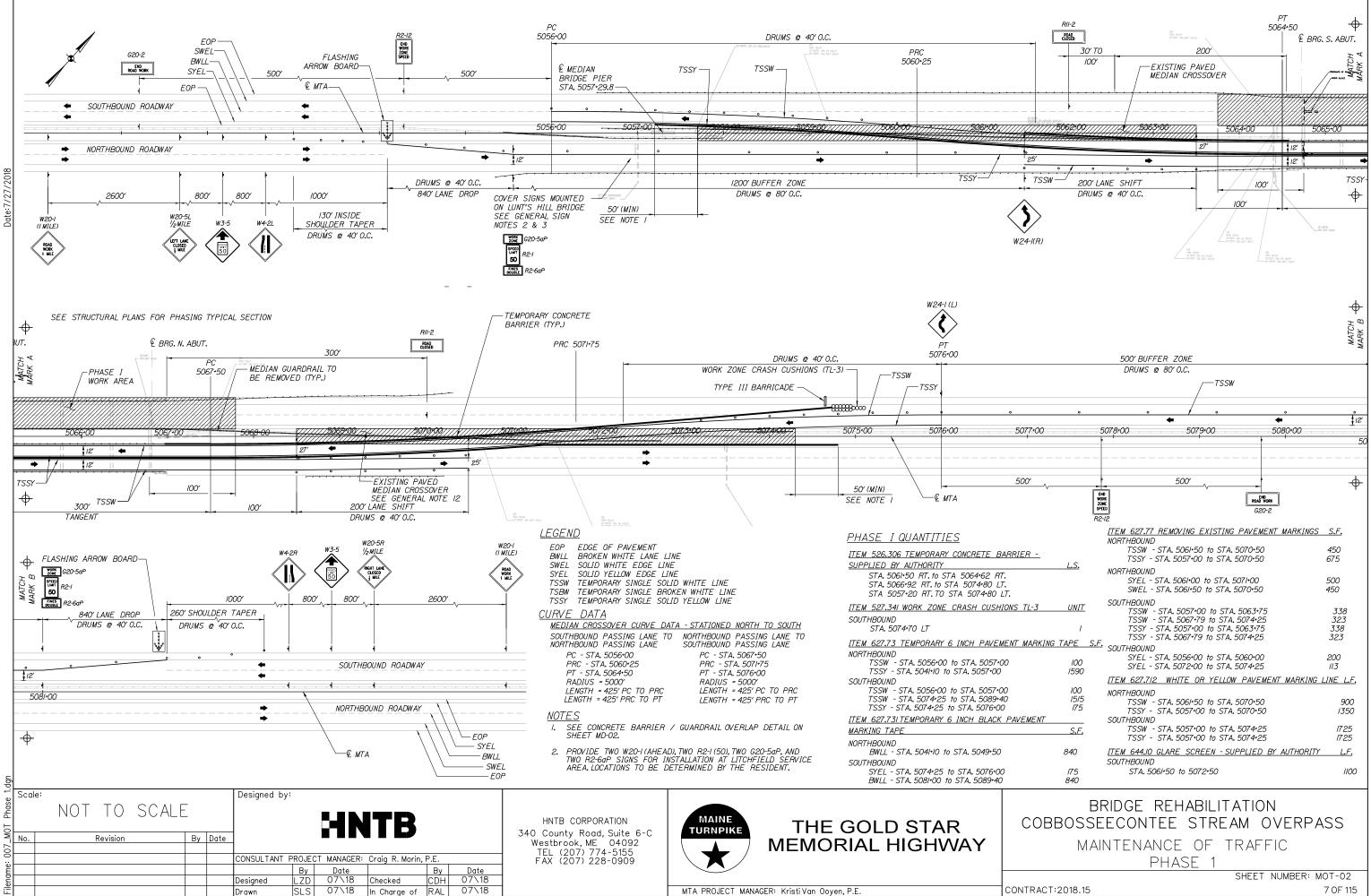
8. PLACE A FOUR FOOT WIDE STRIP OF TEMPORARY EROSION CONTROL BLANKET ON THE SIDE SLOPES ALONG THE TOP OF THE RIPRAP AND BEHIND THE WINGWALLS.

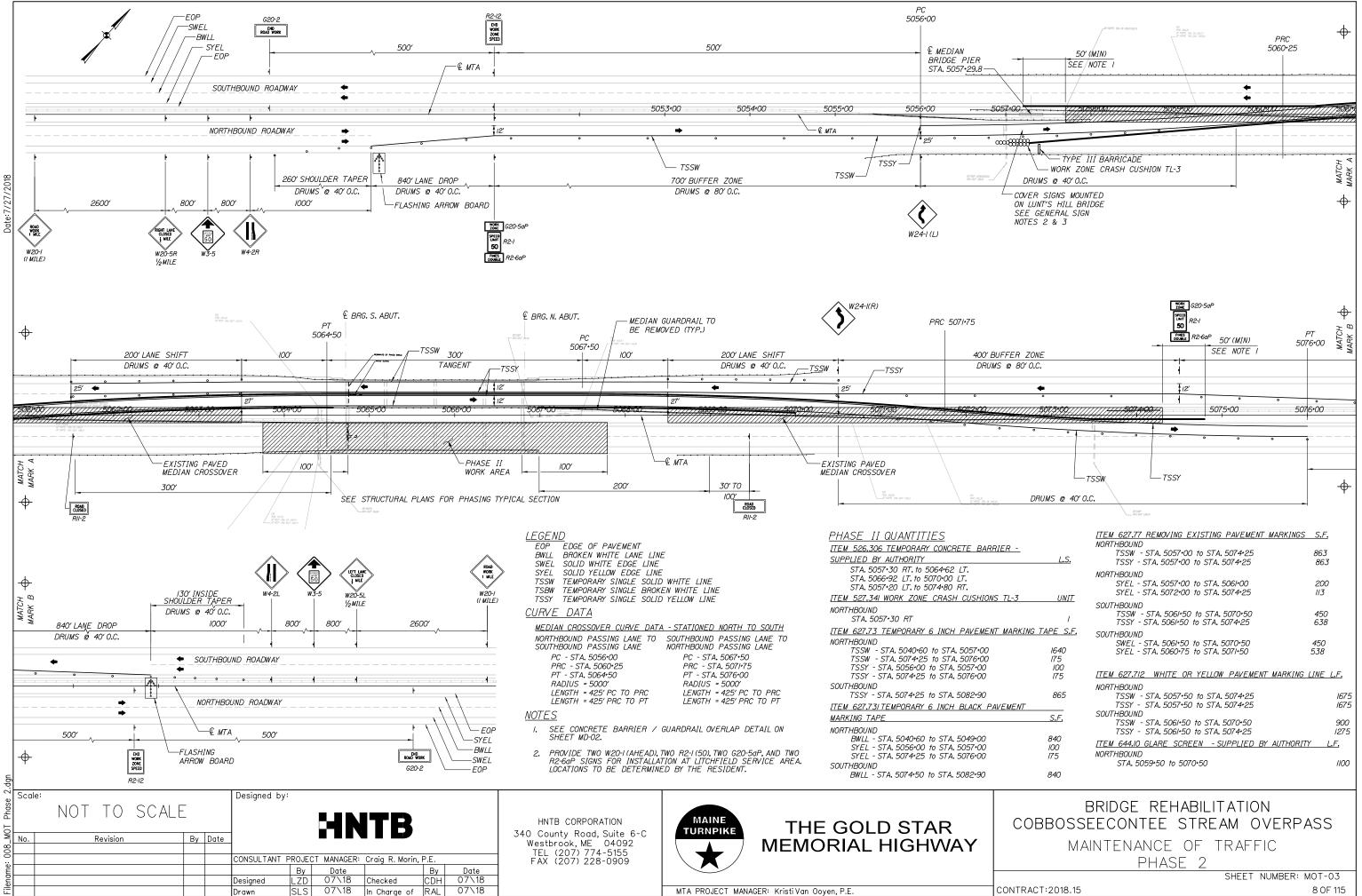
5. ALL TEMPORARY AND PERMANENT EROSION CONTROL DEVICES SHALL BE INSTALLED IN ACCORDANCE WITH THE MAINE DEPARTMENT OF TRANSPORTATION BEST MANAGEMENT PRACTICES.

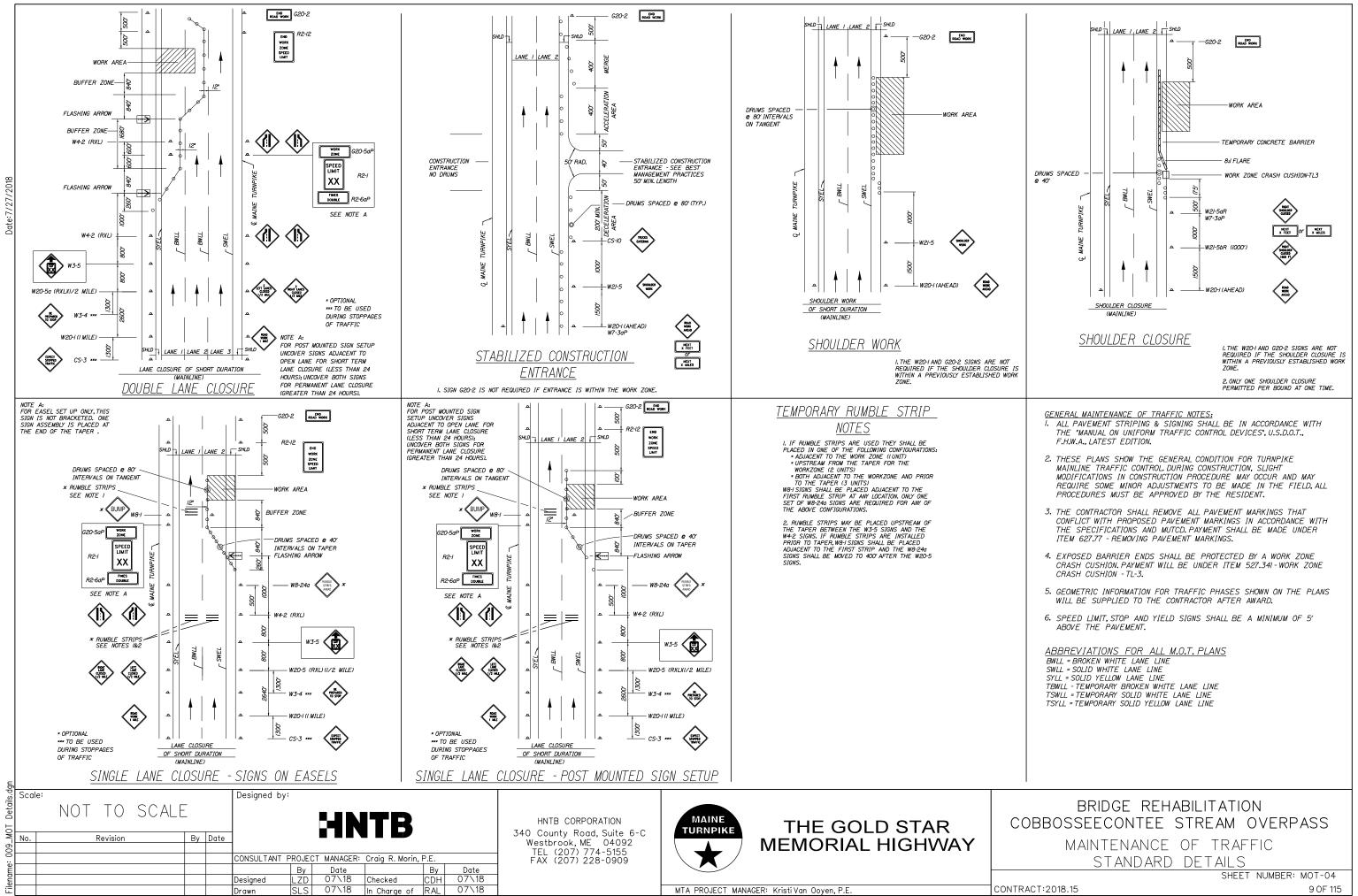
> BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS GENERAL NOTES AND EARTHWORK SUMMARY SHEET NUMBER: GN-01











/2018	
:7/27	
Date	

HEIGHT 24" 24"	TEXT END ROAD WORK ZONE SPEED LIMIT	CONI	GHT EXT L FORM WDAR	DIMENS TO "2	SING SIONS 2004 E 2004 E	ARRON RTE. MP SHALL DITION SIGNS ENT"	K R. -	OF SIGNS REQUIRED 4	STAN	ID COL DRM T DARD	LEGEND BORDER ORS SH 0 "2004 HIGHWA SUPPLE	ALL EDITION Y SIGNS	- 8.00
24"	WORK ZONE SPEED	CONI	FORM NDAR	TO "2 D HIG	2004 E 2HW AY	DITION SIGNS	-	4	STAN	DRM T DARD	0 "2004" HIGHWA	EDITION Y SIGNS	
	SPEED										1		- 8.00
60*								6					
	50							6					
24"	FINES DOUBLED							6					
60"	END WORK ZONE SPEED							4					
30"	ROAD CLOSED							2					
48"	SO							4					
48"								2					
<i>48</i> "								2					/6.00 (32)
48"	ROAD WORK 1 MILE				,			6					/6.00 (96)
	30" 48" 48" 48"	60" WORK ZONE SPEED 30" ROAD CLOSED 48" SO 50 48" 48" 48"	60" VORK ZONE 30" ROAD CLOSED 45" SO 45" SO	60" WORK ZONE 30" CLOSED 45" SPEED 45" SPEED	60" WORK ZONE 30" ROAD CLOSED 45" SEED SO 45" SEED SO	60" WORK ZONE 30" ROAD CLOSED 45" SPEED SO 45" SPEED SO	60" WORK ZONE 30" ROAD CLOSED 45" SPEED SO 45" SPEED SO	60" YORK 30" ROAD 30" ROAD 45" SPEED 45" SPEED	60" YORK 201 30" ROAD 2 45" SEED 4 45" SEED 4 45" SEED 4 46" SEED 2 45" SEED 2	60" YONK 30" ROAD CLOSED 2 48" 4 48" 4 48" 2 48" 2 48" 2 48" 2 48" 2 48" 2 46" 2 46" 2	60° 2000 30° $ROAD$ $CLOSED$ 2 48° 50° 48° 2	60° WORK ZONE SEED 4 30° \boxed{CLOSED} 2 $$ 48° \boxed{CLOSED} 4 $$ 48° $\boxed{50}$ 4 $$ 48° $\boxed{50}$ 2 $$ 48° $\boxed{100}$ 2 $$ 48° $\boxed{100}$ 2 $$ 48° $$ 2 $$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $

IDENTIFI- CATION	SIZE O	F SIGN	TEXT	TE	XT DI	MENS.	IONS	INCHL	ES)	NUMBER OF	COLO	DR		BOR.	DER	AREA IN SQUARE
NUMBER	WIDTH	HEIGHT	IEXI	LET HEI	TER GHT		TICAL CING	ARI RTE.	ROW MKR.	SIGNS REQUIRED	CK- UND	LEG BOR		RAD	DIUS	FEET
W20-5L (1/2 MILE)	48"	48 "	LEFT LANE CLOSED 1/2 MILE	CON	IFORM ANDAF	DIMEN TO "2 RD HIC 2 SUF	2004 i GHWAY	EDITIO SIGN	DN -	2	FORM		2004 E SHWAY	EDITIC SIGN		16.00 (32)
W20-5R (1/2 MILE)	<i>48</i> "	<i>48</i> "	RIGHT LANE CLOSED 1/2 MILE							2						16.00 (32)
W24-IL	48™	<i>48</i> "	< <u>></u>							1						16.00 (16)
W24-IR	<i>48</i> *	<i>48</i> "	\$							1						16.00 (16)

dgn					-									
<u> </u>	Scale:				Designed by:									
Summo		NOT TO	SCALE					ITB			HNTB CORPORATION	MAINE	THE GOLD STAR	
0_Sign	No.	Revision	Ву	Date	-						340 County Road, Suite 6-C Westbrook, ME 04092		MEMORIAL HIGHWAY	
010					CONSULTANT F	ROJECT	MANAGER:	Craig R. Morin	, P.E.		TEL (207) 774-5155 FAX (207) 228-0909			
ΞĽ						By	Date		By	Date				
Bue					Designed	EDD	07\18	Checked	CDH	07\18				
Ē[Drawn	EDD	07\18	In Charge of	RAL	07\18		MTA PROJECT MANA	AGER: Kristi Van Ooyen, P.E.	ЭС

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS MAINTENANCE OF TRAFFIC SIGN SUMMARY

CONTRACT:2018.15

SHEET NUMBER: MOT-05

DIMENSIONS FOR SLOPE OF 2:/

ate:7/27/2018

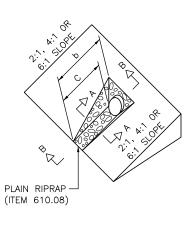
							STONE	STONE
D	a	b	с	e	f	g	DEPTH	(CY)
	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	
12"	1.00	4.00	3.00	2.00	6.00	1.00	1.50	1.30
15"	1.00	4.50	3.37	2.25	6.75	1.63	1.50	1.70
18"	1.00	5.00	3.75	2.50	7.50	2.25	1.50	2.09
21"	1.00	5.50	4.13	2.75	8.25	2.88	1.50	2.58
24"	1.00	6.00	4.50	3.00	9.00	3.50	1.50	3.12
30"	1.00	7.00	5.25	3.50	10.50	4.75	1.50	4.33
36"	1.00	8.00	6.00	4.00	12.00	6.00	1.50	5.75
42"	1.00	9.00	6.75	4.50	13.50	7.25	1.50	7.37
48"	1.00	10.00	7.50	5.00	15.00	8.50	1.50	9.18
54"	1.00	11.00	8.25	5.50	16.50	9.75	1.50	11.19
60"	1.00	12.00	9.00	6.00	18.00	11.00	1.50	13.40
66"	1.00	13.00	9.75	6.50	19.50	12.25	1.50	15.81
72"	1.00	14.00	10.50	7.00	21.00	13.50	1.50	18.41
84"	1.00	16.00	12.00	8.00	24.00	16.00	1.50	24.22

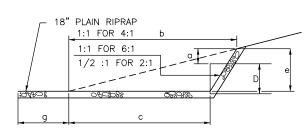
DIMENSIONS FOR SLOPE OF 4:1

							STONE	STONE
D	a	Ь	с	е	f	g	DEPTH	(CY)
	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	
12"	1.00	8.00	6.00	2.00	6.00	0.00	1.50	2.20
15"	1.00	9.00	6.75	2.25	6.75	0.00	1.50	2.80
18"	1.00	10.00	7.50	2.50	7.50	0.00	1.50	3.40
21"	1.00	11.00	8.25	2.75	8.25	0.00	1.50	4.10
24"	1.00	12.00	9.00	3.00	9.00	0.00	1.50	4.86
30"	1.00	14.00	10.50	3.50	10.50	0.00	1.50	6.58
36"	1.00	16.00	12.00	4.00	12.00	0.00	1.50	8.56
42"	1.00	18.00	13.50	4.50	13.50	0.50	1.50	10.92
48"	1.00	20.00	15.00	5.00	15.00	1.00	1.50	13.57
54"	1.00	22.00	16.50	5.50	16.50	1.50	1.50	16.50
60"	1.00	24.00	18.00	6.00	18.00	2.00	1.50	19.72
66"	1.00	26.00	19.50	6.50	19.50	2.50	1.50	23.22
72"	1.00	28.00	21.00	7.00	21.00	3.00	1.50	27.01
84"	1.00	32.00	24.00	8.00	24.00	4.00	1.50	35.45

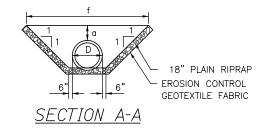
DIMENSIONS FOR SLOPE OF 6:/

							STONE	STONE
D	a	Ь	с	е	f	g	DEPTH	(CY)
	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	
12"	0.50	9.00	7.50	1.50	4.50	0.00	1.50	2.30
15"	0.50	10.50	8.75	1.75	5.50	0.00	1.50	2.93
18"	0.50	12.00	10.00	2.00	6.50	0.00	1.50	3.57
21"	0.50	13.50	11.25	2.25	7.25	0.00	1.50	4.46
24"	0.50	15.00	12.50	2.50	8.00	0.00	1.50	5.44
30"	0.50	18.00	15.00	3.00	9.50	0.00	1.50	7.71
36"	0.50	21.00	17.50	3.50	11.00	0.00	1.50	10.37
42"	0.50	24.00	20.00	4.00	12.50	0.00	1.50	13.42
48"	0.50	27.00	22.50	4.50	14.00	0.00	1.50	16.87
54"	0.50	30.00	25.00	5.00	15.50	0.00	1.50	20.70
60"	0.50	33.00	27.50	5.50	17.00	0.00	1.50	24.93
66"	0.50	36.00	30.00	6.00	18.50	0.00	1.50	29.55
72"	0.50	39.00	32.50	6.50	20.00	0.00	1.50	34.56
84"	0.50	45.00	37.50	7.50	23.00	0.00	1.50	45.76





SECTION B-B



<u>ROADWAY</u>	CULVERT	END
<u>SLOPE</u>	TREATMEN	VT

NOTES:

- 1. THE DIMENSIONS SHOWN ARE APPROXIMATE AND MAY BE MODIFIED BY THE RESIDENT.
- 2. STONE QUANTITIES ARE FOR ONE END OF THE PIPE.

					_						
Sca	le:				Designed by:	:					
	NOT TO	SCAL						ITB			HNTB CORPORATION
No.	Revision		By	Date							340 County Road, Suite 6-C Westbrook, ME 04092
											TEL (207) 774-5155
					CONSULTANT F	PROJE	CT MANAGER:	Craig R. Morin,	. P.E.		FÁX (207) 228-0909
						By	Date		By	Date	
					Designed	EDD	07\18	Checked	CDH	07\18	
					Drawn	SLS	07\18	In Charge of	RAL	07\18	



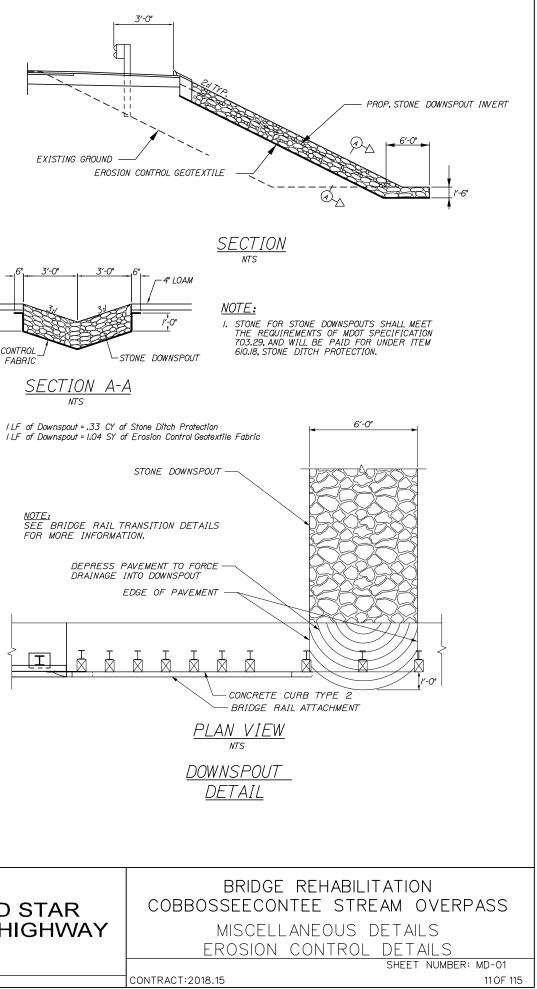
MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.

EROSION CONTROL_ GEOTEXTILE FABRIC SECTION A-A

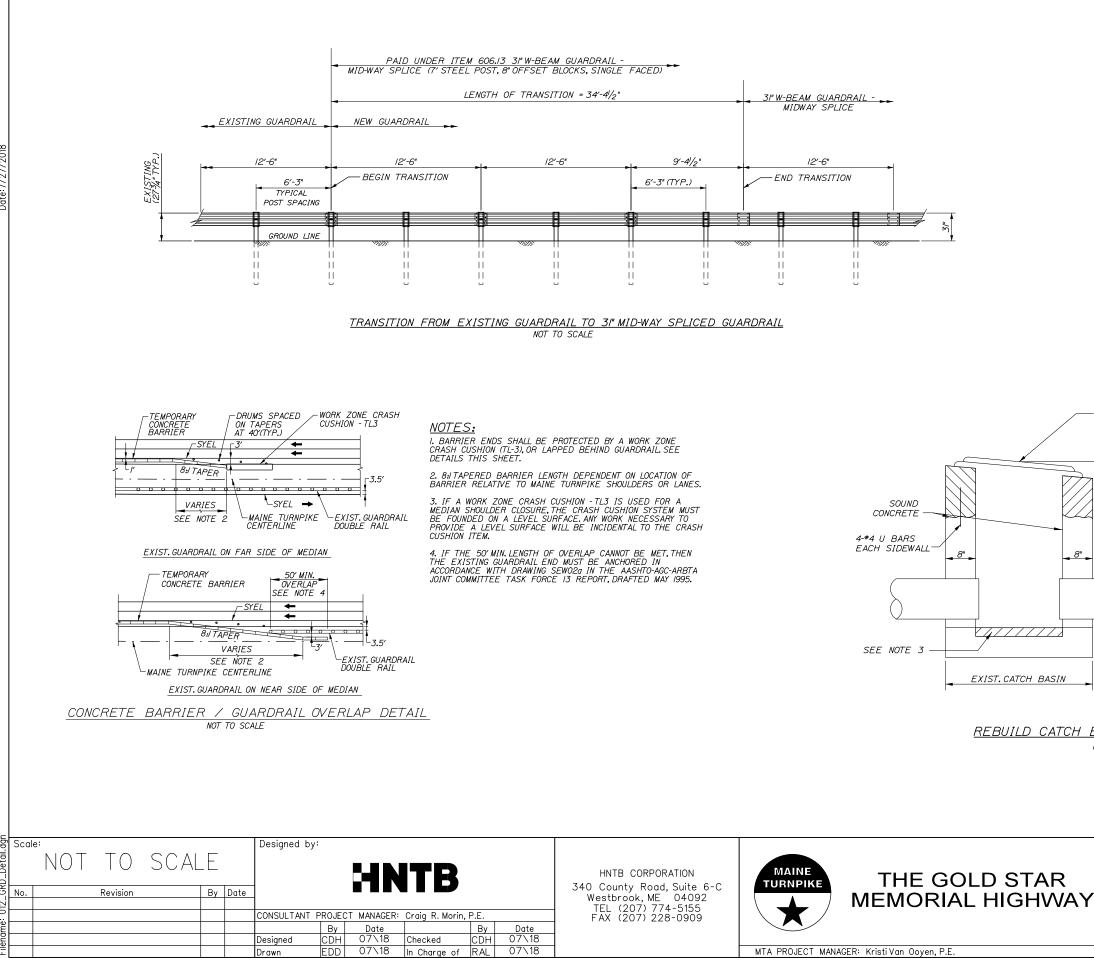
EXISTING GROUND

Ā I

Det1.







FDD

Drawn

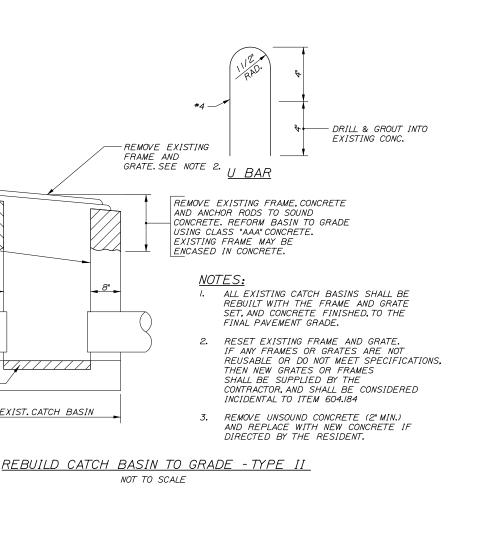
MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

NOTES:

I. MAINTAIN STANDARD I" CLEARANCE OF POST ABOVE PANEL THROUGHOUT THE ENTIRE LENGTH OF TRANSITION.

2. A MINIMUM OF ONE (1) 12'-6" PANEL SHALL BE PLACED BETWEEN THIS TRANSITION AND THE START OF ANY END TREATMENT OR ANCHORAGE.

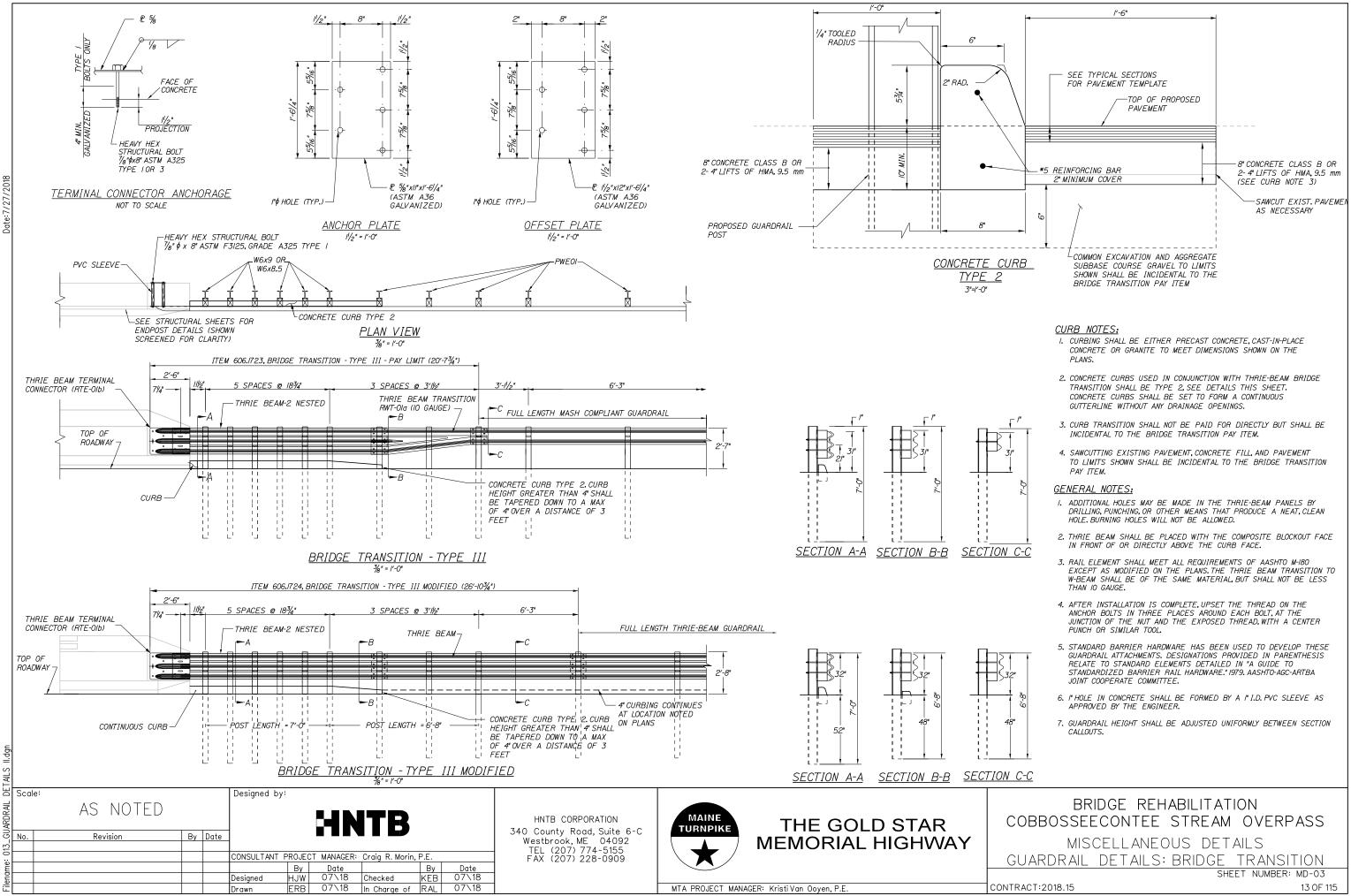
3. ALL NEW POSTS SHALL BE 84" IN LENGTH UNLESS OTHERWISE SPECIFIED BY THE ENGINEER.

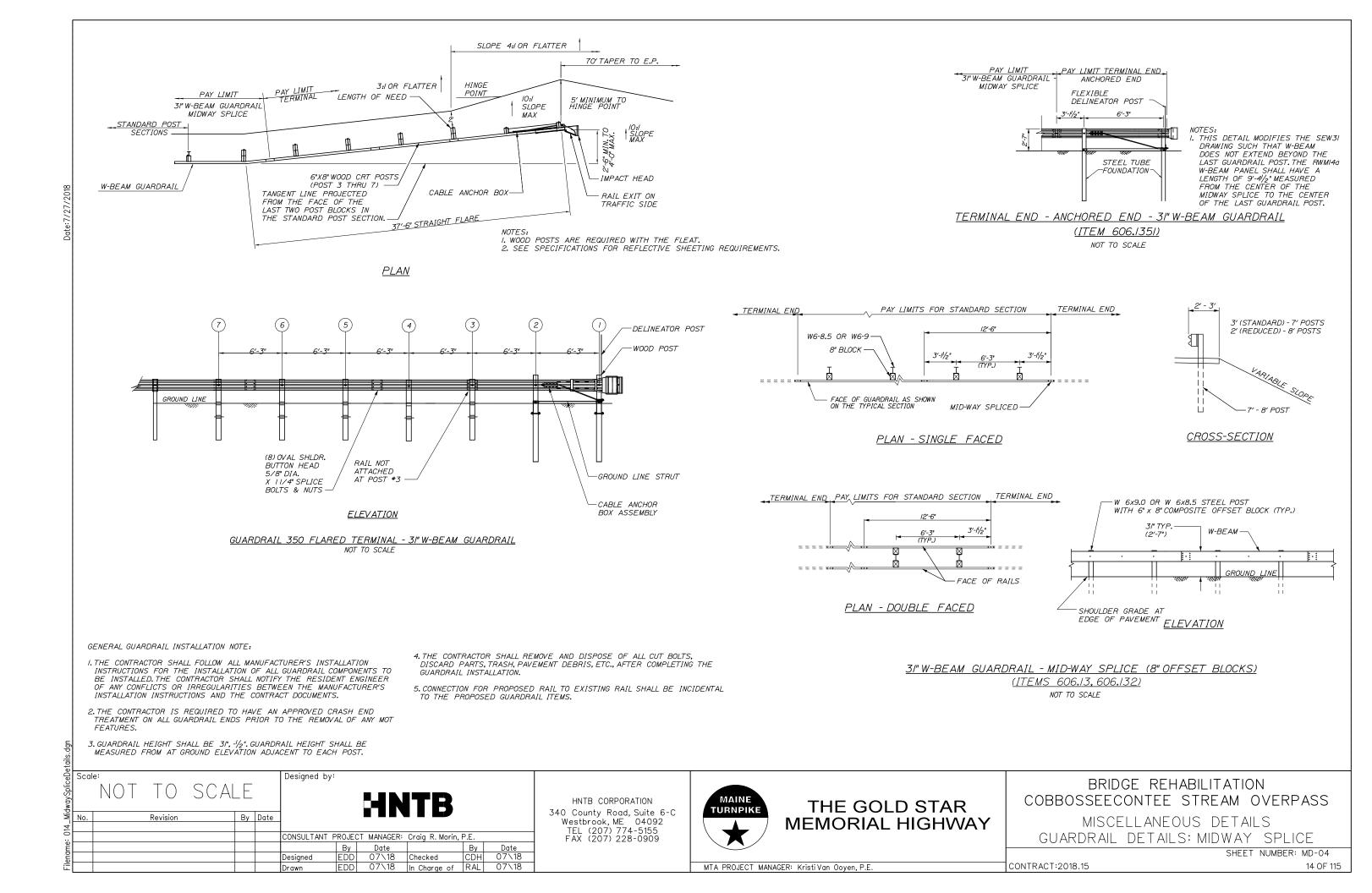


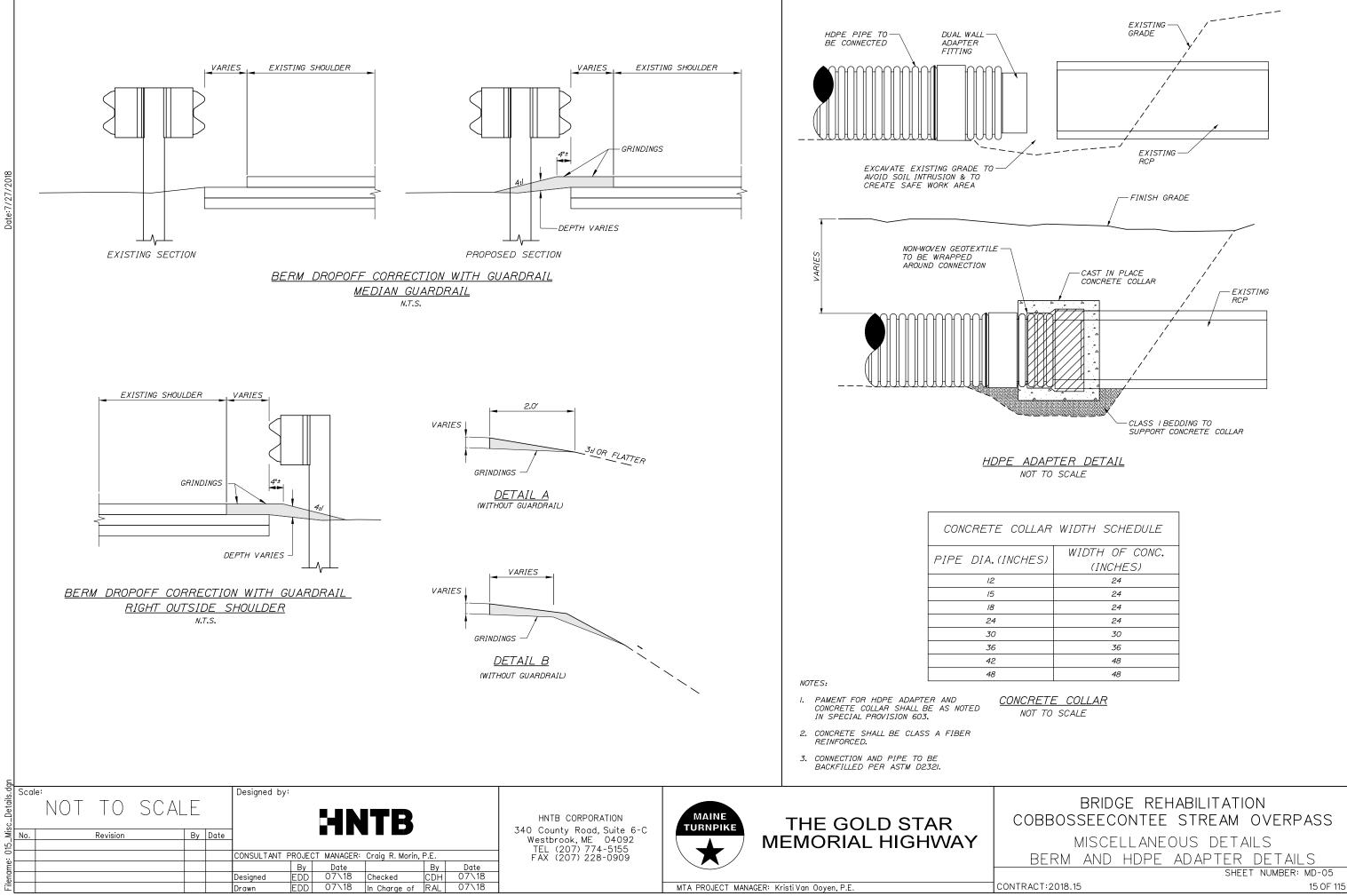
BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS MISCELLANEOUS DETAILS GUARDRAIL AND CATCH BASIN DETAILS SHEET NUMBER: MD-02

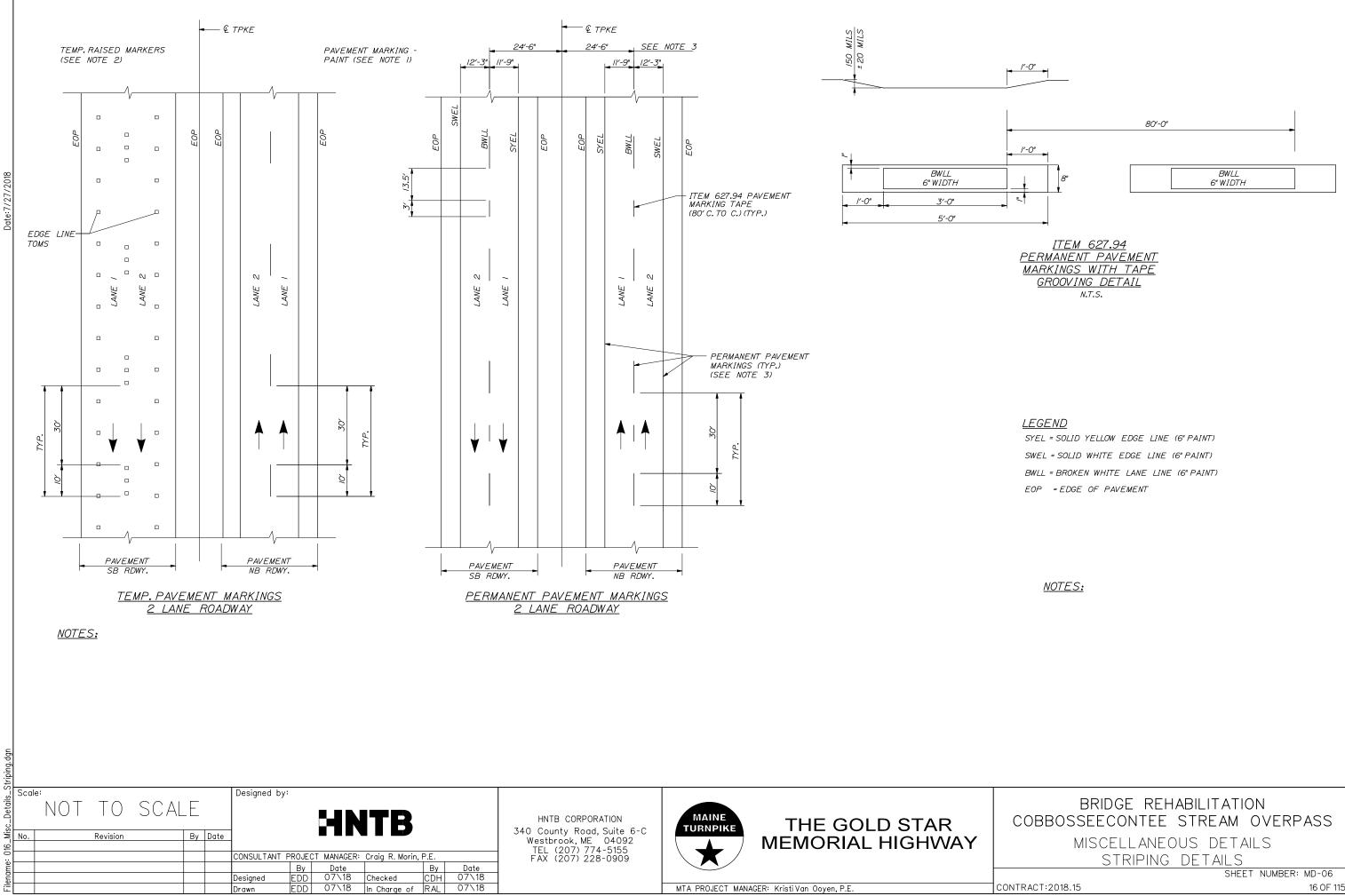
CONTRACT:2018.15

8"





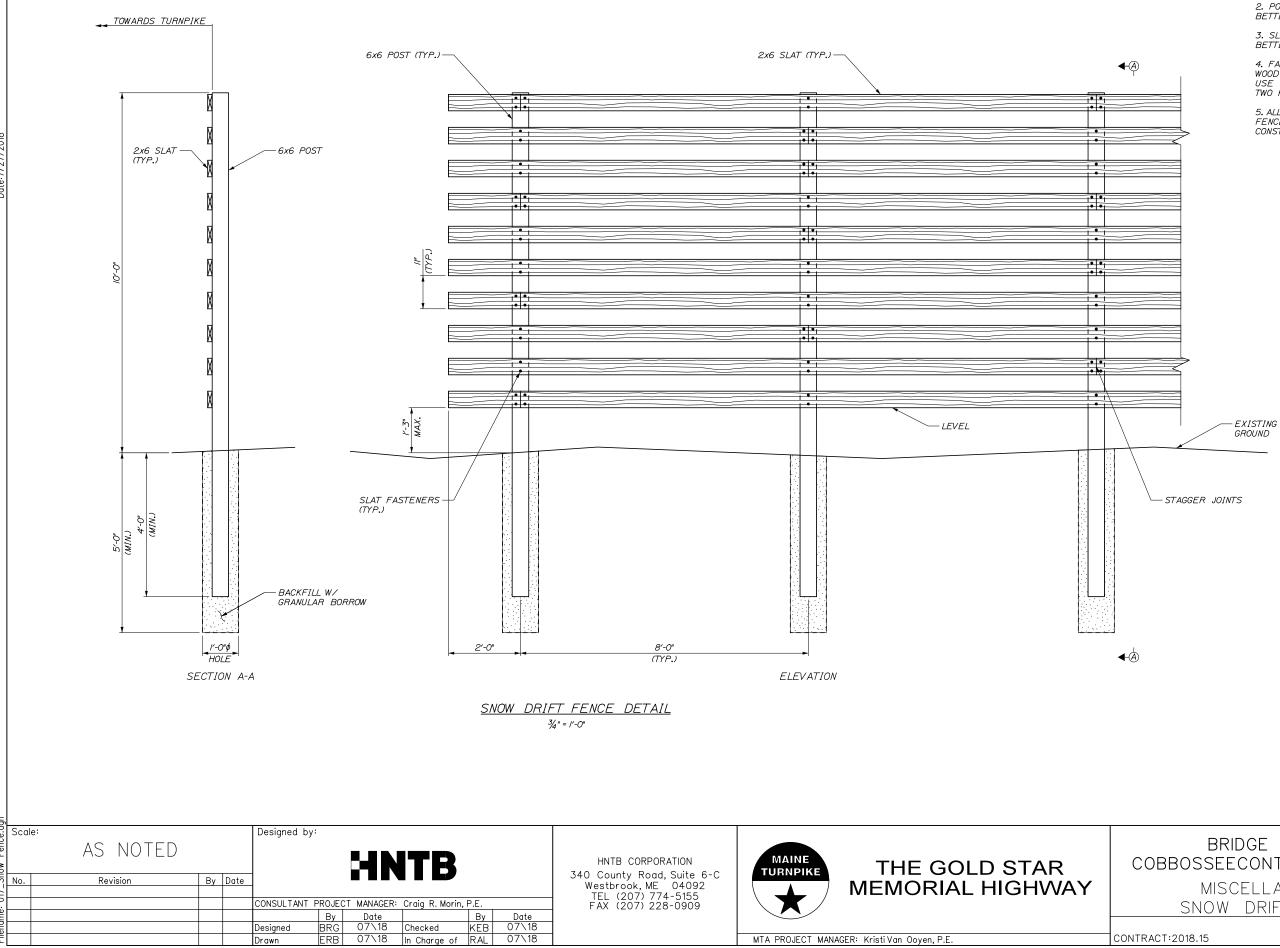




MTA	PROJECT	MANAGER:	Kristi Van	Ooven.	P.E

Drawn

CONTRACT:2018.15



<u>NOTES:</u>

I. ALL TIMBER DIMENSIONS ARE NOMINAL DIMENSIONS PRIOR TO SURFACING UNLESS OTHERWISE NOTED.

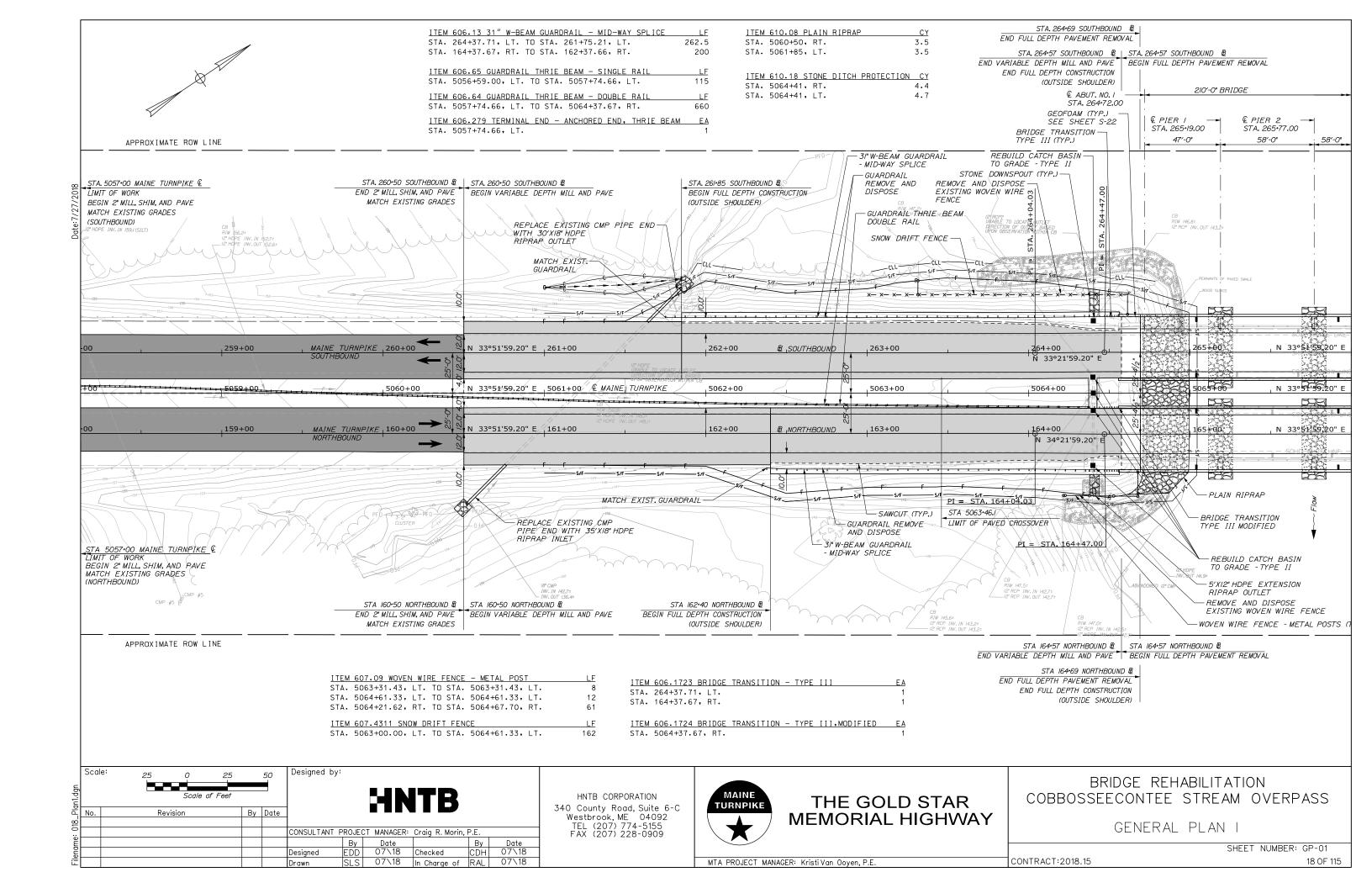
2. POST SHALL BE SOUTHERN YELLOW PINE, NO. I OR BETTER, PRESSURE TREATED FOR GROUND CONTACT.

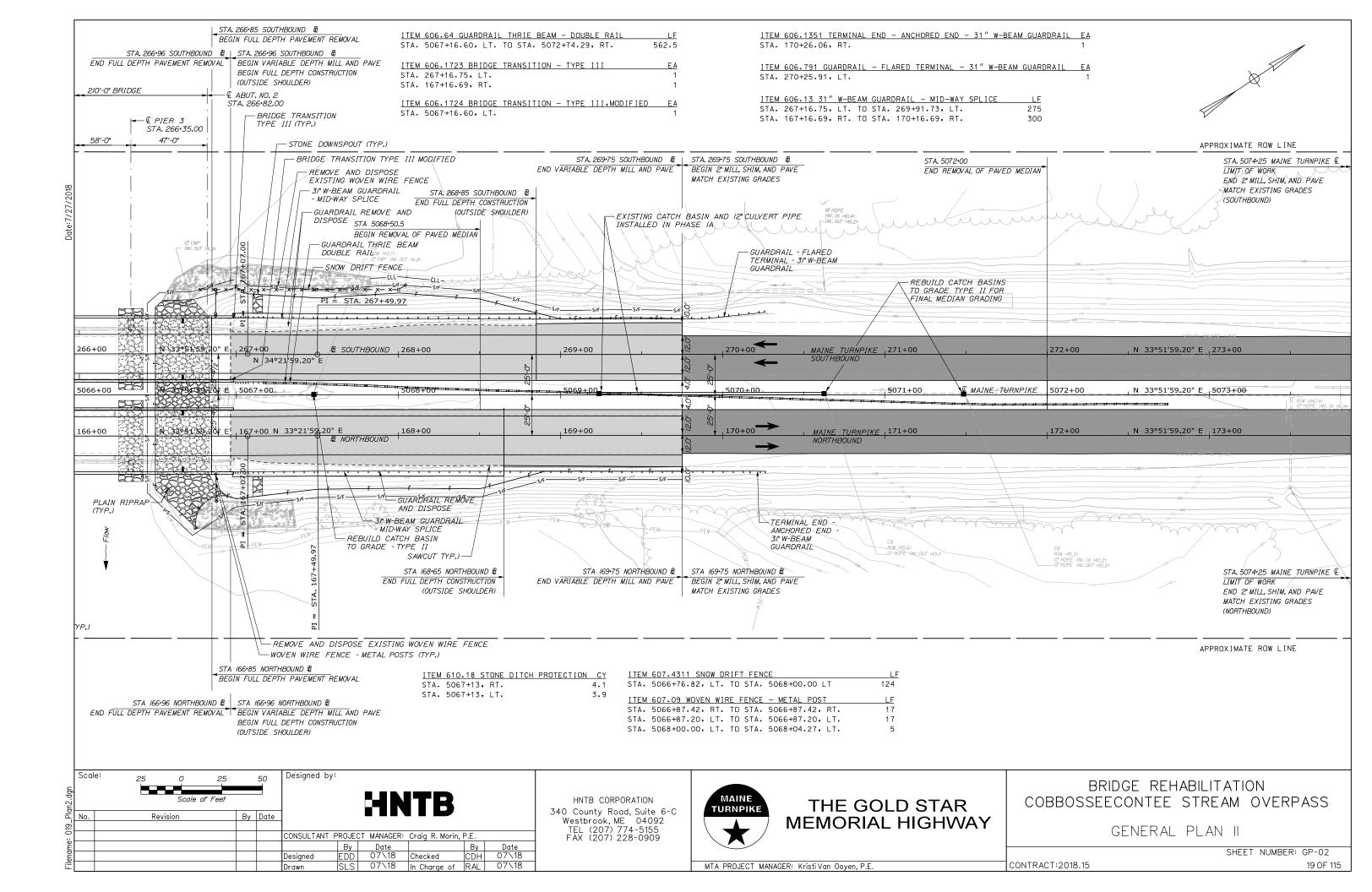
3. SLATS SHALL BE SOUTHERN YELLOW PINE, NO. I OR BETTER, PRESSURE TREATED FOR GROUND CONTACT.

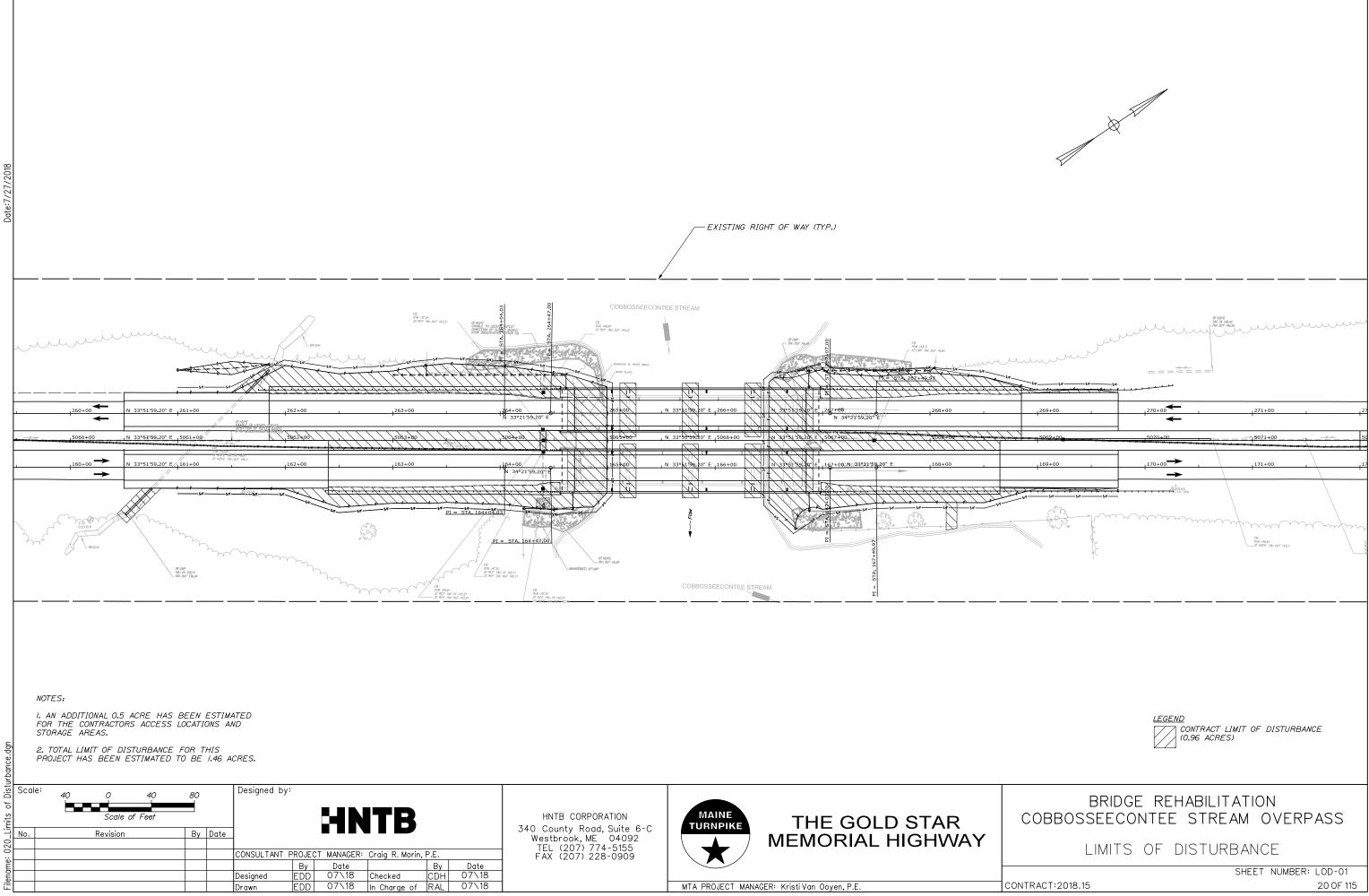
4. FASTENERS SHALL BE TIMBERLOK STRUCTURAL WOOD SCREWS, OR APPROVED EQUAL, PRE-DRILLED FOR USE WITH PRESSURE TREATED LUMBER. A MINIMUM TWO FASTENERS PER CONNECTION IS REQUIRED.

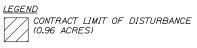
5. ALL WORK REQUIRED TO INSTALL THE SNOW DRIFT FENCE SHALL BE COMPLETED DURING PHASE I CONSTRUCTION.

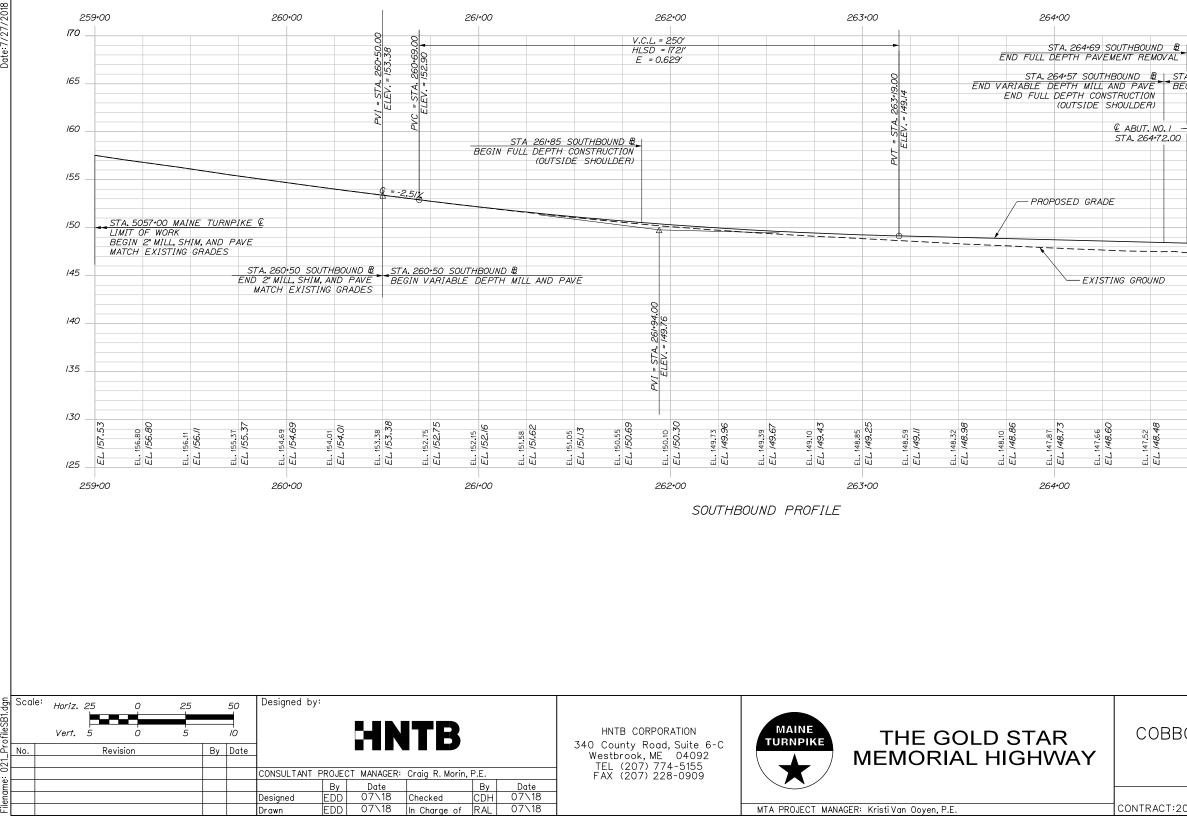
BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS MISCELLANEOUS DETAILS SNOW DRIFT FENCE DETAILS SHEET NUMBER: MD-07











265+00 266+00 . 170 STA. 264-57 SOUTHBOUND B END VARIABLE DEPTH MILL AND PAVE END FULL DEPTH CONSTRUCTION (OUTSIDE SHOULDER) . 165 210'-0" BRIDGE € ABUT. NO. 1 -STA. 264+72.00 160 € PIER I -STA. 265+19.00 € PIER 2 STA. 265+77.00 155 47'-0" 58′-0" 58'-0" 150 G = -0.50% 145 - EXISTING GROUND 140 👿 Q50 EL. 138.91 V QI.I EL. 135.02 /35 130 147.52 148.4 48. 125 265+00 266+00

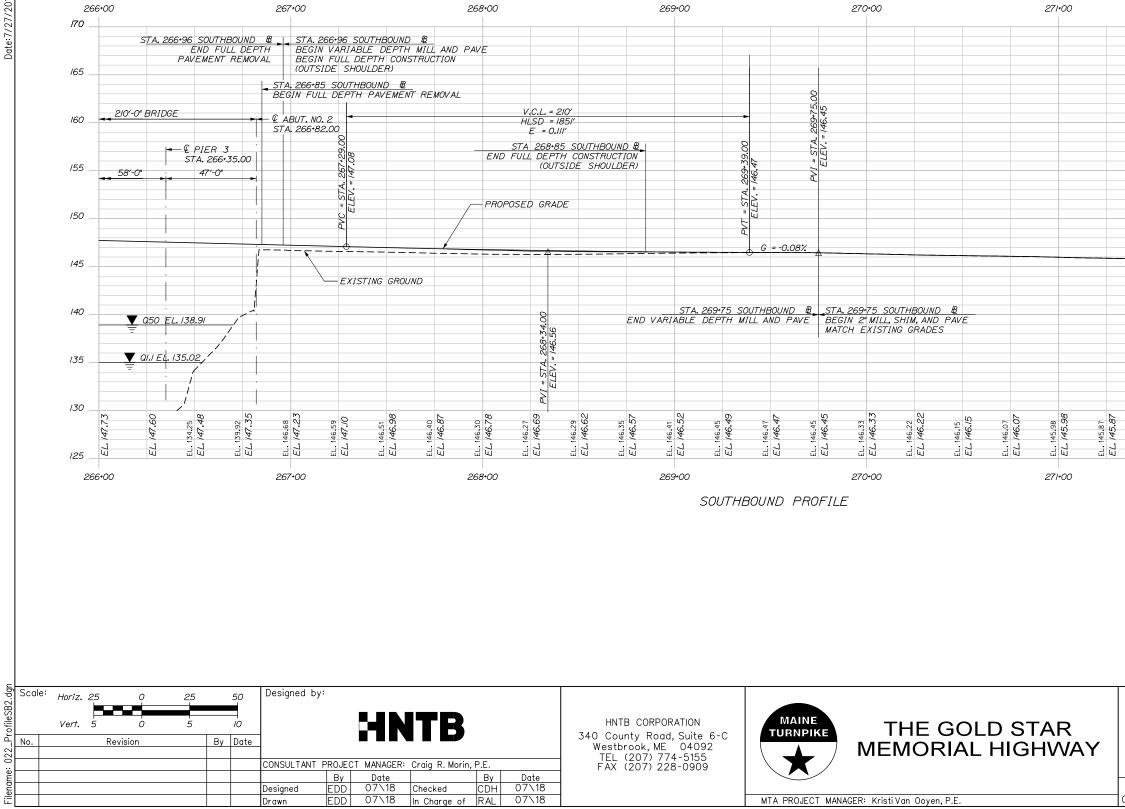
> BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

> > SOUTHBOUND PROFILE I

CONTRACT:2018.15

SHEET NUMBER: PR-01

210F 115



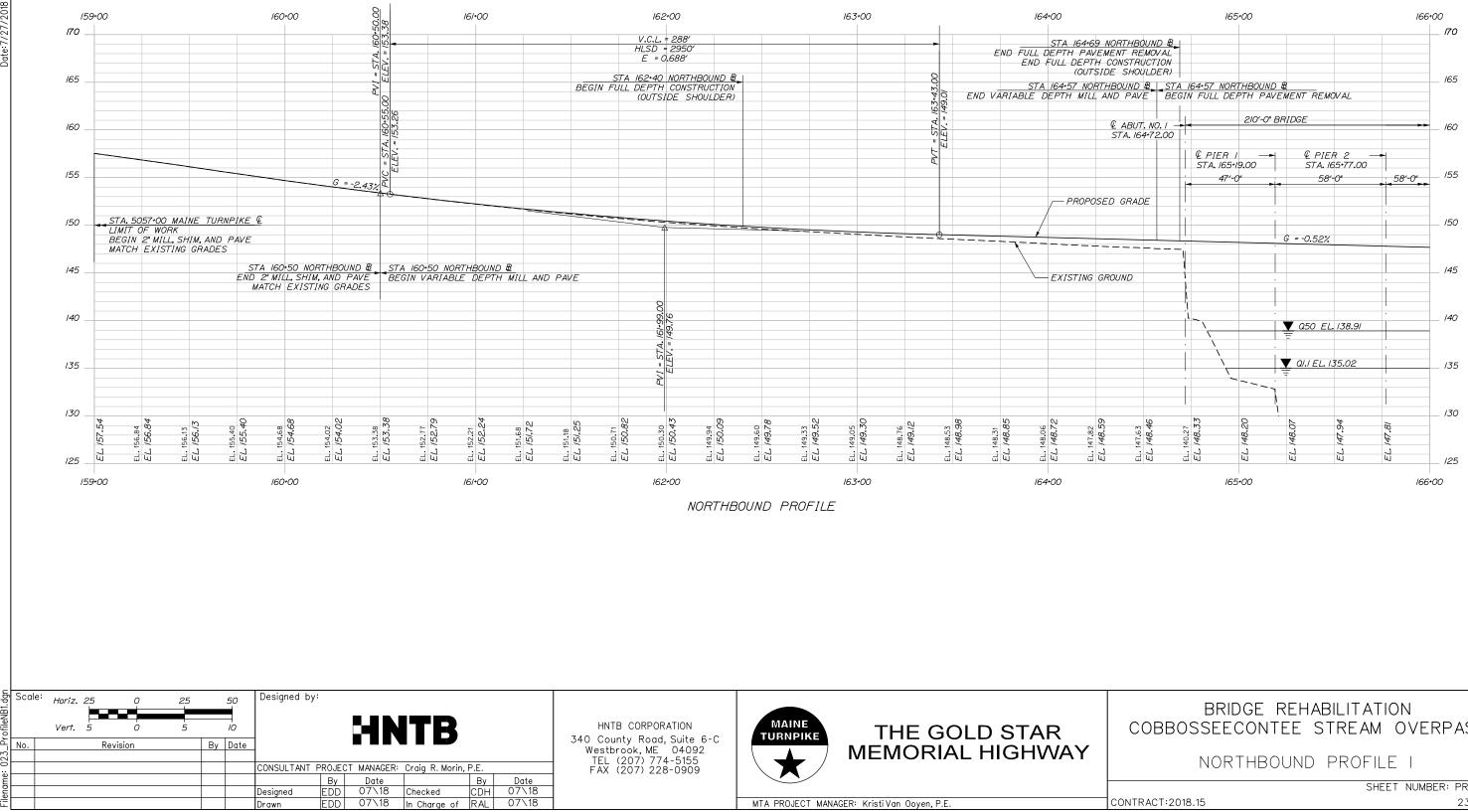
272+00 273+00 . 170 165 160 STA. 5074+25 MAINE TURNPIKE Q LIMIT OF WORK END 2" MILL, SHIM, AND PAVE MATCH EXISTING GRADES /55 150 145 140 /35 130 145.53 *1*45.53 145.61 145.61 145.81 145.81 145.74 145.74 145.77 145.7 145.70 145.7 ШШ ШШ 금납 -i 4 ц. . 125 272+00 273+00

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

SOUTHBOUND PROFILE II

CONTRACT:2018.15

SHEET NUMBER: PR-02



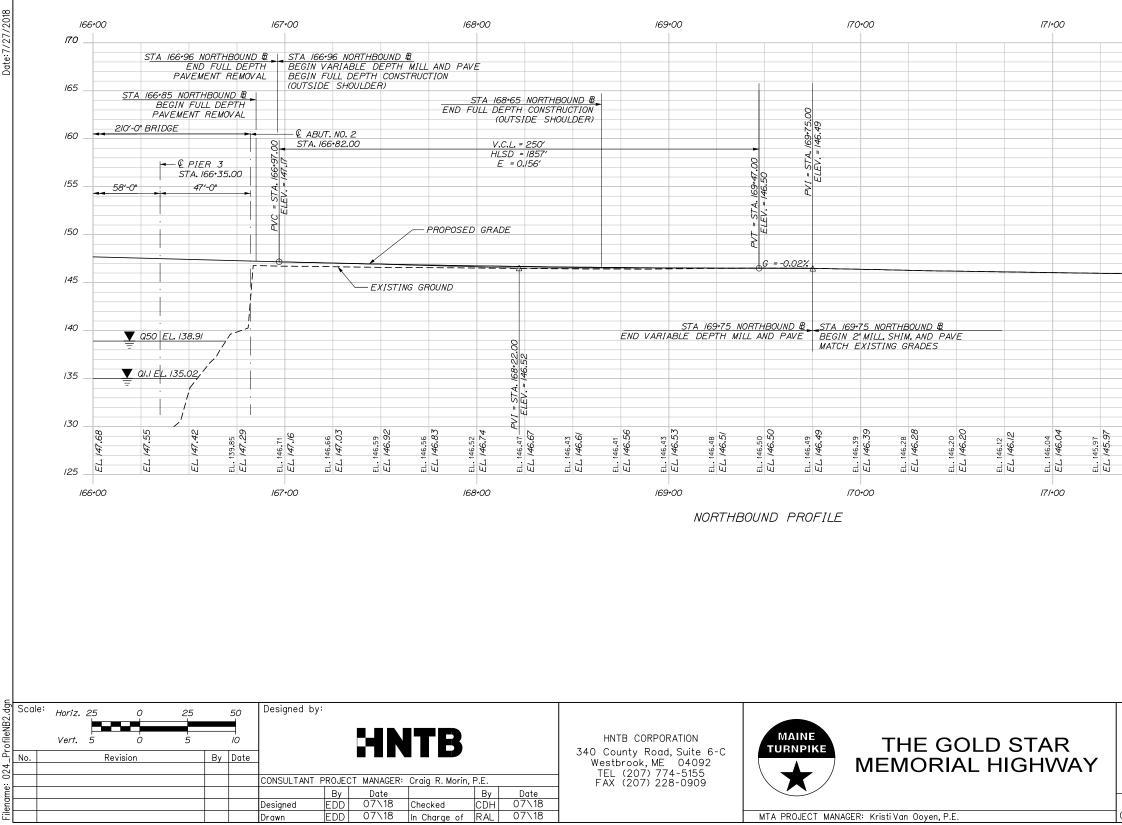
Drawn

MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.

COBBOSSEECONTEE STREAM OVERPASS

CONTRACT:2018.15

SHEET NUMBER: PR-03



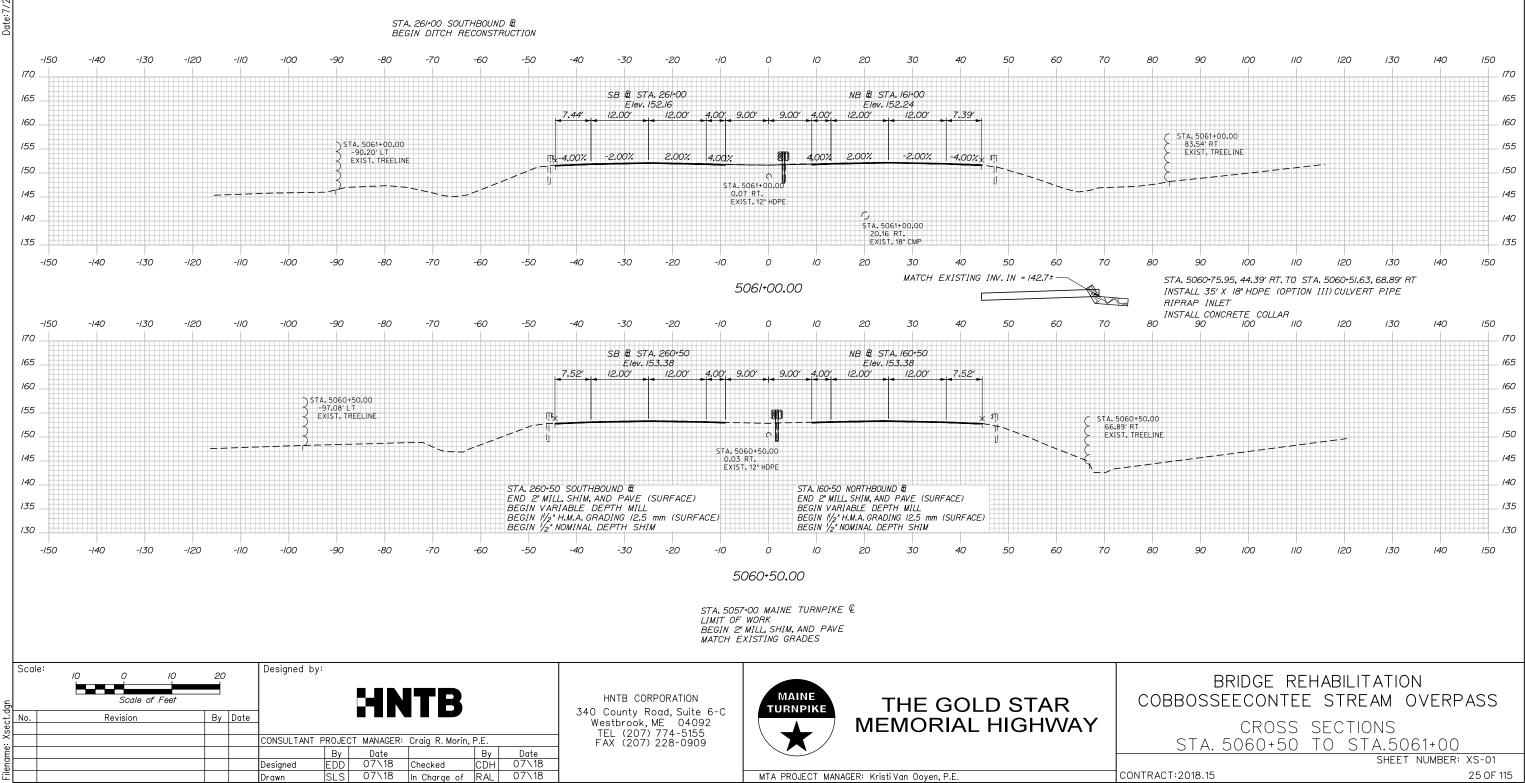
165 STA. 5074-25 MAINE TURNPIKE LIMIT OF WORK END 2" MILL, SHIM, AND PAVE 160 MATCH EXISTING GRADES /55 150 145 140 /35 130 145.53 145.53 145.89 *145.89* 145.60 *145.60* 145.81 145.8/ 45.65 *145.6*5 45.72 14**5.7** 45 Ц. 1 금급 금님 금납 Ц 125 172+00 173+00 BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS NORTHBOUND PROFILE II SHEET NUMBER: PR-04 CONTRACT:2018.15 24 OF 115

172+00

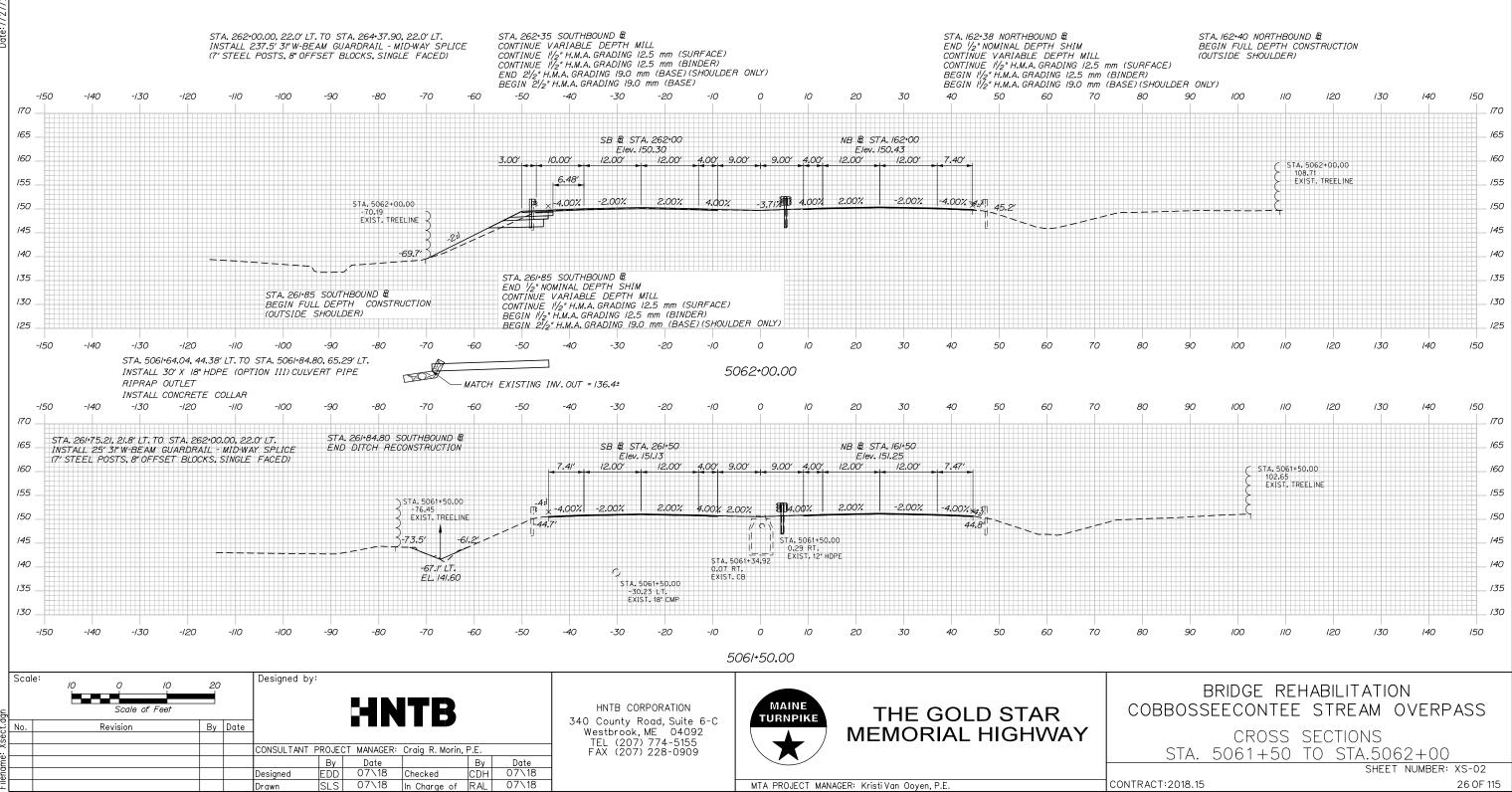
173+00

. 170

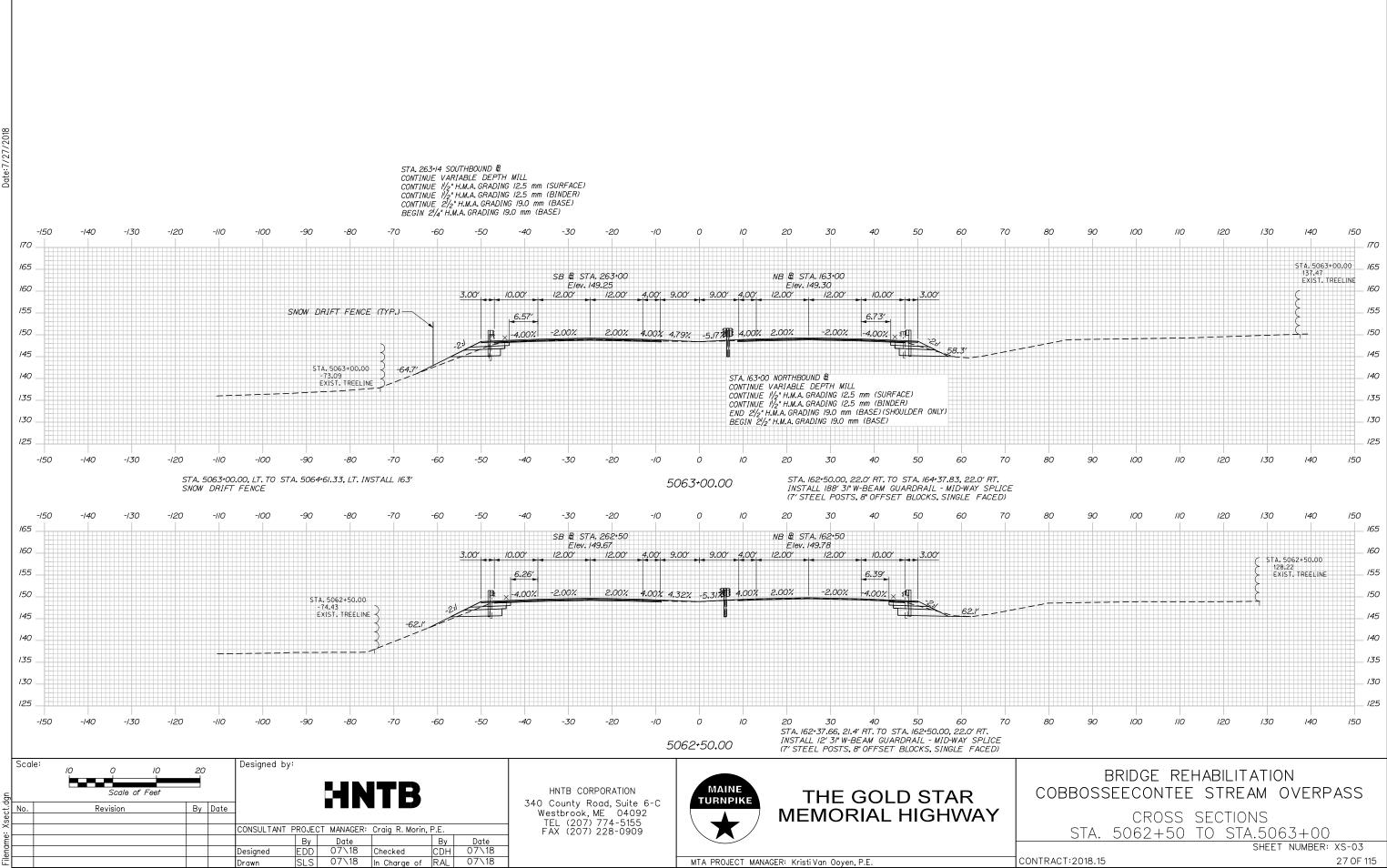
ate:7/27

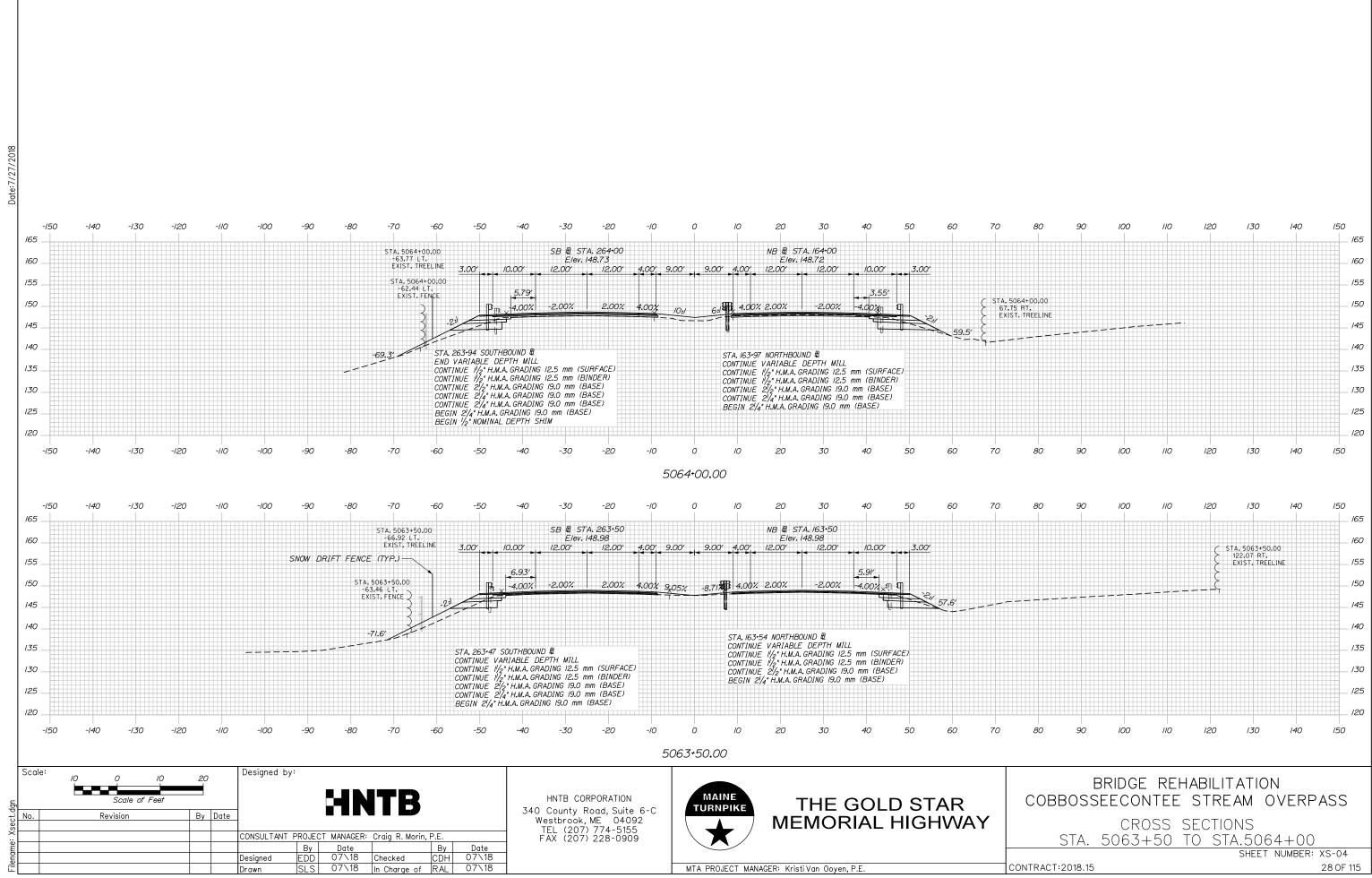


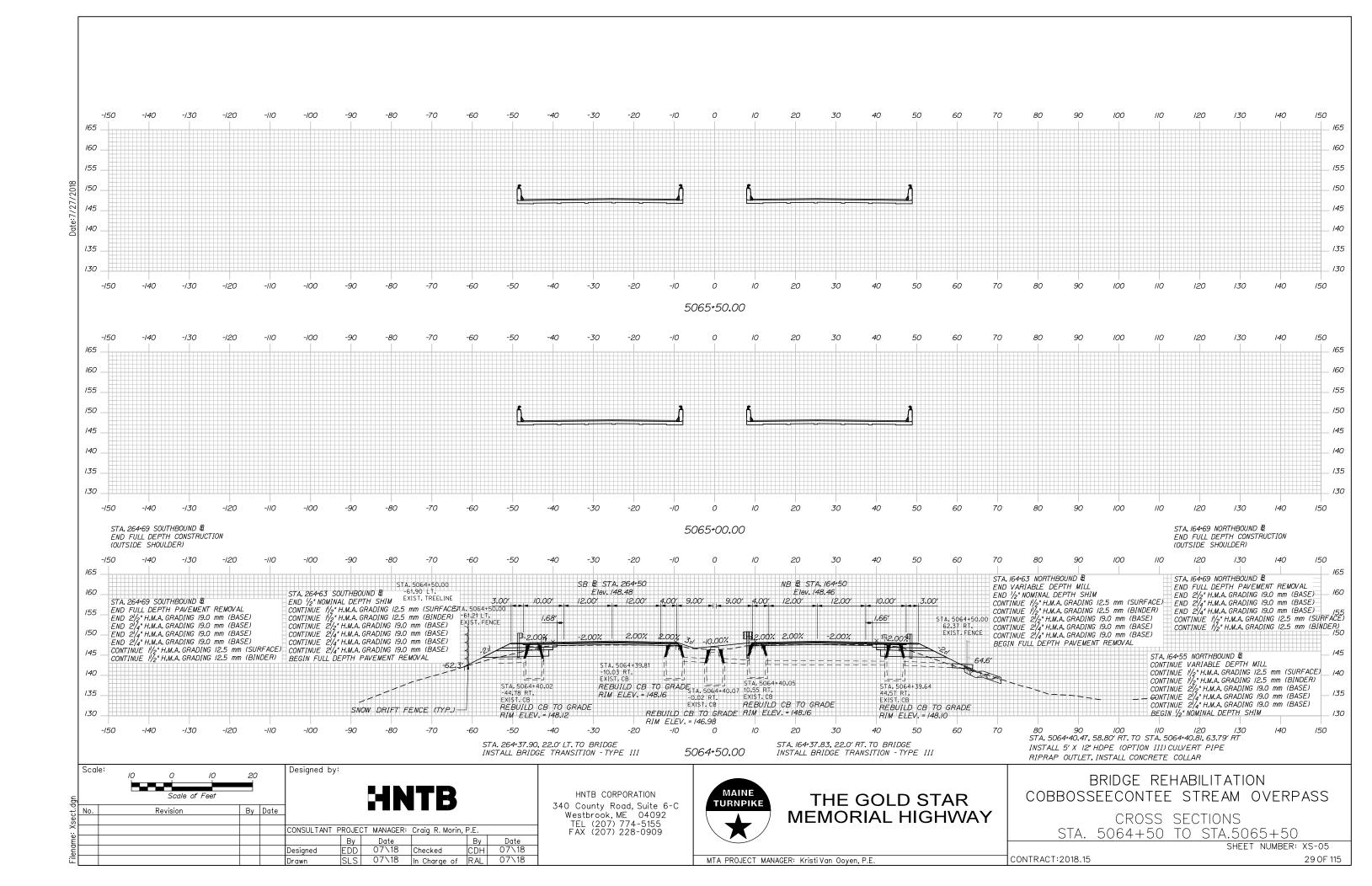


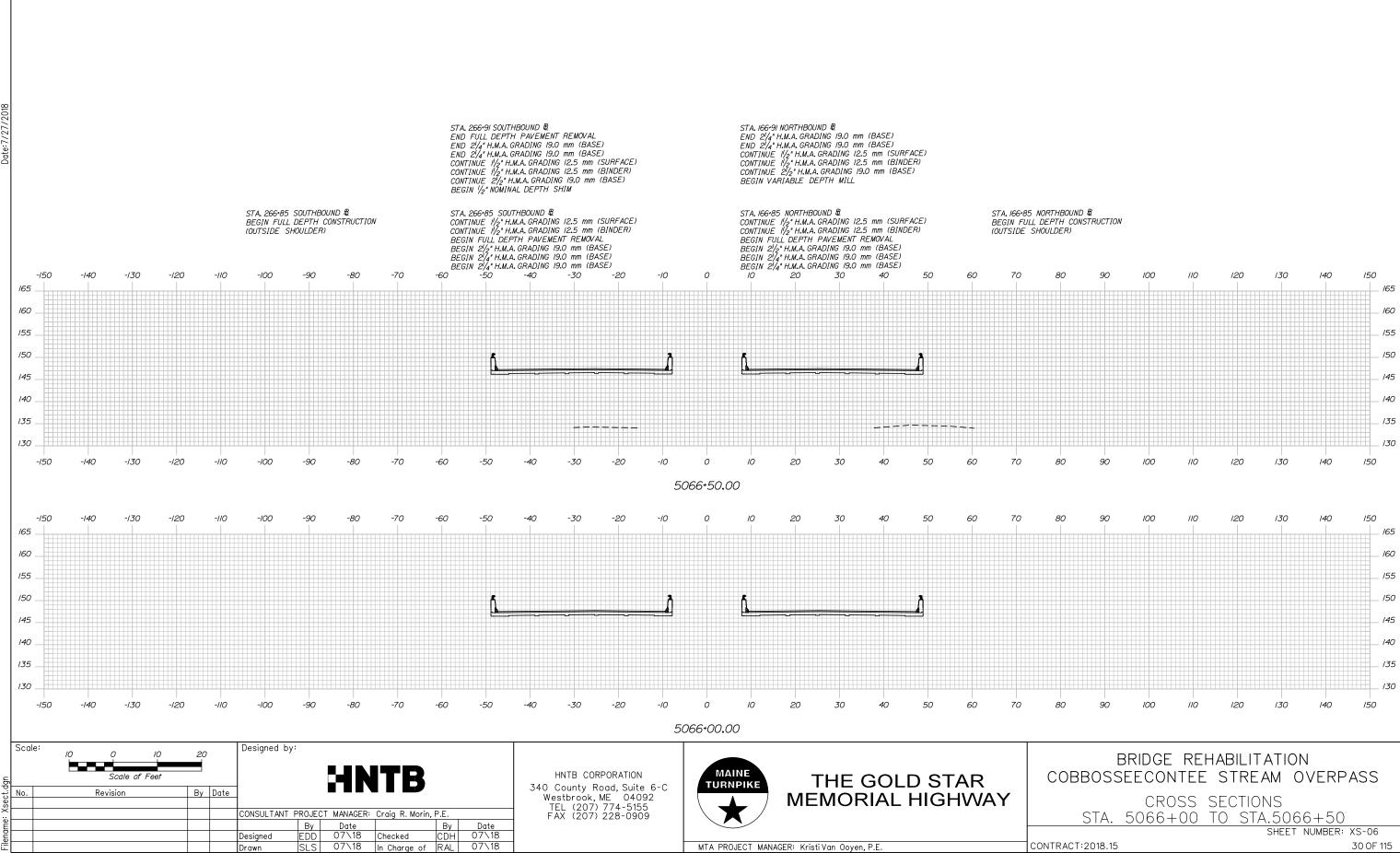


CONTRACT:2018.15

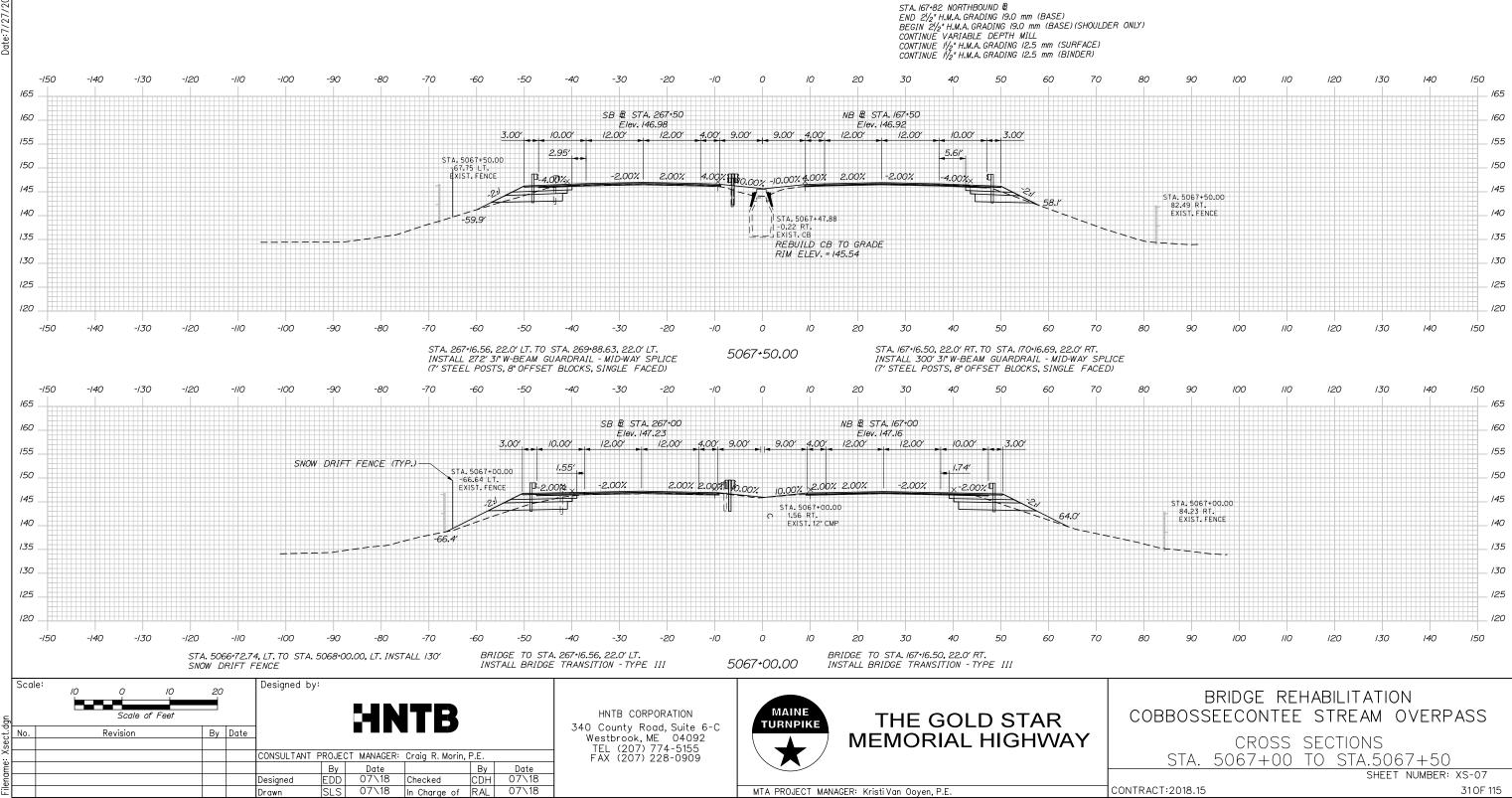




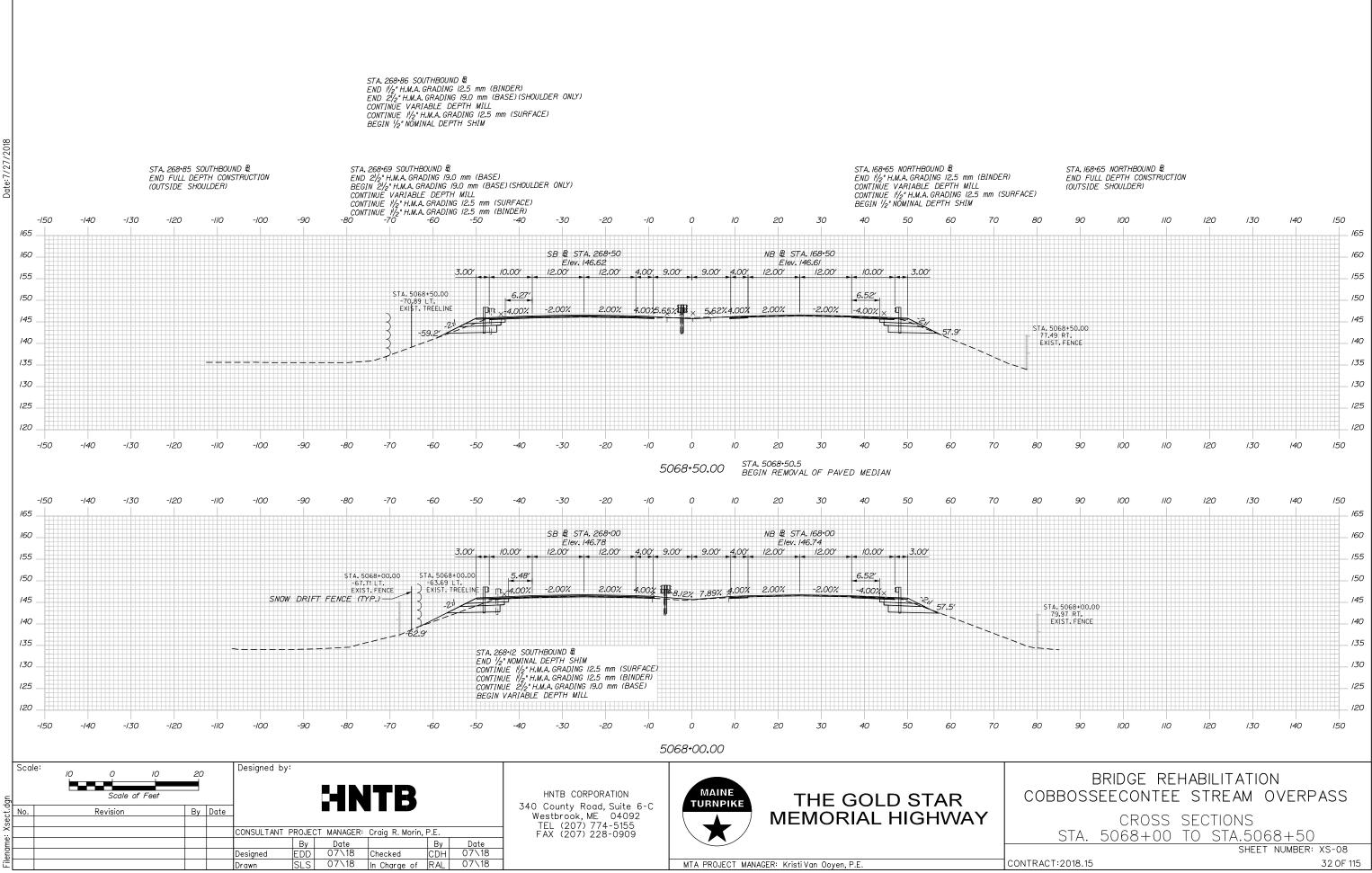


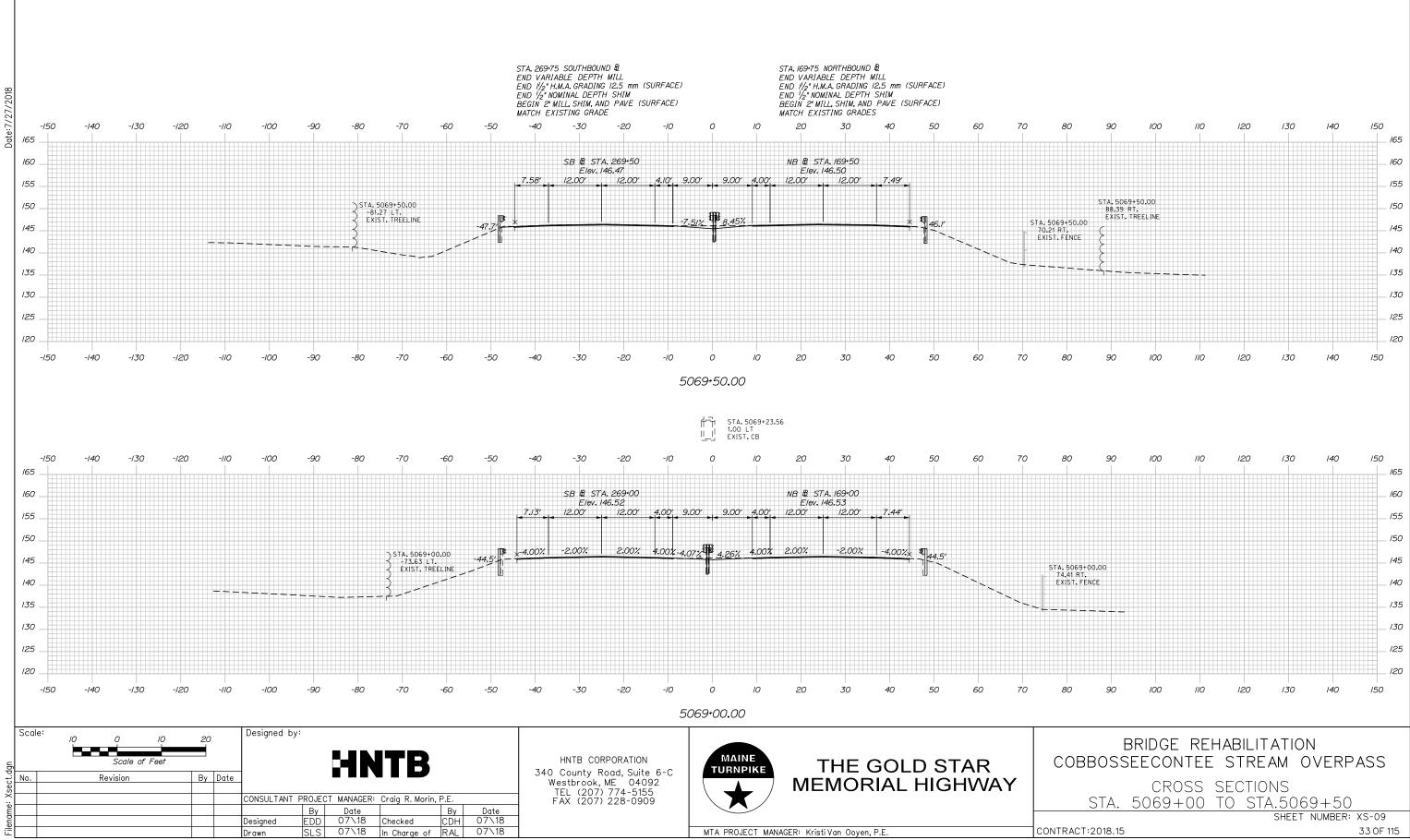


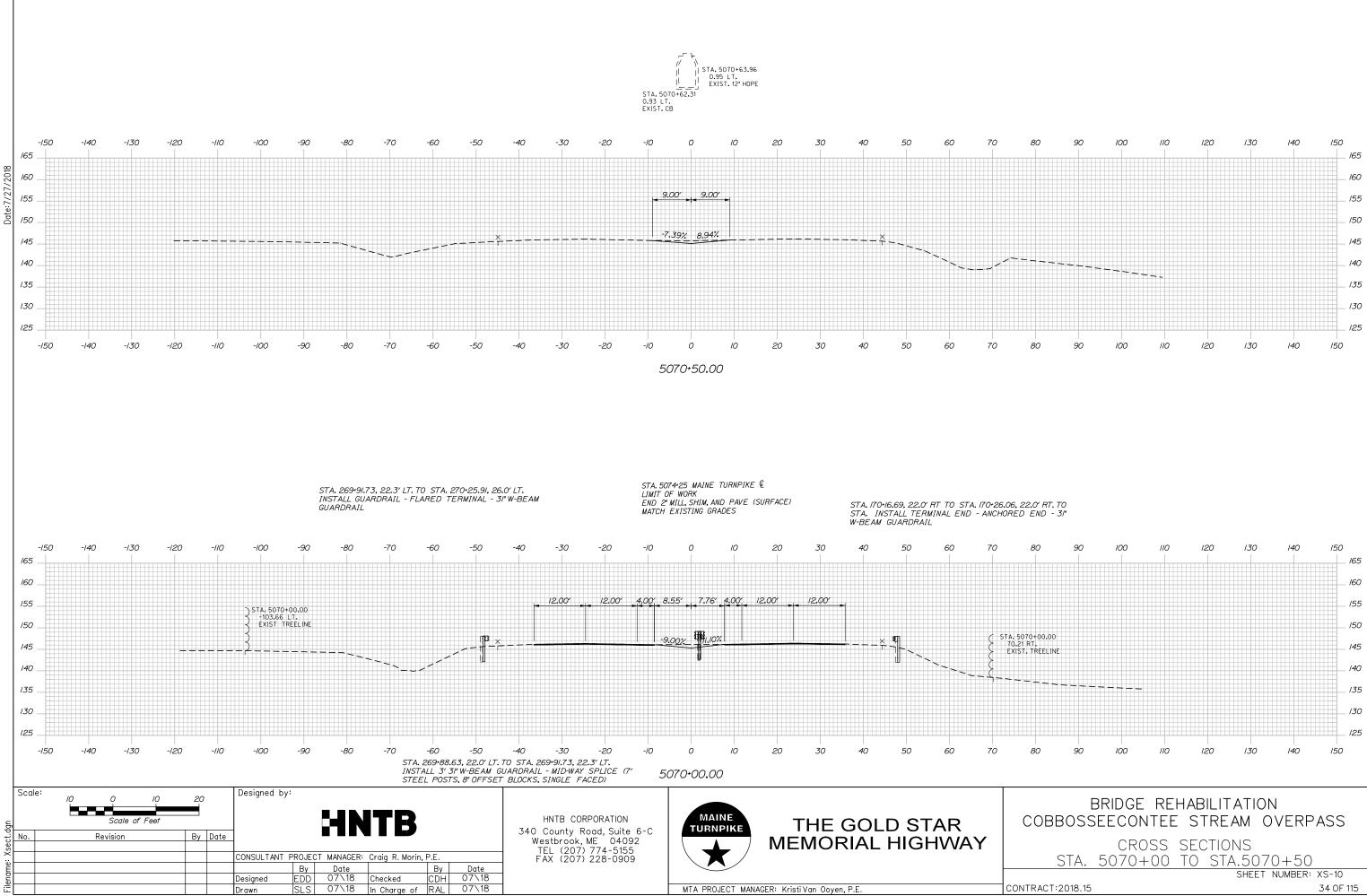




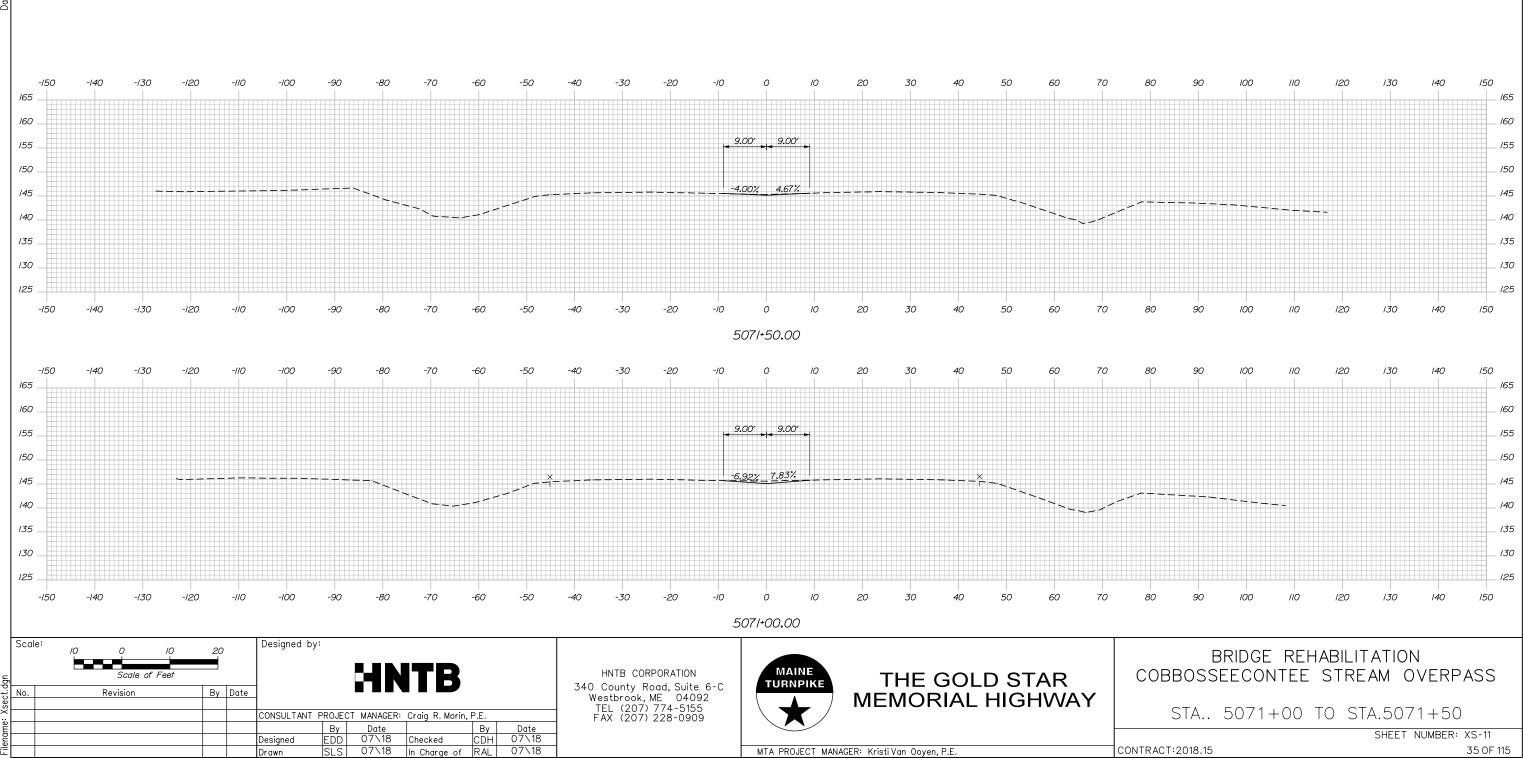
CONTRACT:2018.15

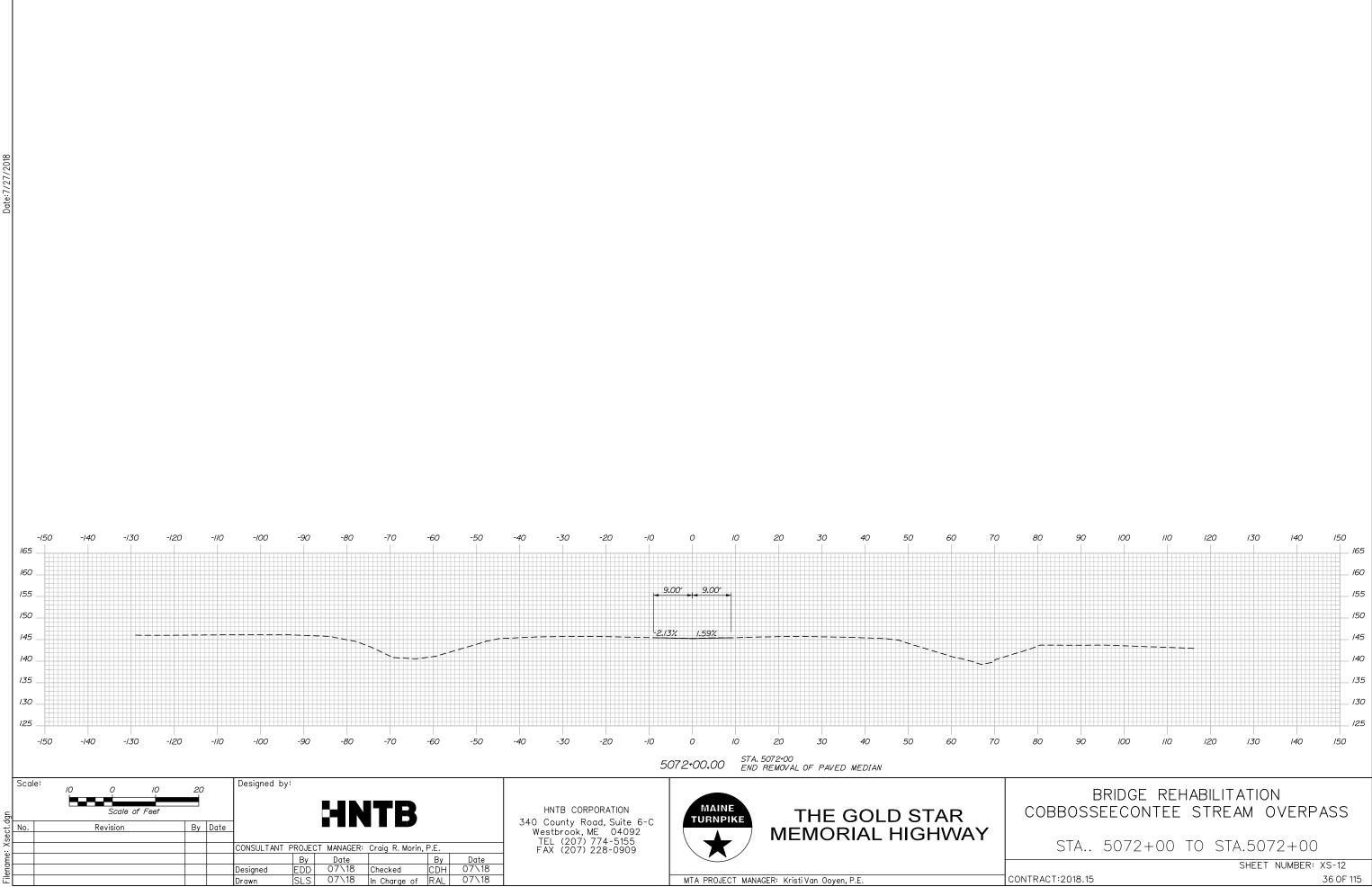












	<u>SPECIFICATIONS</u> <u>DESIGN</u> AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 8TH EDITION. CONSTRUCTION	ш	EM NO.	ITEM DES CRIPTION	REFERENCE QUANTITY	UNIT	COBB. STRUCTURAI QUANTITY
	<u>CONSTRUCTION</u> STATE OF MAINE, DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS, HIGHWAYS AND BRIDGES, REVISIO	202. 202.		Removing Existing Superstructure Property of Contractor Removing Existing Structural Concrete	420 CY	LS CY	1 140
	OF NOVEMBER 2014.	202.	.13	Removing Existing Railings Retained by Authority		LF	980
	STATE OF MAINE DEPARTMENT OF TRANSPORTATION STANDARD DETAILS FOR HIGHWAYS AND BRIDGES, DECEMBE			Geofoam Lightweight Fill		CY	280
	2014 WITH LATEST REVISIONS.	203.		Leveling Sand		CY	69
	AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS, THIRD EDITION WITH 2017 INTERIMS.	206. 206.		Structural Earth Excavation - Major Structures, Plan Quantity Structural Earth Excavation - Piers		CY CY	420
	DESIGN LOADING			Hot Mix As phalt, 12.5 mm (Polymer Modified) - RAP		Ton	150
	LIVE LOAD-HL-93			Hot Mix Asphalt, 12.5 mm Nominal Maximum Size (Base and			
		403.		Intermediate Base Course)		Ton	150
	MATERIALS	409.		Bituminous Tack Coat - Applied		Gal	110
	<u>CONCRETE</u>			Dynamic Loading Test		EA	4
∞	DECK CONCRETE - CLASS AAA - DECK	501. 501.		Steel H-beam Piles 42 lb/ft, delivered Steel H-beam Piles 42 lb/ft, in place		LF	510 510
50	SEAL CONCRETE - CLASS S ALL OTHER CONCRETE SHALL BE CLASS AAA.	501.		Pile Tips		EA	16
27	REINFORCING STEEL	501.		File Splices		EA	5
Date:7/27/2018	AASHTO M31, GRADE 60 EPOXY COATED	501.		Pile Driving Equipment Mobilization		LS	1
ate			.219	Structural Concrete, Abutments and Retaining Walls	160 CY	LS	1
	ANCHOR RODS SHALL MEET THE REQUIREMENTS OF ASTM F1554, GRADE 55 AND SHALL BE SWEDGED OR THREAL THE EMBEDDED PORTION OF THE ROD.	DED ON 502.	.239	Structural Concrete Piers	170 CY	LS	1
		502.		Structural Concrete Piers (placed under water)		CY	310
	<u>STRUCTURAL STEEL</u>	502.		Structural Concrete Roadway and Sidewalk Slab on Steel Bridges	460 CY	LS	1
	GIRDERS: FLANGES, WEBS, SPLICE PLATES, FILLER PLATES, DIAPHRAGMS, AND BEARING STIFFENERS SHALL BE AA	ASHTO 502.		Structural Concrete Parapets Structural Concrete Approach Slab	110 CY 24 CY	LS LS	1
	M270, GRADE 50.	503.		Epoxy-Coated Reinforcing Steel, Fabricated and Delivered	24 C 1	LS	258,900
	STEEL H-PILES INCLUDING PILE TIPS SHALL BE ASTM A572, GRADE 50.	503.		Epoxy-Coated Reinforcing Steel, Placing		LB	258,900
	ALL OTHER STRUCTURAL STEEL SHALL BE AASHTO M270, GRADE 36, OR APPROVED EQUAL.	504.	.702	Structural Steel, Fabricated and Delivered, Welded	80,400 LB	LS	1
		504.	.71	Structural Steel Erection	80,400 LB	LS	1
	HIGH STRENGTH BOLTS SHALL BE AASHTO MI64 (ASTM F3125, GRADE A325, TYPE 1). BOLTS SHALL BE HOT DIPPEL GALVANIZED TYPE 1.	500.		Shear Connectors	1258 EA	LS	1
				Zinc Rich Coating System (Shop Applied)	80,400 LB	LS	1
	<u>PROTECTIVE COATING</u>	507		Field Touch-Up of Existing Steel	33,000 LB	LS	1
	ALL NEW STRUCTURAL STEEL EXCEPT DIAPHRAGMS SHALL BE SHOP COATED WITH NEPCOAT QUALIFIED PRODUCT FROM LIST A. PAINT COLOR SHALL BE SELECTED TO MATCH EXISTING GIRDERS.	- 507.		Aluminum Bridge Railing, 1 Bar High Performance Waterproofing Membrane	872 LF 1820 SY	LS	1
	FROM LIST A. FAINT COLOR SHALL BE SELECTED TO MATCH EXISTING GIRDERS.	508.		Cofferdam Pier 1 - NB	1020 3 1	LS	1
	DIAPHRAGMS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123.	511.		Cofferdam Pier 1 - SB		LS	1
	BASIC DESIGN STRESSES	511.	.073	Cofferdam Pier 2 - NB		LS	1
	<u>CONCRETE</u> - CLASS AAA, f'c = 4,500 P.S.I.	511.		Cofferdam Pier 2 - SB		LS	1
	- CLASS AAA - DECK f'c = 4,500 P.S.I.	511.		Cofferdam Pier 3 - NB		LS	1
	<u>REINFORCING_STEEL</u> - fy = 60,000_P.S.I.	511.		Cofferdam Pier 3 - SB		LS	1
	STRUCTURAL STEEL AASHTO M270 (ASTM A709)GRADE 36,Fy = 36,000 P.S.I.	514.		Curing Box for Concrete Cylinders Pigmented Protective Coating for Concrete Surfaces		EA SY	1 1,400
	AASHTO M270 (ASTM A709) GRADE 50, $Fy = 50,000$ P.S.I.	515.		Clear Protective Coating for Concrete Surfaces		SY	1,400
	AASHTO M270 (ASTM A572)GRADE 50, Fy = 50,000 P.S.I.	518.		Epoxy Injection Crack Repair		LF	45
	<u>GENERAL NOTES:</u>	518.	.401	Epoxy Injection Crack Repair - Below Waterline		LF	15
	. THE PROPOSED ELEVATIONS ARE BASED ON THE NAVD 88 DATUM.THE AS-BUILT PLANS ARE BASED ON NGVD 29	DATUM. 518.	.51	Repair of Upward Facing Surfaces - Below Reinforcing Steel < 8 inches		SF	24
	2. FOR ADDITIONAL DETAILS REFERENCED OR NOT SHOWN IN THESE DRAWINGS, SEE THE STATE OF MAINE, DEPARTM			Repair of Vertical Surfaces < 8 inches		SF	43
	TRANSPORTATION STANDARD DETAILS, HIGHWAYS AND BRIDGES, NOVEMBER 2014 WITH UPDATES.	518.		Repair of Vertical Surfaces < 8 inches - Below Waterline		SF	37
	3. COPIES OF THE AS-BUILT PLANS ARE POSTED ON THE MAINE TURNPIKE AUTHORITY WEBSITE AT	520.		Asphaltic Plug Joint Joint Armor Repair		LF	160
	WWW.MAINETURNPIKE.COM/PROJECT-AND-PLANNING/CONSTRUCTION-CONTRACTS.ASPX.THE COMPLETENESS AND ACCURACY (NOT COLORATECE TO A CONTRACT OF A CONTRACT	OF THESE PLANS IS		Bearing Installation		EA	60
	NOT GUARANTEED.			Laminated Elastomeric Bearings, Fixed		EA	12
	4. REINFORCING STEEL SHALL HAVE A CLEAR COVER OF 2", UNLESS OTHERWISE NOTED.	523.	.54021	PTFE Elastomeric Bearings, Expansion		EA	48
	5. CHAMFER ALL EXPOSED CONCRETE EDGES $\frac{3}{4}$ " UNLESS OTHERWISE NOTED.	524.	.302	Temporary Structural Support - Cobbosseecontee Girders		LS	1
		524.		Temporary Structural Support - Cobbosseecontee Braces		LS	1
	5. ALL BRIDGE PARAPET, BARRIER, WINGWALL AND ENDPOST CONCRETE, INSIDE FACE AND TOP FACE, SHALL HAVE A R PRIOR TO THE APPLICATION OF THE PROTECTIVE COATING FOR CONCRETE SURFACE.			Protective Shielding - Steel Girders		SY	2,250
	ANDA TO THE AFFECATION OF THE FROTEFINE COATING TOR CONCRETE SUM ACE.			Bridge Transition - Type III Bridge Transition - Type III, Modified		EA EA	4
	THE EXISTING GIRDERS SHALL BE JACKED IN-PLACE TO FACILITATE PEDESTAL RECONSTRUCTION, AND BEARING RE	EPLACEMENT. THIS		Sloped Curb Type 1		LF	970
	NORK SHALL BE PAID FOR UNDER ITEM 523.52, INSTALLATION. THE EXISTING EXTERIOR GIRDERS SHALL BE TEMPOR, FACILITATE SUBSTRUCTURE WIDENING. THIS WORK SHALL BE PAID FOR UNDER ITEM 524.302, TEMPORARY STRUCTURA.	ANILI SUITUNILD IU -		Plain Riprap		CY	1,100
		620.		HDPE Geomembrane		SY	700
	B. THE STEEL PORTIONS OF THE EXISTING BRIDGE ARE COATED WITH A LEAD-BASED PAINT SYSTEM. THE CONTRACT FOR THE CONTAINMENT, PROPER MANAGEMENT, AND DISPOSAL OF ALL LEAD-CONTAMINATED HAZARDOUS WASTE GENERA PROCESS OF THE BRIDGE PROJECT. THE CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTING APPROPRIATE OSHA MAN PROTECTION STANDARDS RELATED TO THIS PROCESS.	ATED BY THE	.64	Boom Supported Floating Silt Fence		LF	116
	2. CLEAR PROTECTIVE COATING FOR CONCRETE SURFACES SHALL BE APPLIED TO THE FOLLOWING AREAS: - EXPOSED SURFACES OF THE PARAPETS, AND ENDPOSTS; - VERTICAL FACES OF THE DECK FASCIA EXTENDING BENEATH THE DECK TO THE GIRDER TOP FLANGE.						
	O. PIGMENTED PROTECTIVE COATING FOR CONCRETE SURFACES SHALL BE APPLIED TO EXPOSED SURFACES OF THE VINGWALLS AND PIERS.	ABUTMENTS,					
÷≝	N.WHERE DRILLING AND ANCHORING OF REINFORCING STEEL IS SPECIFIED THE CONTRACTOR SHALL USE A MATERIAL MAINEDDT PREQUALIFIED LIST OF CHEMICAL ANCHORING MATERIALS.THE DEPTH OF EMBEDMENT SHALL BE SUFFICIEN OF THE YIELD STRENGTH OF THE BAR,BUT SHALL BE NO LESS THAN THE MINIMUM DEPTH OF EMBEDMENT WHEN SF	NT TO DEVELOP 125%					
and Qua	cale: Designed by:						
		HNTB CORPORATION		MAINE TURNPIKE THE GO	חור	ST/	2R
	lo. Revision By Date 3	340 County Road, Suite					
037_		Westbrook, ME 0409 TEL (207) 774-5155			۱L HI	GΗ	VVAY
	CONSULTANT PROJECT MANAGER: Craig R. Morin, P.E.	FAX (207) 228-090	19	$ \setminus \mathbf{X} /$			
щĽ	By Date By Date						
l≣	Designed HJW 07\18 Checked KEB 07\18 Drawn ERB 07\18 In Charge of RAL 07\18						
ΨL	Drawn ERB 07\18 In Charge of RAL 07\18			MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.			

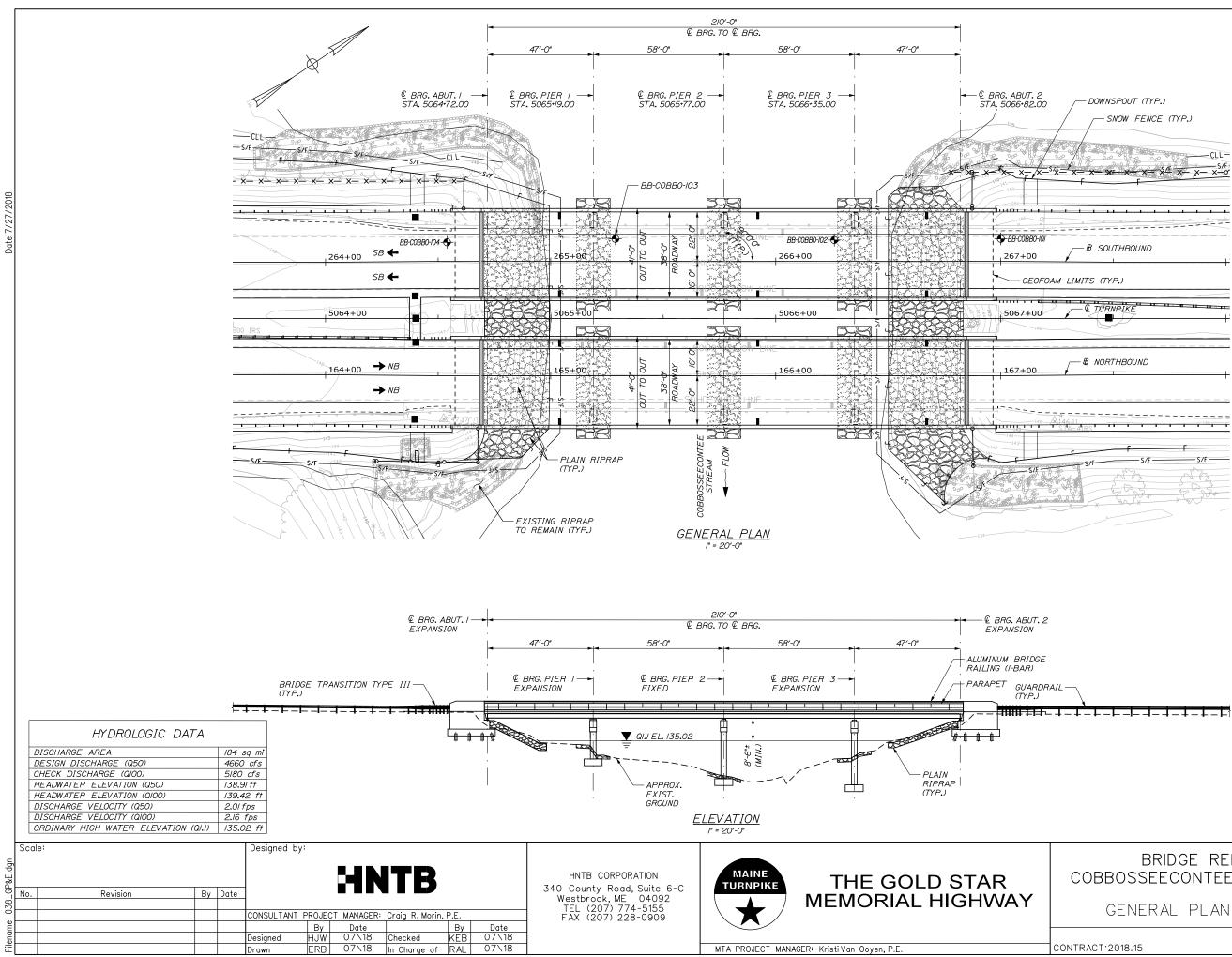


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BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

GENERAL NOTES, INDEX, AND QUANTITIES

CONTRACT:2018.15



SHEET NUMBER: S-02 38 OF 115

GENERAL PLAN AND ELEVATION

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

<u>NOTE:</u> SEE HIGHWAY PLAN SHEETS FOR FULL PROJECT LIMITS.

			CHONEWALE		PROJ
			NGINEERING		
			SSOCIATES, I		LOCA
Drille			lew England		ntractors
	rator:		chaefer / Tit	us	
	ged By:		chonewald		
	Start/Fi	~	1/8/17; 0825 67+00; 10 ft L1	- 11/9/17;	noon
Bori	ng Loca	tion: a	butment)	(ob mainin	e, nonneny
D = Sp MC = U U = Th MU = U V = Ins	lit Spoon S Jnsuccess in Wall Tul Jnsuccess itu Vane S	ful Split Spoo be Sample ful Thin Wall hear Test	on Sample atten Tube Sample a <u>e Shear Test at</u>	npt ttempt tempt	ADDITIONA N-uncorrect N ₆₀ = N va hammer eft S _u = Insitu R = Rock C RQD = Roc
		~		Sample In	tormation
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear	or RQD (%)
0					
	1D	24/14	2.0 - 4.0	20-24	-18-24
- 5 -					
	20	24/14	5.0 - 7.0	11-14	-11-10
10 -	3D	24/8	10.0 - 12.0	7-9-	9-12
					-
- 15 -	4D	24/11	15.0 - 17.0	21-18	-25-24

priller:		INGINEERING			CT:	Main	e Turi	npike I	MM9	9.2	Boring No.:	BB-COBBC	D-101	
viller:			ar.			Cobb	osse	econte	e Bri	idges Rehabilitation	Proj. No.:	17-03	5	
		Associates, ^I New England		LOCATI ntractors		VVest vation			/iaine		Core Barrel: N	IQ2		
perator:		Schaefer / Titu				tum:	()		. (-,		tandard split spoo	on	
ogged By:	;	Schonewald			Rig	Type:		Mobil	e Drill	B-51	Hammer Wt./Fall: 14	40 lbs/30 inches		
ate Start/F		11/8/17; 0825 267+00; 10 ft LT	- 11/9/17;	noon		lling M				n boring		ope & cathead		
loring Loca	ation:	abutment)	(op mainir	le, nonmeny		sing ID				4 ft; NW(3") to 45.5 ft	Hammer Efficiency:	0.6		
-SITU SAMPL	ING AND T	ESTING:		ADDITIONAL D	EFINIT	ger ID/0 10NS:	JD:	SSA	(4.5") 1 ADDI	TIONAL DEFINITIONS:	Water Level*: LABORATORY TEST RE	SULTS:		
= Thin Wall Tu U = Unsuccest = Insitu Vane \$	sful Split Spo Ibe Sample sful Thin Wa Shear Test	oon Sample attem II Tube Sample at <u>ine Shear Test att</u>	tempt empt	N-uncorrected N ₆₀ = N value hammer efficit S _U = Insitu Fie R = Rock Corr RQD = Rock C	correct ancy = c ild Vane a Sampl	ted for ha calculated s Shear S le	hammer trength (p	efficiency	BOF SSA	H = weight of 140lb. hammer R = weight of rods not recorded REHOLE ADVANCEMENT METH VHSA=solid/hollow stem auger roller cone/OPEN/PUSH=hydraul	LL=Liquid Limit / PL=P	WC = water conte idation test ndrained triaxial test Plastic Limit / PI=Plasti	icity Index	
Depth (ft.) Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	(i	or ROD (%)	N-uncorrected	09-N	Casing Blows	Elevation (ft.)	Graphic Log	Visual D	escription and Remarks		Lab. Testing Results	
0							SSA			HMA				
1D	24/14	2.0 - 4.0	20-24	-18-24	42	42		146.2		1D: Tan, damp, dense, f Gravel, trace Silt. GRAN	ine to coarse SAND, litte IULAR FILL	0.8-		
5 20	24/14	5.0 - 7.0	11-16	1-11-10	25	25	СШВ		61 	2D: Brown-tan, damp, m	. dense, Gravelly fine to	coarse SAND,		
	2414									trace Silt.	ice Silt.			
30	24/8	10.0 - 12.0	7-9-	9-12	18	18	15 27			3D: Brown, m. dense, G Silt.	ravelly fine to coarse SA	ND, trace to little		
							70 90							
5							90			Grey-brown, dense, fine	to coarse Sandy GRAV	El trace to little	AASHTO	
4D	24/11	15.0 - 17.0	21-18	-25-24	43	43	45			Silt; changing at 16.5 fee	et to		CORROSIVI SERIES	
							50	130.5		4D: Olive-brown and gre trace to little very fine Sa	y, mottled and desiccate	ed SILT & CLAY,		
	-						77				, appeare remained.			
							72	1						
		7]		52	407						
50 50	24/20	20.0 - 22.0	3-5	-6-5	11	11	58	127.0		5D: Olive brown, mottled with zones of SILT & CL	I, stiff, SILT, little to som	e very fine Sand		
							68	1		SILTS AND SANDS	, , , appeara unuaturbe.			
							74							
							72	1						
							72	1						
25 temarks:		approximate bou					gradual			e to conditions other than those	Page 1 of 3 Boring No.:			

			Engineering Associates, In				obosse st Garc			dges Rehabilitation Proj. No.: 17-03	5
Drille	r:			Boring Contractors		levatio		147 ft.			
per	ator:	:	Schaefer / Titu	S	0	Datum:				Sampler: standard split spor	on
	ed By:		Schonewald			Rig Typ		Mobile			
_	Start/Fi		11/8/17; 0825	 11/9/17; noon (SB mainline, northerly) 			Method:			boring Hammer Type: rope & cathead	
ori	ig Loca	tion:	abutment)	(ob manine, normenj	`	asing			,	4 ft; NW(3") to 45.5 ft Hammer Efficiency: 0.6 o 5 ft Water Level*:	
-SIT	J SAMPLI	NG AND T	ESTING:	ADDITIONA	L DEFI		00D:	SSA (ADDI	IONAL DEFINITIONS: LABORATORY TEST RESULTS:	
) = L = Thi J = L = Ins	n Wall Tut Insuccessi itu Vane S	ul Split Spo e Sample ul Thin Wa hear Test	oon Sample attemp Il Tube Sample att ine Shear Test atte	empt S _u = Insit. R = Rock (empt RQD = Ro	alue con fficiency J Field V Core Sa Ick Quali	rected for = calcula ane Shea mple	hammer eff led hammer r Strength (j ation (%)	iciency efficiency osf)	WOR = 1 BOR	I = weight of 140lb. hammer AASHTO / USCS soid classifications to weight of 160lb. hammer of recorded CONSUL - 1-D consolidation test ENCLE ADVINCEMENT METHODS: UL-IU-toronsolidated undrained trainal test NSA-solid/hollow stem auger Li_Liquid Limit / PL-Pasts Limit / PIP- dire consc/DEVPM/USH-indralul point UCT or a peak corrests/ws stempts of roc	
		~		Sample Informatio		-					
Depth (ft.)	Sample No.	Pen./Rec. (in.	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (psf) or RQD (%)	N-uncorrected	09-N	Casing Blows	Elevation (ft.)	Graphic Log	Visual Description and Remarks	Lab. Testing Results
25	6D	24/24	25.0 - 27.0	WOH/12*-2/12*	1	1	50			6D: Olive brown, mottled SILT & CLAY with pockets and partings Silty fine Sand; changing at 26.5 ft to	
					1		60	120.5	23 A		
					+	+	60		(J)	Dark grey, CLAY & SILT, trace very fine Sand. MARINE SILT- CLAY	
					-	_			Í.		
							68		1)		
30 -							75		H		
	7P	24/24	30.0 - 32.0 30.6 - 31.0	VANE INTERVAL Su=481 / 69 psf	-		52		H	7D: Dark grey, Silty CLAY, trace very fine Sand. V1: Tu=17.5/Tr=2.5 ft-lbs (65 mm x 130 mm vane)	CL WC=42%
	V2		31.6 - 32.0	Su=440 / 55 psf			58		H		LL=40 PL=20
					+	-	52		Ì	V2: Tu=16/Tr=2 ft-lbs (65 mm x 130 mm vane)	<u>PI=20</u>
					-	-					
					_	_	57		19		CL
35					1		59			8D: Dark grey, Silty CLAY, trace very fine Sand.	
	8D V3	24/24	35.0 - 37.0 35.6 - 36.0	VANE INTERVAL Su=481 / 55 psf			58			V3: Tu=17.5/Tr=2 ft-lbs (65 mm x 130 mm vane)	WC=36%
	V4		36.6 - 37.0	Su=494 / 69 psf			54		H	V4: Tu=18/Tr=2.5 ft-lbs (65 mm x 130 mm vane)	LL=35 PL=17 <u>PI=18</u>
							59		H		
							65		H		
					+	+		107.5	B		
0 -					_	_	64	107.5		39.5 ft: Drilling behavior suggests stratum charge. 9D: Dark grey, loose, fine to medium SAND, little Silt, trace	1
	9D	24/12	40.0 - 42.0	8-5-5-9	10	10	58		0.	oarse Sand, with gravely seam at top of sample. TILL	
							52				
							60				
					1	1	63				
					+	+	+	103.0	1100		1
15 -					+	_	-	102.2		44.8 ft: HW casing bounces; spin NW casing to 45.5 ft.	1
	R1	60/55	45.5 - 50.5	RQD: 24" = 40%	1	_				R1: Hard, typically fresh, fine to medium grained, light brown and light and medium grey, METASANDSTONE with abundant mica	
										(biotite and phlogopite) and quartzite and calcsilicate veins. Close	
					1					to moderately spaced, typically high angle breaks; undulating, rough, fresh to slightly weathered, and open, with occasional	
					1					sand infilling. Highly fractured from 45.5 to 46.7 ft. Core times:/ 1:40/1:45/1:50/1:45 min:sec/ft POOR ROCK QUALITY	1
					1	+					UCT qp =
50	arks:										11.85 ksi

ıdgı												
Js 1	Scal	le:		Designed by:								
39_Boring Log	No.	Revision	By Date			HN	ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155		THE GOLD STAR MEMORIAL HIGHWAY
0				CONSULTANT PRO	DJECT	MANAGER	Craig R. Morin,	P.E.		FAX (207) 228-0909		
me				E	By	Date		By	Date			
ena				Designed Hu	JW	07\18	Checked	KEB	07\18			
Ľ.				Drawn EF	RB	07\18	In Charge of	RAL	07\18		MTA PROJECT MANA	GER: KristiVan Ooyen, P.E.

		E	CHONEWALI NGINEERING ASSOCIATES, ^I		PROJ LOCAT		Cobb	ossee		e Br	idges Rehabilitation				
Drille	er:	1	Vew England	Boring Co			vation		147 ft.			Core Barrel:	NQ2		
Oper	ator:		Schaefer / Tit			Dat	tum:					Sampler:	standard split spoo	n	
	ed By:		Schonewald				j Type:		Mobile			Hammer Wt./Fall			
	Start/Finis		11/8/17; 0825 67+00; 10 ft L1	- 11/9/17;	noon		lling M				n boring	Hammer Type:	rope & cathead		
Borir	ng Locatio	n: _	ibutment)	(OD mainin	ie, normeny		sing ID ger ID/		HW(4 SSA (14 ft; NW(3") to 45.5 ft	Hammer Efficien Water Level*:	cy: 0.6		
IN-SIT	J SAMPLING	AND TE	ESTING:		ADDITIONAL	DEFINIT	IONS:	JD:	55A (TIONAL DEFINITIONS	LABORATORY TES	T RESULTS:		
D = Spi MD = U U = Thi MU = U V = Ins	lit Spoon Sam Insuccessful 8 In Wall Tube 9 Insuccessful 7 itu Vane Shea	ple Split Spo Sample Thin Wal r Test	on Sample atten I Tube Sample a <u>ne Shear Test at</u>	npt ttempt	N-uncorrect N ₆₀ = N val hammer effi S _u = Insitu F R = Rock Co RQD = Rock	ed = N va ue correc ciency = r field Vani ore Samp	due ted for ha calculated e Shear S de	mmer effi I hammer trength (p on (%)	ciency efficiency xsf)	WO WO = BO	H = weight of 140lb. hammer R = weight of rods not recorded REHOLE ADVANCEMENT METH VHSA=solid/hollow stem auger	AASHTO / USCS -#200 = percent fi CONSOL= 1-D co IODS: UU=Unconsolidat LL=Liquid Limit /	soil classifications nes WC = water conten assolidation test ed undrained triaxial test PL=Plastic Limit / PI=Plasti appressive strength of rock	rt (%) :ity Index	
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	ê	strengtri (psf) or RQD (%)	N-uncorrected	09-N	Casing Blows	Elevation (ft.)	Graphic Log	Visual D	escription and Rema	rks	Lab. Testing Results	
50									96.5				50.5		
55 -											Bottom of Exploration	n at 50.5 feet delow	ground surrace.		
60 -															
65 -															
70 -															
75 Rem	arks:		1				I				1				
			approximate bou							scur du	e to conditions other than those	Page 3 of 3			
prese	ent at the time	measur	ements were ma	ide.							e to conditions other than those	Boring N	o.: BB-COBB	J-101	

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

BORING LOGS I

CONTRACT:2018.15

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			Schonewale Engineering	PRC	JEC.	T: Mai Cob				9.2 idges Rehabilitation	Boring No.: _ Proj. No.:	BB-COBB 17-03		
			Associates, I	C. LOCA			st Gard				FT0J. NO	17-03	5	
Drill			-	Boring Contractors	_	Elevatio	1 (ft.)	129	t. (muc	lline, esťd)	Core Barrel:	NQ2		
	rator:		Schaefer / Titu	IS	_	Datum:					Sampler:	standard split spo	on	
	ged By:		Schonewald			Rig Type			e Drill		Hammer Wt./Fall:			
	Start/Fi			- 11/3/17; 1400 (SB mainline, south	_	Drilling I				n boring	Hammer Type:	rope & cathead		
Bori	ng Loca	tion:	Pier 3)	(SD mainine, south		Casing I				2 ft; NW(3") to 26.3 ft	Hammer Efficien	c y: 0.6		
D = Sp MC = l U = Th MI = I	in Wall Tub	ample ful Split Sp e Sample ful Thin W	oon Sample attem all Tube Sample at <u>ane Shear Test att</u>	tempt S _U = Insi R = Rock empt RQD = R	IAL DEF ected = 1 value co efficienc tu Field V core Sa cock Qua	N value rrected for I y = calculat Vane Shear	hammer eff ad hammer Strength (j	iciency efficienc	ADDI WOI WOI	hru deck TIONAL DEFINITIONS: H = weight of 140b, hammer R = weight of rods not recorded REHOLE ADVANCEMENT METHO VISA=solidhablow stem auger roller cone/OPEN/PUSH=hydraulic	Water Level*: LABORATORY TES' AASHTO / USCS #200 = percent fit CONSOL= 1-D co UU=Unconsolidat LU=Liquid Limit / F cpush_UCT qp = peak co	soil classifications nes WC = water conte	city Index	
	-	~		Sample Information			1	1	-					
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear Strength (psf) or RQD (%)	N-Incorrected	09-N	Casing Blows	Elevation (ft.)	Graphic Log		scription and Remain		Lab. Testing Results	
U	MD	24/0	0.0 - 2.0	WOR/24"	-	_				MD: MUCK (RECENT AL attempts with 3-inch dia.	LUVIUM); no recove spoon.	ery after two		
	1D	24/3	2.0 - 4.0	WOR-9-10-16			21			1D: Grey brown, m. dens Silt, trace Gravel. RECE	e, fine to coarse SA NT ALLUVIUM	ND, trace to little	HYDROMETI A-3(1) -#200=5.2%	
						+	30							
5 -	2D	24/4	5.0 - 7.0	1-2-7-4	9	9	3			Grey, Silty SAND; chang	ing at bottom of sam	ple to:		
							7	100		2D: Grey Silty CLAY, trac	o fine Sand			
	3D	24/19	7.0 - 9.0	1-WOH/12"-1	0	• PUSH		122.0		3D: Olive grey to grey, v. pockets and partings. MA	soft, Silty CLAY, tra	7.0- ce fine Sand as few		
10 -						12.0 ft: Unable to push ve 4D: Cilive grey, soft, Silly 1								
	4D	24/12	12.0 - 14.0	1-1-1-1	2	2	PUSH			concretion.	CEAT, take life of	nd with one	WC=41% LL=38 PL=19 <u>PI=19</u>	
15 -	V1 V2		14.6 - 15.0	Su=302 / 41 psf Su=398 / 165 psf	_					V1: Tu=11/Tr=1.5 ft-lbs (i				
	*4		10.0 - 10.0	59-565 / 105 p81			20	112.8		V2: Tu=14.5/Tr=6 ft-lbs (i on something on side of l 16.2 ft: Unable to hydraul	nole as rotate)			
	5D	24/11	17.0 - 19.0	13-18-17-19	35	5 35	58		مرد در در مرد در در در	stratum change. 5D: Grey, dense, fine to o				
20 -					+	+				19.6 ft: Casing refusal. 20 ft: Attempted to core broken rock	rock. Cobbles and/c	r weathered/ highly		
	R1	36/36	22.0 - 25.0	RQD: 18* = 50%	+			107.0		R1: Hard, typically fresh, medium grey and brown and calcsilicate veins; hig Close to moderately spac	grey, METASANDS h angle remnant str ed, moderately dipp	ONE with quartzite ucture visible. ing breaks;	UCT qp =	
25										undulating, rough, typical infilling. Highly broken fro	iy tresh, and open, v om 22.0 to 22.7 ft. C	ore times: 1:25/	6.37 ksi	
1. E 2. E	Bridge de	ck: 4" H	IMA over 7" co	147; mudline (groun norete; easily pene using at end of day	trated v	with solid	stem au	iger; fria	ble.	26.3 feet the following mor				
Ch-F'	ination lin		t approvimente 1	ndaries between soil typ	une tres	ritions	an arc to t				Page 1 of 2			
ottatif	reation lines	 represer 	approximate bou	iuaries between soil typ		swons may								

			Schonewali Engineering		PROJI	ECT:					9.2 idges Rehabilitation	Boring No.: BB-COB	
			Associates,		LOCAT	ION:						Proj. No.:17-0	35
Drille	er:		New England	Boring Co			evation				dline, est'd)	Core Barrel: NQ2	
	ator:		Schaefer / Tit	us		Da	itum:					Sampler: standard split s	poon
	ed By:		Schonewald				g Type:		Mobil			Hammer Wt./Fall: 140 lbs/30 inches	
Date	Start/Fi	nish:	11/2/17; 0945			Dr	illing M	ethod:	cased	i wasł	n boring	Hammer Type: rope & cathead	
Borir	ng Loca	tion:	266+26; 9.5 ft L Pier 3)	I (SB mainlii	ne, south of	Ca	ising ID	VOD:			22 ft; NW(3") to 26.3 ft	Hammer Efficiency: 0.6	
					ADDITIONAL		iger ID/	OD:	SSA(hru deck	Water Level*: LABORATORY TEST RESULTS:	
) = Spl /D = L /J = Thi /U = L /= Insi	in Wall Tub Insuccessi itu Vane S	ample ul Split Sp e Sample ul Thin W hear Test	oon Sample atten	npt ttempt	N-uncorrecte N ₆₀ = N valu hammer effic S _u = Insitu F R = Rock Co RQD = Rock	d = N w e corren iency = ield Van re Sam	alue cted for ha calculated ne Shear S ple	d hammer Strength (p	efficiency	WO WO = BOI SS4	TIONAL DEFINITIONS: H = weight of 140lb, hammer R = weight of rods not recorded REHOLE ADVANCEMENT METH- VHSA=solid/hollow stem auger roller cone/OPEN/PUSH=hydraul	AASHTO / USCS scil classifications -#200 = percent fines WC = water c CONSOL= 1-D consolidation test UU=Unconsolidated undrained triaxial to LL=Liauid Limit / PL=Plastic Limit / PL=F	est lasticity Index
					formation								
Depth (ft.)	Sample No.	Pen./Rec. (in.)	Sample Depth (ft.)	Blows (/6 in.) Shear	strengtri (psf) or RQD (%)	N-uncorrected	N-60	Casing Blows	Elevation (ft.)	Graphic Log	Visual D	Description and Remarks	Lab. Testing Results
25											1:30/ 1:15 min:sec/ft. P 25.0 to 26.3 ft: Fractures	OOR ROCK QUALITY d rock, See remark #3.	
	R2	60/56	26.3 - 31.3	RQD: 4	2* = 70%						R2: Same as R1 except	moderately spaced breaks. Core time	s:
											1:25/ 1:20/ 1:15/ 1: 20/ 1	1:30 min:sec/ft FAIR ROCK QUALITY	
30													
									97.7	5053) S	D.H	n at 31.3 feet below ground surface.	1.3
							-				Bottom of Exploratio	n at 31.3 feet below ground surface.	
							-						
35													
~													
			-				-						
							<u> </u>						
40													
ł			1				1						
							-						
45 -													
*" 1													
			-	<u> </u>			-						
							-						
50	arks:												
1. E 2. B	levation ridge de	ck: 4" ⊢	approximately IMA over 7" co	ncrete; eas	sily penetrat	ed wit	h solid s	stem aug	ger; frial	ble.			
3. H	lole cave	d when	pulled back c	asing at en	d of day to	open l	ane to ti	raffic. S	pun cas	ing to	26.3 feet the following mo	rning in order to resume coring.	
Stratifi	cation line:	represer	t approximate bou	indaries betwi	en soil types:	transitio	ins may be	e gradual.				Page 2 of 2	

Drille	er:	Ν
	ator:	S
	ed By:	S
	Start/Fing Loca	
IN-SIT D = Sp MD = U U = Th MU = U V = Ins MV = U	U SAMPLI lit Spoon S Insuccessi in Wall Tut Insuccessi itu Vane S Insuccessi	NG AND TE ample ful Split Spor be Sample ful Thin Wall hear Test ful Insitu Van
Depth (ft.)	Sample No.	Pen/Rec. (in.)
0	1D	42/6
5 -		
ĭ	2D	24/2
10 -		
	3D	12/6
	R1	60/60
15 -		
	R2	60/59
20 -		
25		
Rem	arks:	
1. E 2. B	levation ridge de	of deck a ck: 4" HN
Stratifi	cation line:	s represent a
' Wate	r level rea	dings have b ime measure
p. 00		

2.dgi														
S	Scal	e:			Designed by:									
40_Boring Loo	No.	Revision	Ву	Date			HN	TB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092		THE GOLD STAR MEMORIAL HIGHWAY	
Ó is					CONSULTANT PRO	JECT	MANAGER:	Craig R. Morin	, P.E.		TEL (207) 774-5155 FAX (207) 228-0909			
Ĕ					В	3y	Date		By	Date				\vdash
enc					Designed HJ			Checked	KEB	07\18				
Ē					Drawn ER	RB	07\18	In Charge of	RAL	07\18		MTA PROJECT MAN	IAGER: Kristi Van Ooyen, P.E.	C

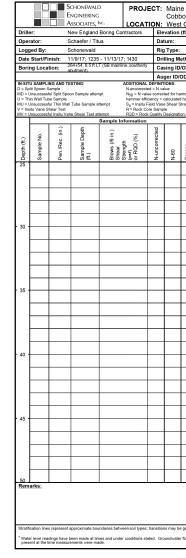
Finish:	CHONEWALE ENGINEERING ASSOCIATES, ^{II} New England Schaefer / Titu Schonewald 11/14/17; 072/ 285540; 10 ft LT 1)	NC. Boring Cor us 0 - 1545	PROJ LOCAT	ION: Ele Dat Rig	Cobb	ossee Gard	iner, I	e Bri Maine	dges Rehabilitation Proj. No.: 17-035
Finish:	Associates, ^{II} New England Schaefer / Titu Schonewald 11/14/17; 072	NC. Boring Cor us 0 - 1545		Ele Dat Rig	West vation tum:	Gard	iner, I	Maine	dudline, est'd) Core Barrel: NQ2
Finish:	New England Schaefer / Titu Schonewald 11/14/17; 072	Boring Cor us 0 - 1545		Ele Dat Rig	vation tum:				udline, est'd) Core Barrel: NQ2
Finish:	Schaefer / Titu Schonewald 11/14/17; 072	us 0 - 1545		Dat Rig	tum:				
Finish:	Schonewald 11/14/17; 072	0 - 1545		Rig					
Finish:	11/14/17; 072	0 - 1545 (SB mainlin					Mobi	e Drill	
ation:	265+40; 10 ft LT 1)	(SB mainlin		Dri	lling M	ethod:			boring Hammer Type: rope & cathead
LING AND TI	μ		ie, north of Pi		sing ID				1 ft; NW(3") to 12.7 ft Hammer Efficiency: 0.6
n Sample				_	ger ID/				nru deck Water Level*:
	ESTING:		ADDITIONAL	DEFINIT	IONS:			ADDI	TIONAL DEFINITIONS: LABORATORY TEST RESULTS:
ube Sample ssful Thin Wal Shear Test	oon Sample attem II Tube Sample at <u>ne Shear Test att</u>	ttempt	N-uncorrecte N ₆₀ = N valu hammer effic S _u = Insitu F R = Rock Cc RQD = Rock	ie correc ciency = i ield Vani ore Samp	ted for ha calculated e Shear S ile	hammer trength (p	efficiency	WO -= BOI SSA	1 = weight of 140b. hammer AASHTO / USCS soil dassifications 2 = weight of cost = 4.200 = percent fires //WC = water content (%) of recorded CENERT METHOD CONSC = 1-10 consolidation test ENCL ROVMERCENENT METHOD // Unconsolidated under thirsing test PENCE ROVMERCENENT METHOD // Unconsolidated under thirsing test conter conerCPENPUSH-thirditude push. UCT op = peak compressive strength of rock
Ê		~		p				1	
U.U.	Depth	.E	(%)	ecte				Log	Visual Description and Remarks Testing
Rei	ple	2) 2) 2)	ul dec	corr		e s	atio	hic	Results
en.	Bam Sam	Shea	psf) r R(un-	1-60	Casi 3lov	#:)	Srap	
42/6	0.0 - 3.5			0	0	woc		M	1D: Brown, Sandy ORGANIC SILT, trace to little Gravel. MUCK HYDROMET (RECENT ALLUVIUM) [3" dia split-spoon; let sink; fetches up at 3.5 th; dive to 4.0 @)
						woc		R.	3.5 ft; drive to 4.0 ft] -#200=6.15
						woc			
						-	119.0		3.5 ft: casing fetches up.
24/2	60.70	0.11	0.11	20	20				2D: Grey, Sandy GRAVEL, trace to little Silt. ⊺ILL
2402	3.0-1.0	0.11	-9-11	20	20				
						12			
						55			
						38			
12/6	10.0 - 11.0	7-7	0/6*	-		55			3D: Grey, Sandy GRAVEL, trace to little Silt.
						RC			11.0 ft: HW casing refusal on possible boulder; roller cone to 12.6 ft.
60/60	12.7 - 17.7	RQD: 42	2" = 70%				109.9		12.6 ft: Apparent top of bedrock; spin NW casing to 12.7 ft.
									R1: Hard, fresh, fine to medium grained, light grey with brownish bands METASANDSTONE with abundant mica (biotite and phlogopite) and numerous calcsilicate veins. Close to
									moderately spaced, moderately dipping breaks; undulating, rough, fresh to slightly discolored, and open. Highly broken with
+									weathered feldspars from 13. 7 to 14.4 ft. Core times: 3:20/ 2:25/ 2: 00/ 1:50/ 2:00 min:sec/ft. FAIR ROCK QUALITY
60/59	17.7 - 22.7	RQD: 43	3" = 72%						D0. Similar to D4 annual forms because the bands and more
									R2: Similar to R1, except fewer brownish bands and more abundant quartzite and calcsilicate veins. Typically moderately spaced and moderately dipping breaks. Schistosity developing.
									Core times: 2:30/ 1:55/ 2:00/ 2:20/ 2:00 min:sec/ft. FAIR ROCK QUALITY
+									
							99.8	0030	22.7- Bottom of Exploration at 22.7 feet below ground surface.
+									
1									
	² 92/434 4286 4286 4286 4286 242 242 126 00860 00860	By By By 428 0.0-35 428 0.0-35 428 0.0-35 428 0.0-37 428 0.0-37 242 5.0-70 243 244 244 244 244 244 245 244 244 244 </td <td>BC BC BC<</td> <td>By Carl Baseline Statistics 42:8 0.0.35 WOR42" 42:8 0.0.35 WOR42" - - - <td< td=""><td>age age age<td>By Used Observation Opposite Opposite</td><td>Break Break <th< td=""><td>No. No. No.</td></th<></td></td></td<><td>ng ng ng<</td></td>	BC BC<	By Carl Baseline Statistics 42:8 0.0.35 WOR42" 42:8 0.0.35 WOR42" - - - <td< td=""><td>age age age<td>By Used Observation Opposite Opposite</td><td>Break Break <th< td=""><td>No. No. No.</td></th<></td></td></td<> <td>ng ng ng<</td>	age age <td>By Used Observation Opposite Opposite</td> <td>Break Break <th< td=""><td>No. No. No.</td></th<></td>	By Used Observation Opposite Opposite	Break Break <th< td=""><td>No. No. No.</td></th<>	No. No.	ng ng<

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

BORING LOGS II

CONTRACT:2018.15

PROJECT: Maine Tumpike MM99.2 Cobbossecontee Bridges Rehabilitation SCHONEWALD ENGINEERING Boring No.: BB-COBBO-104 Proj. No.: 17-035 Associates, Mc. LOCATION: West Gardiner, Maine New England Boring Contractors Elevation (ft.) 148 ft. (est'd) Associates, INC Core Barrel: NQ2 Operator: Schaefer / Titus Datum: Sampler: standard split spoor Hammer Wt./Fall: 140 lbs/30 inches
 Operator.
 Schoneval
 Sampler.
 Rig Type: Mobile Drill B-51
 Numerical
 Construint
 Construi N-SITU SAMPLING AND TESTING: xial test RC=:oller cone/OPEN/PUSH=hydraulic push UCT gp = peak compressive strength of ro Sample Information (in.) epth 6 in.) N-uncorrected N-60 Casing Blows Elevation (ft.) Graphic Log Lab. Testing Results (%) Visual Description and Remarks Pen./Rec. Blows (/6 Shear Strength (psf) or RQD (% Sample (ft.)
 SSA
 147.2
 нма 1D: Tan, damp, m. dense, fine to medium SAND, trace Silt, trace Gravel, trace coarse Sand. GRANULAR FILL 1D 24/15 2.0 - 4.0 13-12-11-11 2D 24/15 5.0 - 7.0 3-3-4-4 13 13 25 58 3D 24/12 10.0 - 12.0 19-23-21-26 44 4 33 54 66 84 108 AASHTC FROSIV 17 4D 24/11 15.0 - 17.0 10-12-6-5 130.0 130.0 130.0 130.0 131.0 13 26 39 92 105 RC 5D 24/8 20.0 - 22.0 25-20-13-22 33 33 124.7 State 23.3 ft: HW casing refusal 23.3 ft: HW casing refusal 23.3 ft: Roller cone practicable refusal; spin NW casing to 23.5 ft: Roller cone practicable refusal; spin NW casing to 23.5 ft: 23.5 ft: Roller cone practicable refusal; spin NW casing to 23.5 ft: Roller cone practicable refusal; spin NW c 60/57 23.5 - 28.5 R1 RQD: 53" = 88% tification lines represent approximate boundaries between soil types; transitions may be gradual Page 1 of 2 Water level readings have been made at times and under conditions stated. Groundwater fluctuations may occur due to conditions other than those present at the time measurements were made. Boring No.: BB-COBBO-104



- <u>6</u> -													
s S	Scale	9:		Designed by:									
Ĭ													
ing							ITB			HNTB CORPORATION			THE GOLD STAR
Â	No.	Revision	By Date							340 County Road, Suite 6-C Westbrook, ME 04092	TORNPIK	AE	
4										TEL (207) 774-5155			MEMORIAL HIGHWAY
				CONSULTANT F	PROJEC	T MANAGER:	Craig R. Morin	н, Р.Е.		FAX (207) 228-0909			
Ĕ					By	Date		By	Date				
Bu				Designed	HJW	07\18	Checked	KEB	07\18				
Ē[Drawn	ERB	07\18	In Charge of	RAL	07\18		MTA PROJECT	T MANA	GER: KristiVan Ooyen, P.E.

e Turr	npike M	/M9	9.2	Во	ring No.:	BB-COBBC	D-104
			dges Rehabilitation		roj. No.:	17-035	
	iner, N					17-000	,
(ft.)	148 ft			Co	ore Barrel:	NQ2	
				Sa	mpler:	standard split spor	n
	Mobil	e Drill	B-51	На	mmer Wt./Fall:	140 lbs/30 inches	
thod:	cased	wash	boring	Ha	mmer Type:	rope & cathead	
OD:			3.3 ft; NW(3") to 23.5ft		mmer Efficiency		
DD:	SSA (4.5") 1	o 5 ft		ater Level*:		
		ADDI	TIONAL DEFINITIONS:		ABORATORY TEST	RESULTS:	
mmer effi	ciency	WOR	H = weight of 140lb. hammer R = weight of rods		AASHTO / USCS se -#200 = percent fine	is WC = water conter	nt (%)
hammer trength (p	efficiency		not recorded	IODS-	CONSOL= 1-D conr	solidation test I undrained triaxial test	
		SSA	/HSA=solid/hollow stem auger roller cone/OPEN/PUSH=hydraul		LL=Liquid Limit / PL	=Plastic Limit / PI=Plasti pressive strength of rock	city Index
an (%)		- RC-	Ioner cone/or Envroism=nydradi	ic pasi	r och gp - peak con	pressive strength of rock	
	-	DG _	Vieual D	escrit	otion and Remark		Lab. Testing
۶°	atio	hic	Vidual D	cacin	cion and reeman	.0	Results
Casing Blows	Elevation (ft.)	Graphic Log					
Ощ	ш€	0	R1: V. hard, fresh, medi	um to	coarse grained	light grev	LICT on T
		部合	PEGMATITE, with garne	ets vis	lible from 28.1 to	28.5 ft. Moderately	UCT qp = 17.67 ksi
	1	National States	spaced, moderately dipp discolored and open. C	oing b	reaks; undulating	, rough, slightly	
		144	min:sec/ft. GOOD TO E	XCE	LENT ROCK QU	JALITY	
	119.5						
	119.5		Bottom of Exploratio	n at 2	8.5 feet below g	round surface.	
	1						
	1						
	1						
	1						
	1						
	1						
gradual.				_	Page 2 of 2		
-					-		
nuctuatio	ons may or	cur due	to conditions other than those		Boring No	.: BB-COBB	D-104

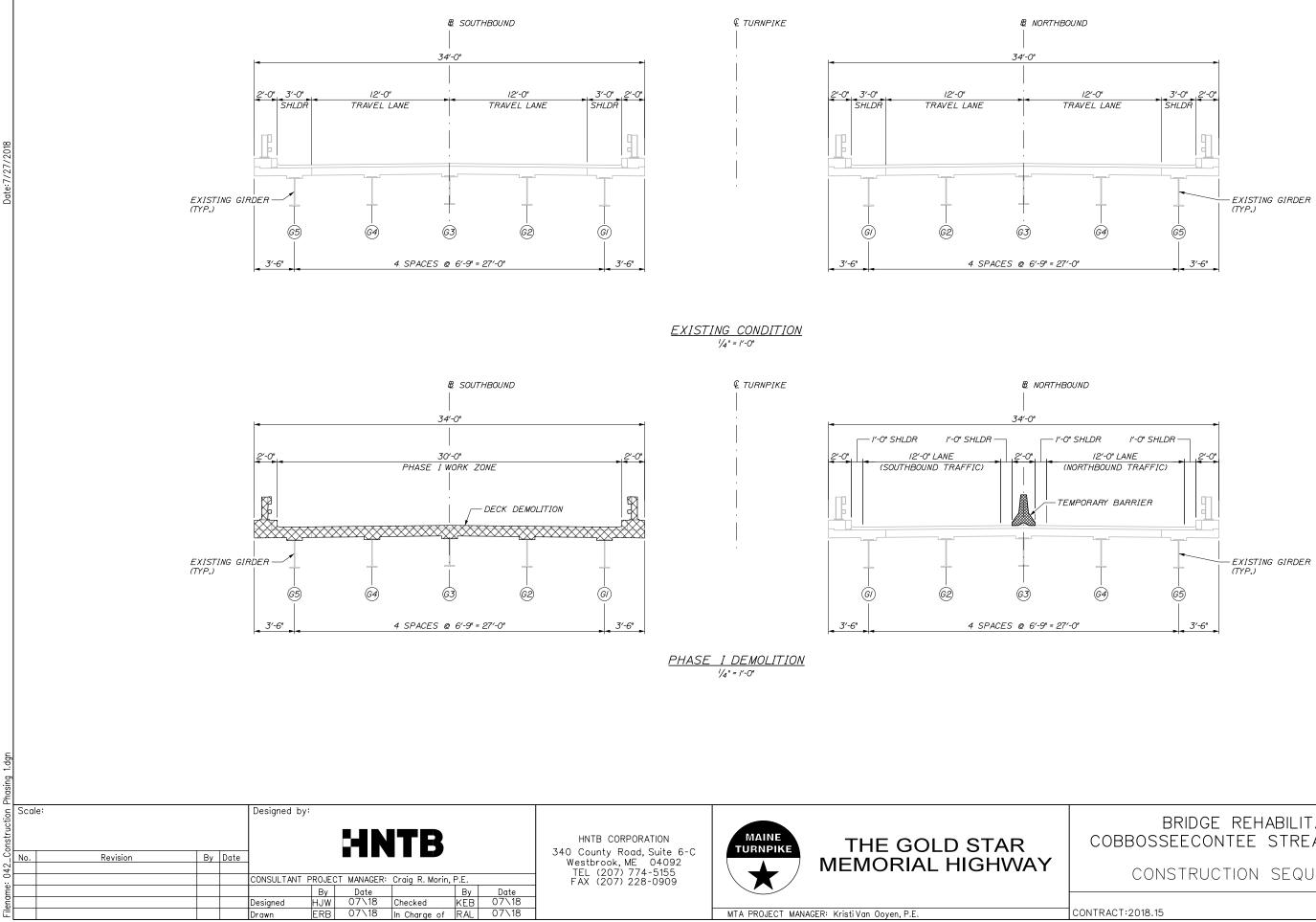
BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

BORING LOGS III

CONTRACT:2018.15

SHEET NUMBER: S-05

410F 115



Drawn

MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

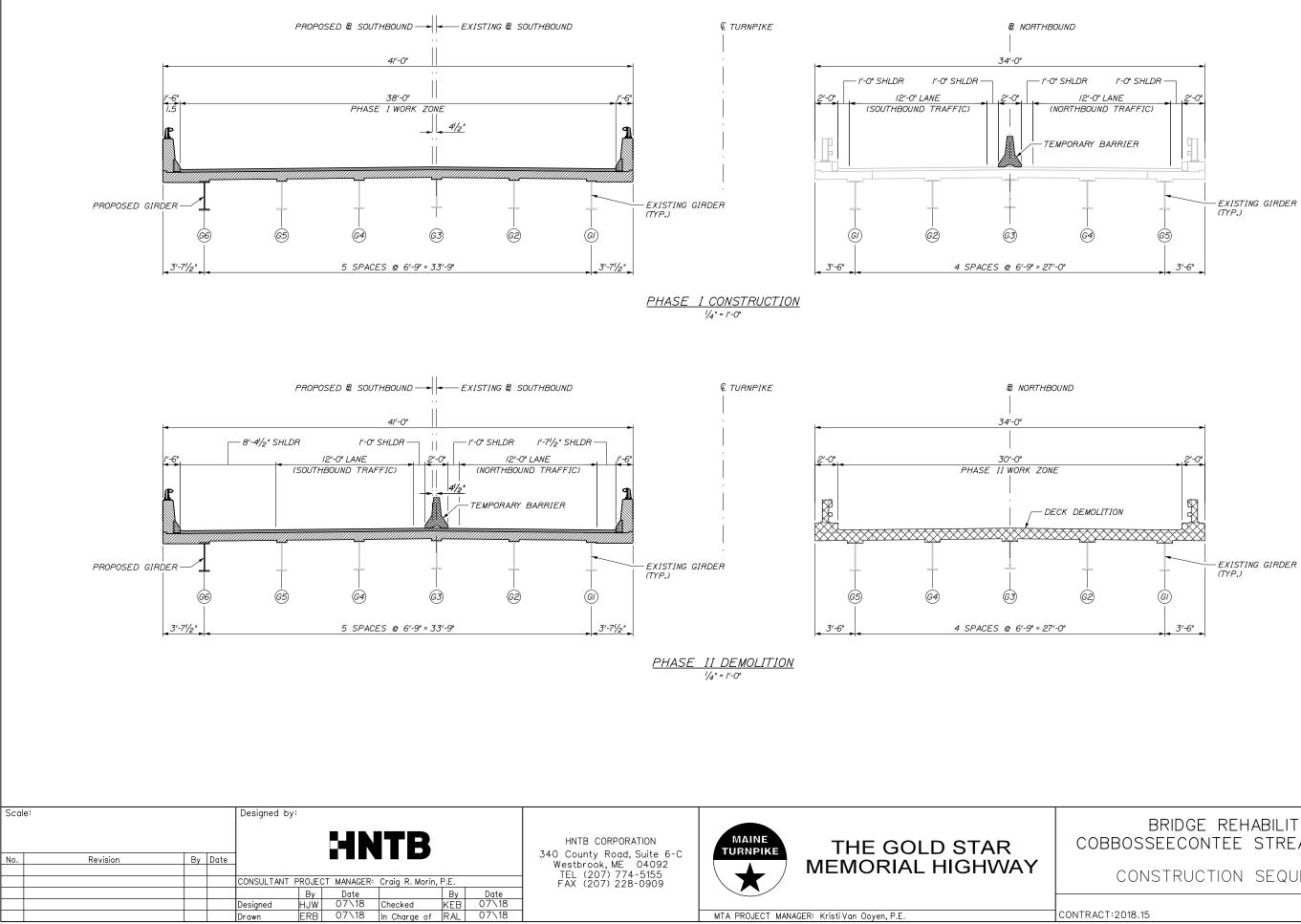
BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

CONTRACT:2018.15

SHEET NUMBER: S-06

42 OF 115

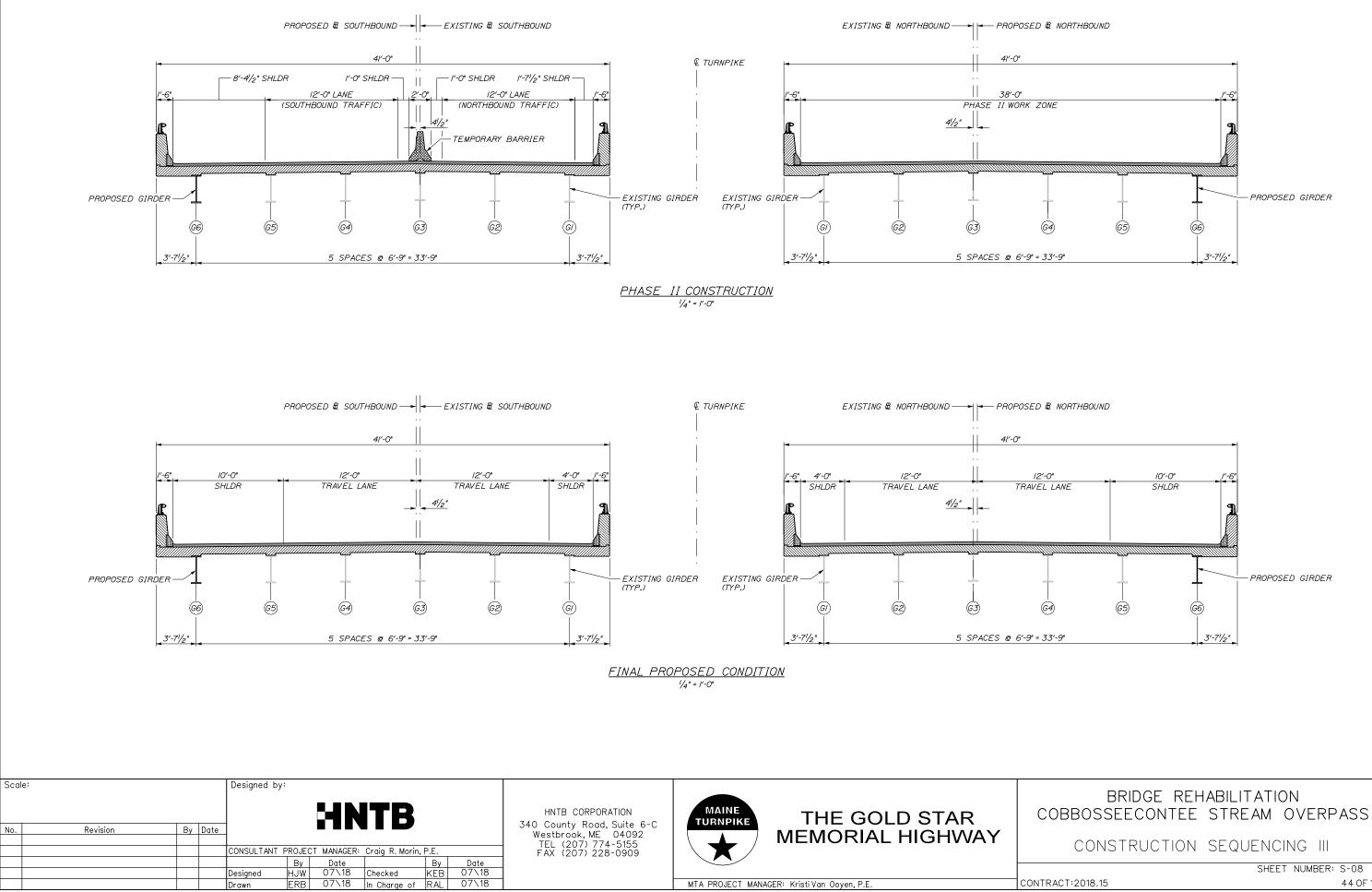
CONSTRUCTION SEQUENCING I



BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

CONSTRUCTION SEQUENCING II

CONTRACT:2018.15



ABUTMENT NOTES:

I. ABUTMENT REPAIR WORK SHALL INCLUDE: ABUTMENT SURFACE REPAIRS, EPOXY INJECTION CRACK REPAIR AND SEALING EXISTING CONSTRUCTION JOINTS. ALL ABUTMENT REPAIR AREAS ARE ABOVE THE QLI ELEVATION.

2. WHERE ABUTMENT SURFACE REPAIRS ARE SPECIFIED THE WORK SHALL INCLUDE REMOVAL OF UNSOUND CONCRETE AND PLACEMENT AND CURING OF REPAIR MATERIALS. REPAIRS SHALL BE MEASURED FOR PAYMENT UNDER PAY ITEM 5/8.60, "REPAIR OF VERTICAL SURFACES <8 INCHES" AND 5/8.51 "REPAIR OF UPWARD FACING SURFACES - BELOW REINFORCING STEEL < 8 INCHES".

3. WHERE EPOXY INJECTION CRACK REPAIR IS SPECIFIED THE WORK SHALL INCLUDE PRESSURE INJECTION OF CRACKS 1/8" OR MORE IN WIDTH. WORK SHALL BE PAID UNDER PAY ITEM 518.40, "EPOXY INJECTION CRACK REPAIR".

4. FOR APPROXIMATE LOCATIONS OF ABUTMENT REPAIRS, SEE SUBSTRUCTURE REPAIRS SOUTHBOUND ABUTMENTS AND SUBSTRUCTURE REPAIRS NORTHBOUND ABUTMENTS SHEETS. ACTUAL REPAIR AREAS WILL BE DETERMINED BY THE RESIDENT DURING CONSTRUCTION.

5. ALL EXCAVATION REQUIRED TO COMPLETE THE ABUTMENT REPAIRS WILL NOT BE MEASURED FOR PAYMENT SEPARATELY, BUT SHALL BE INCIDENTAL TO PAY ITEM 518.60, "REPAIR OF VERTICAL SURFACES <8 INCHES".

6. THE ESTIMATED REPAIR QUANTITIES ARE BASED ON AN ANNUAL INSPECTION COMPLETED IN MARCH 2017 AND AN UNDERWATER INSPECTION IN MAY 2016.

PIER NOTES:

I. PIER REPAIR WORK SHALL INCLUDE: PROVIDING ACCESS FOR PIER INSPECTION, PIER SURFACE REPAIRS AND EPOXY INJECTION CRACK REPAIR.

2. WHERE PIER SURFACE REPAIRS ARE SPECIFIED THE WORK SHALL INCLUDE REMOVAL OF UNSOUND CONCRETE AND PLACEMENT AND CURING OF REPAIR MATERIALS. REPAIRS SHALL BE MEASURED FOR PAYMENT UNDER PAY ITEM 518.60, "REPAIR OF VERTICAL SURFACES <8 INCHES", AND PAY ITEM 518.601, "REPAIR OF VERTICAL SURFACES <8 INCHES - BELOW WATERLINE"

3. WHERE EPOXY INJECTION CRACK REPAIR IS SPECIFIED THE WORK SHALL INCLUDE PRESSURE INJECTION OF CRACKS 1/6" OR MORE IN WIDTH. WORK SHALL BE PAID UNDER PAY ITEM 518.40, "EPOXY INJECTION CRACK REPAIR" AND 518.401, "EPOXY INJECTION CRACK REPAIR - BELOW WATERLINE".

4. THE CONTRACTOR SHALL PROVIDE AN ACCEPTABLE MEANS OF ACCESS AT ALL PIER LOCATIONS THAT WILL ALLOW THE RESIDENT TO SAFELY PERFORM A DETAILED CONCRETE INSPECTION OF ALL PIER SURFACES LOCATED ABOVE THE WATERLINE. PROVIDING ACCESS AT PIER LOCATIONS SHALL BE COMPLETED IN ACCORDANCE WITH SPECIAL PROVISION 524, "TEMPORARY ACCESS PLATFORMS FOR PIER INSPECTION AND REPAIR".

5. THE QUANTITIES OF PIER REPAIR NOTED ON THESE DRAWINGS ARE BASED ON BOTH AN UNDERWATER INSPECTION AND A VISUAL INSPECTION. BELOW WATER REPAIRS ARE BASED ON THE WATER LEVEL AT THE TIME OF INSPECTION. IF DIFFERENCES IN UNDERWATER REPAIR QUANTITIES ARE EXPERIENCED THE RESIDENT SHOULD BE NOTIFIED.

6. FOR APPROXIMATE LOCATIONS OF REPAIR SEE SUBSTRUCTURE REPAIRS PIER I, SUBSTRUCTURE REPAIRS PIER 2, AND SUBSTRUCTURE REPAIRS PIER 3 SHEETS, ACTUAL REPAIR AREAS WILL BE DETERMINED BY THE RESIDENT DURING CONSTRUCTION. THE RESIDENT SHALL COORDINATE DIRECTLY WITH THE CONTRACTOR'S DIVE TEAMS TO AGREE ON UNDERWATER REPAIR LIMITS.

> PER SPECIAL PROVISION 518

(TYP.)



I. PRIOR TO THE START OF CONCRETE REPAIRS THE RESIDENT AND THE CONTRACTOR SHALL SOUND ALL ABUTMENT AND PIER SURFACES AND AGREE ON THE REPAIR LIMITS.

2. PERFORM "DEEP SAW CUTS ALONG LIMITS OF REMOVAL.

3. CHIP CONCRETE TO THE DEPTH SPECIFIED IN SPECIAL PROVISION 518. IF THE REMOVAL LIMITS CHANGE DURING THE DEMOLITION PROCESS THE CONTRACTOR SHALL NOTIFY THE RESIDENT. THE RESIDENT AND CONTRACTOR SHALL AGREE ON THE REVISED PAY LIMITS PRIOR TO THE CONTRACTOR CONTINUING THE REMOVALS.

4. PREPARE AND PATCH REPAIR AREAS. SEE SPECIFICATIONS FOR SURFACE PREPARATION, MATERIALS, PLACEMENT AND CURING REQUIREMENTS.

5. PERFORM GENERAL FINISHING.

EPOXY INJECTION CRACK REPAIR:

I. ALL CRACK REPAIRS SHALL BE COMPLETED IN ACCORDANCE WITH SPECIAL PROVISION 518.

2. THE SELECTED REPAIR MATERIAL FOR UNDERWATER CRACK REPAIR SHALL BE APPROVED BY THE MANUFACTURER FOR PLACEMENT UNDERWATER.

3. PREPARE CONCRETE SURFACE AND CRACK AREA BY CLEANING SUBSTRATE WITH WIRE BRUSH TO REMOVE LAITANCE AND CONTAMINANTS. BLAST CLEAN THE CRACK AND CONCRETE SURFACE; DO NOT USE WATER.

4. SET INJECTION PORTS ALONG THE LENGTH OF THE CRACK USING A HIGH-MODULUS, LOW VISCOSITY EPOXY SUCH AS SIKADUR 35, SIKADUR 55, OR APPROVED EQUAL ONCE SET SEAL PORTS AND CRACK WITH THE SAME EPOXY ADHESIVE. EPOXY SEAL AROUND PORTS SHALL BE ALLOWED TO CURE PRIOR TO CRACK INJECTION.

GENERAL FINISHING:

I. CONTRACTOR SHALL REMOVE GRAFFITI AND TECTYL COATING WHERE PRESENT PRIOR TO APPLYING PROTECTIVE CONCRETE COATING.WORK SHALL BE INCIDENTAL TO THE SPECIFIED ITEM IN SPECIAL PROVISION 518.

2. ALL EXPOSED SUBSTRUCTURE SURFACES SHALL BE COATED WITH A PROTECTIVE COATING SUITABLE FOR CONCRETE SURFACES AFTER PATCHING IS COMPLETE AND PATCH MATERIALS HAVE CURED IN ACCORDANCE WITH SPECIAL PROVISION 515.

DEMOLITION NOTES:

I. ALL LIMITS OF DEMOLITION SHALL BE SAWCUT I" DEEP.

2. FOR AREAS OF ABUTMENT AND PIER REPAIRS, SEE SUBSTRUCTURE REPAIRS SHEETS.

3. DIMENSIONS OF EXISTING ABUTMENT AND PIER ELEMENTS ARE BASED ON AS-BUILT INFORMATION AND LIMITED FIELD SURVEYS.

4. ALL EXPOSED SURFACES SHALL BE COATED WITH CLEAR PROTECTIVE COATING FOR CONCRETE SURFACES AFTER RECONSTRUCTION IS COMPLETE AND MATERIALS HAVE CURED.

5. REMOVE CONCRETE AND REINFORCEMENT AS NOTED WITHIN DEMOLITION LIMITS.

6. EXISTING REINFORCEMENT TO REMAIN AT ABUTMENTS OR PIERS SHALL BE SANDBLASTED. THE RESIDENT SHALL INSPECT THE CONDITION OF THE REINFORCEMENT AND DETERMINE WHETHER INDIVIDUAL BARS ARE DETERIORATED BEYOND REUSE. IF DETERIORATION IS DEEMED EXCESSIVE BY THE RESIDENT, A REINFORCEMENT REHABILITATION DETAIL WILL BE PROVIDED BY THE ENGINEER. PAYMENT SHALL BE MADE UNDER 503.14 AND 503.15.

Notes				
୍ର Sca	le:	Designed by:		
45_Substructu	Revision By Date	HNTB	HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155	THE GOLD STAR
0		CONSULTANT PROJECT MANAGER: Craig R. Morin, P.E.	FAX (207) 228-0909	
Ĕ		By Date By Date		
G		Designed HJW 07\18 Checked KEB 07\18		
Ē		Drawn ERB 07\18 In Charge of RAL 07\18		MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

LIMITS OF CONCRETE REPAIR TO CONSIST OF REMOVAL AND PLACEMENT. EXISTING REINFORCING STEEL TO

LIMIT OF MEASUREMENT

REMAIN. (TYP.)

-FACE OF CONCRETE

CONCRETE SURFACE REPAIR DETAIL

I" MIN. SAWCUT (TYP.)

CONTRACT:2018.15

45 OF 115

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS SUBSTRUCTURE REPAIRS CONCRETE REPAIR NOTES AND DETAILS SHEET NUMBER: S-09

REPAIR QUANTITIES

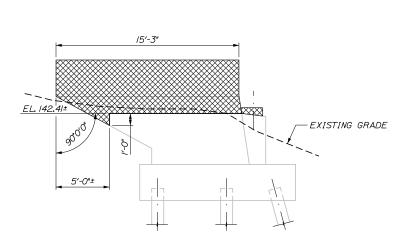
REPAIR OF UPWARD FACING SURFACES < 8 INCHES	9 S.F.*
REPAIR OF VERTICAL SURFACES < 8 INCHES	5 S.F.*
EPOXY INJECTION CRACK REPAIR	5 L.F.**
* INCLUDES 5 S.F. ADDITIONAL REPAIR QUANTITY AS A CC	DNTINGENCY.

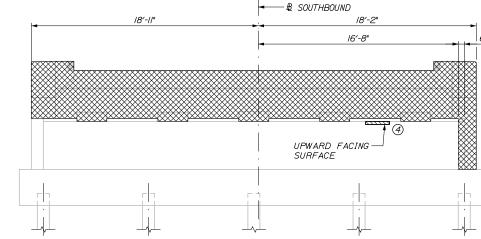
** ASSUMES 5 L.F. OF ADDITIONAL CRACK REPAIR

<u>LEGEND</u>

LIMIT OF DEMOLITION

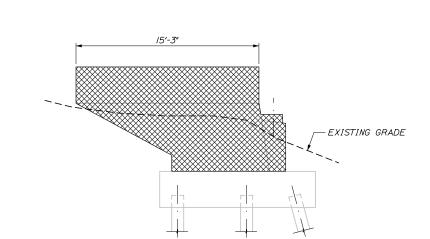
- LIMIT OF SURFACE PATCH REPAIR
- (#) SQUARE FOOT AREA OF REPAIR
- # LINEAR FOOT LENGTH OF CRACK
- EPOXY INJECTION CRACK REPAIR \sim



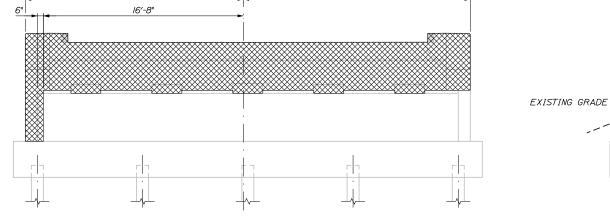


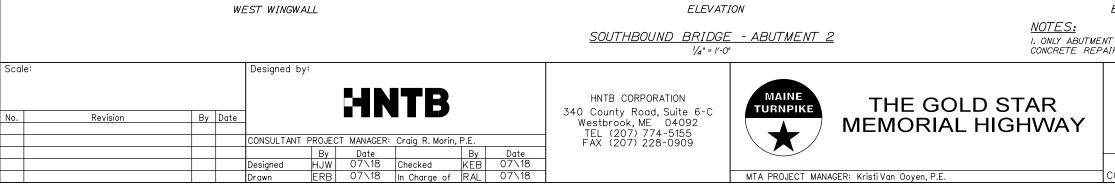
EAST WINGWALL

ELEVATION



SOUTHBOUND BRIDGE - ABUTMENT I 1/4" = 1'-0" —₿ SOUTHBOUND 18'-11" 18′-2"





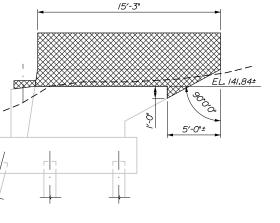
CONTRACT:2018.15

46 OF 115

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS SUBSTRUCTURE REPAIRS SOUTHBOUND ABUTMENTS SHEET NUMBER: S-10

. ONLY ABUTMENT REPAIRS BEYOND THE FULL DEMOLITION LIMITS WILL BE PAID UNDER CONCRETE REPAIR ITEMS NOTED ON THIS SHEET.

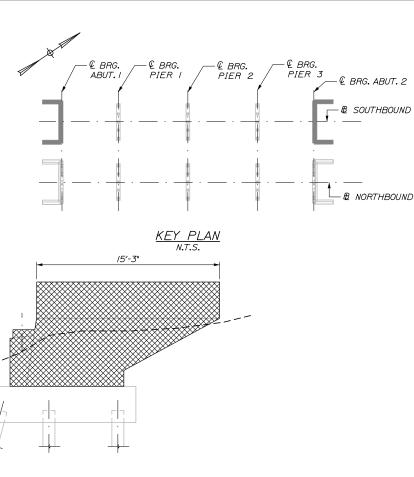
EAST WINGWALL





WEST WINGWALL

EXISTING GRADE



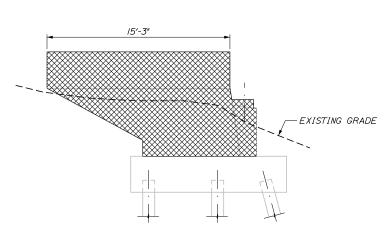
REPAIR QUANTITIES

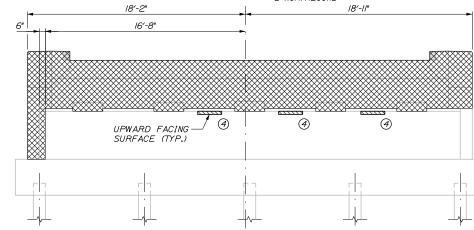
REPAIR OF UPWARD FACING SURFACES < 8 INCHES	15 S.F.*
REPAIR OF VERTICAL SURFACES < 8 INCHES	5 S.F.*
EPOXY INJECTION CRACK REPAIR	5 L.F.**
* INCLUDES 5 S.F. ADDITIONAL REPAIR QUANTITY AS A C	CONTINGENCY.

<u>LEGEND</u>

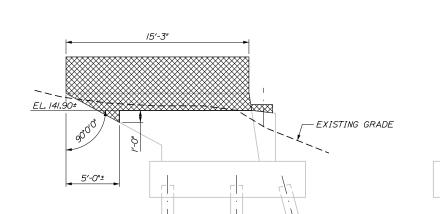
LIMIT OF DEMOLITION

No.



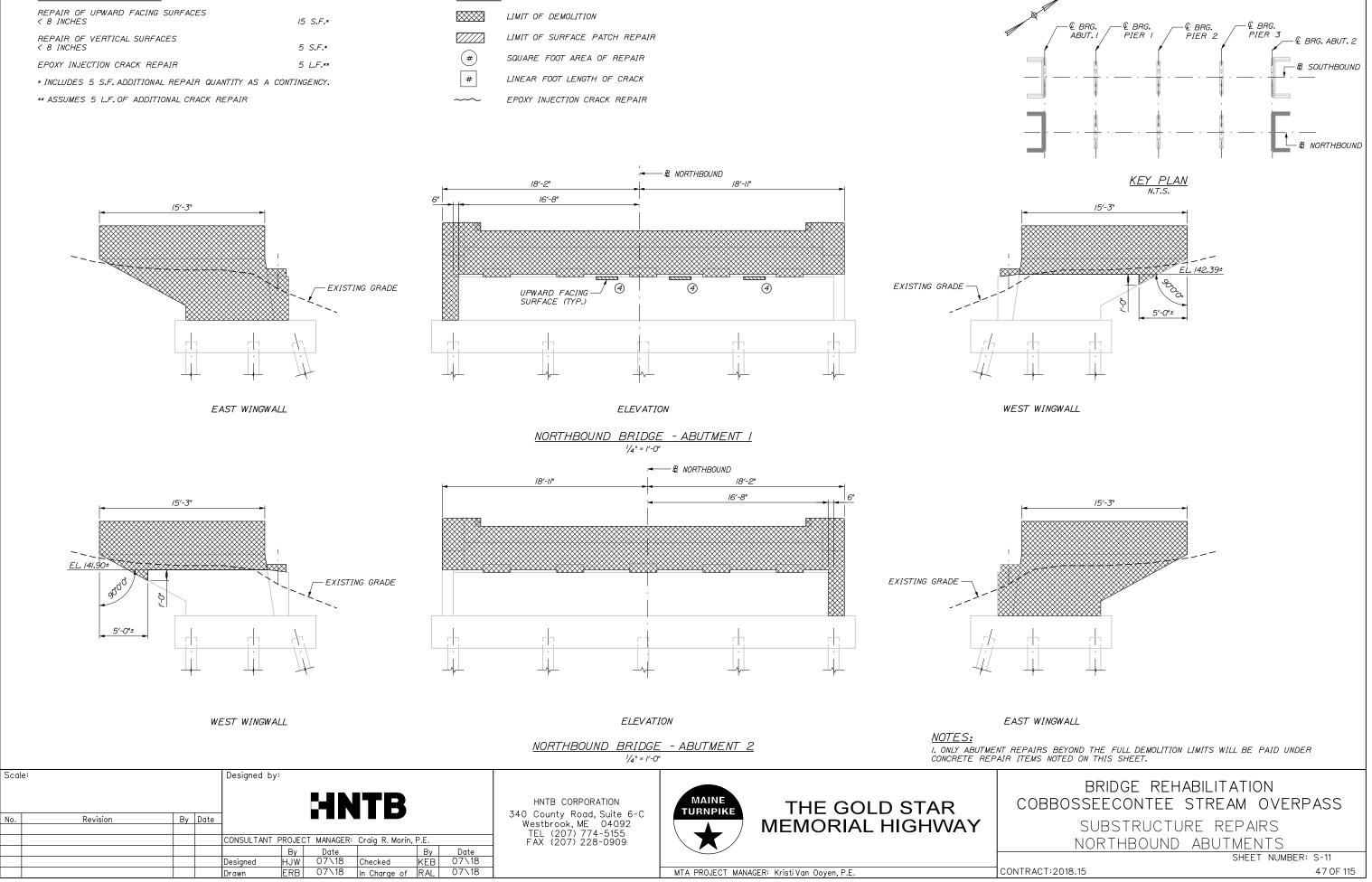


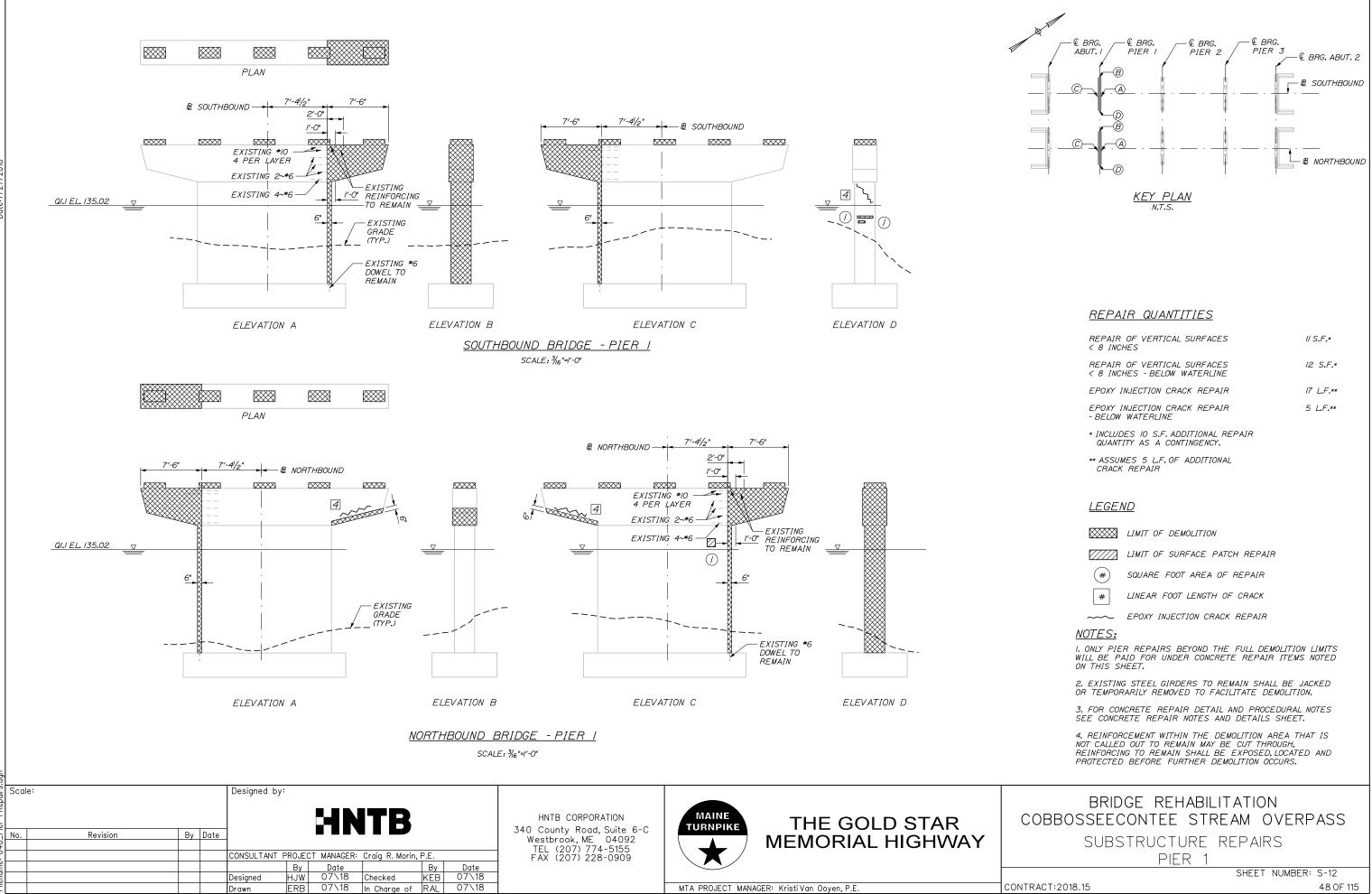




|8′-||" 18'-2" /6′-8"

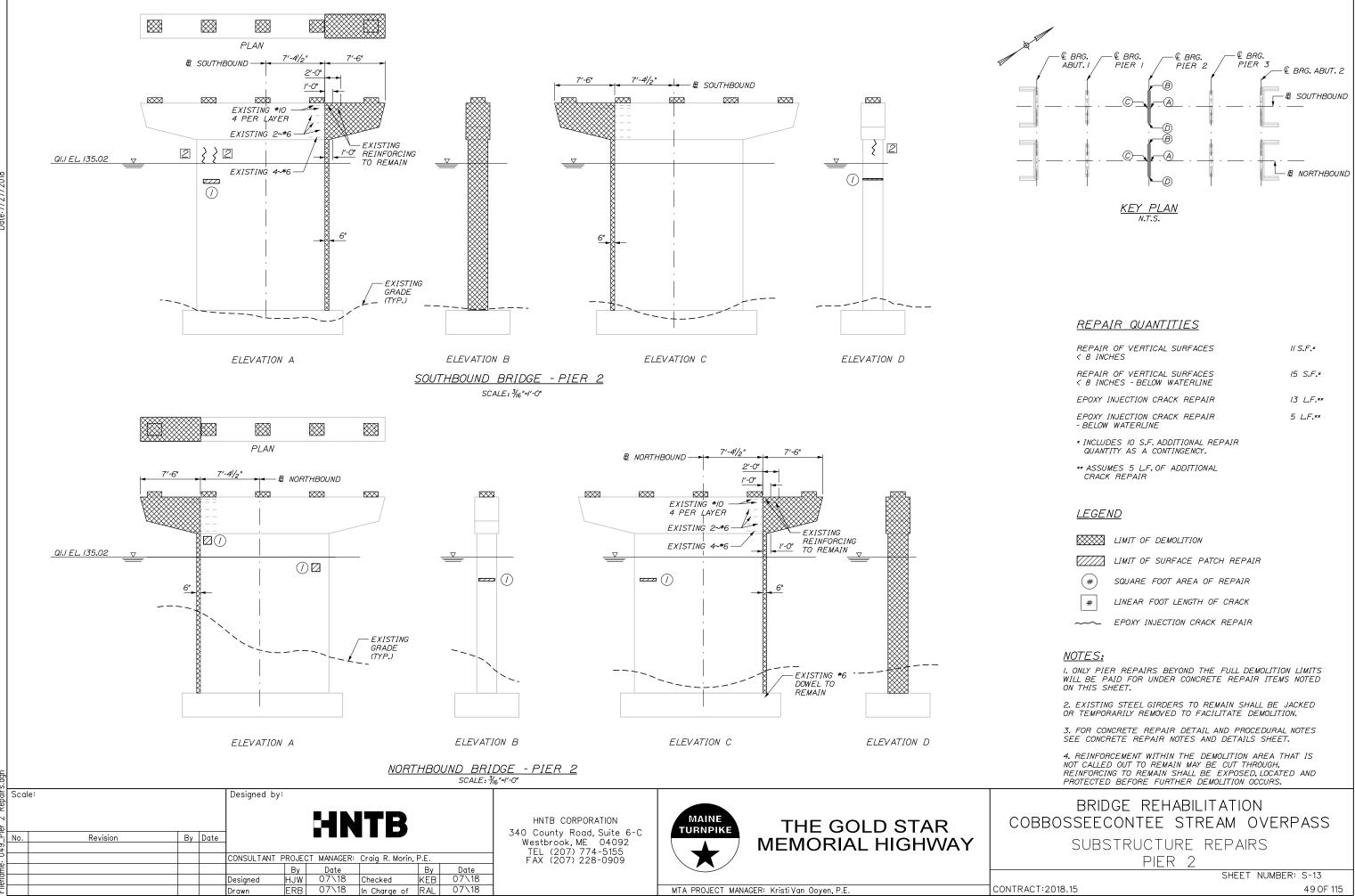






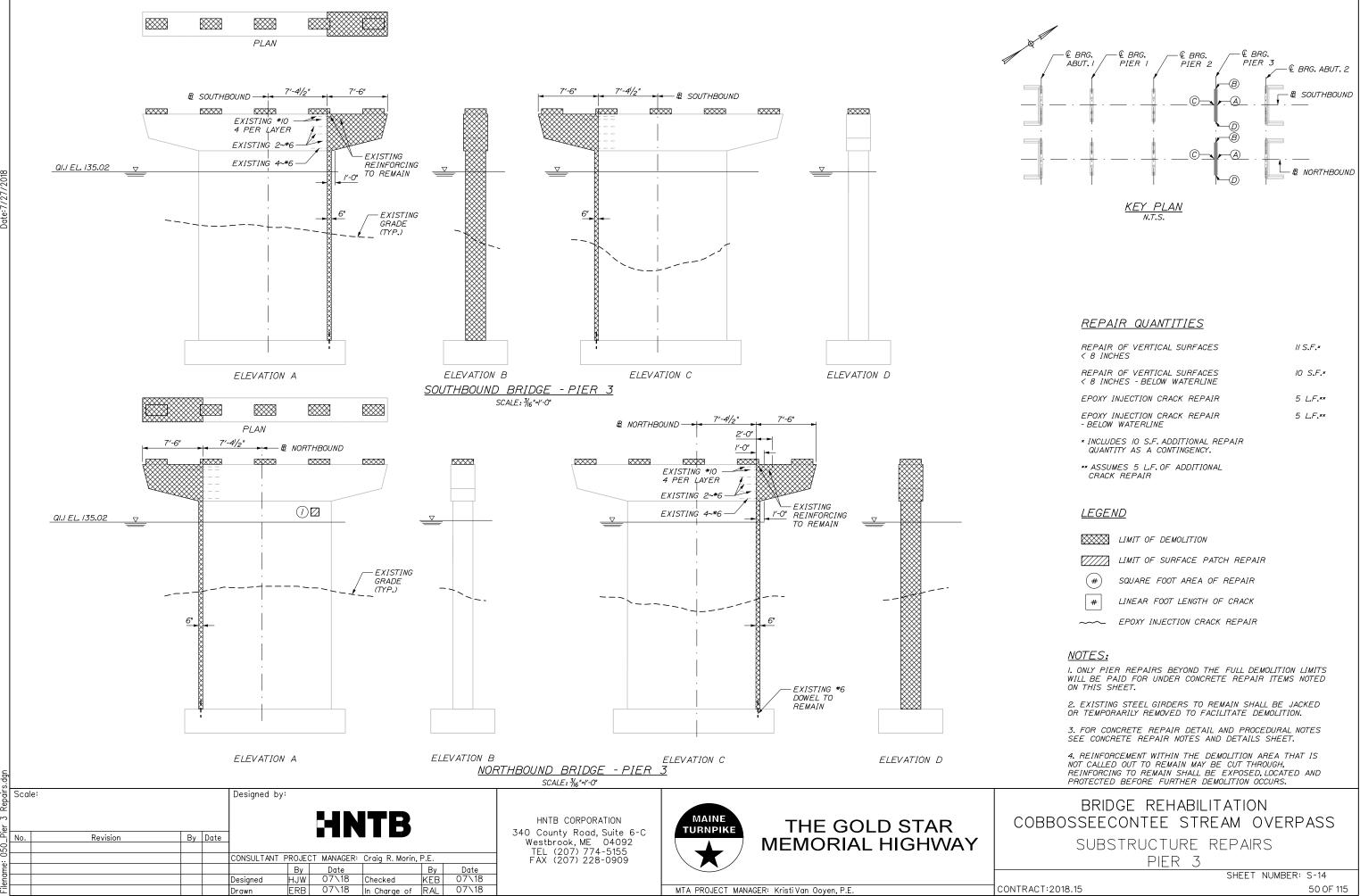
REPAIR OF VERTICAL SURFACES < 8 INCHES	// S.F.*
REPAIR OF VERTICAL SURFACES < 8 INCHES - BELOW WATERLINE	12 S.F.*
EPOXY INJECTION CRACK REPAIR	17 L.F.**
EPOXY INJECTION CRACK REPAIR - BELOW WATERLINE	5 L.F.**

\boxtimes	LIMIT	0F	DEMOLITION
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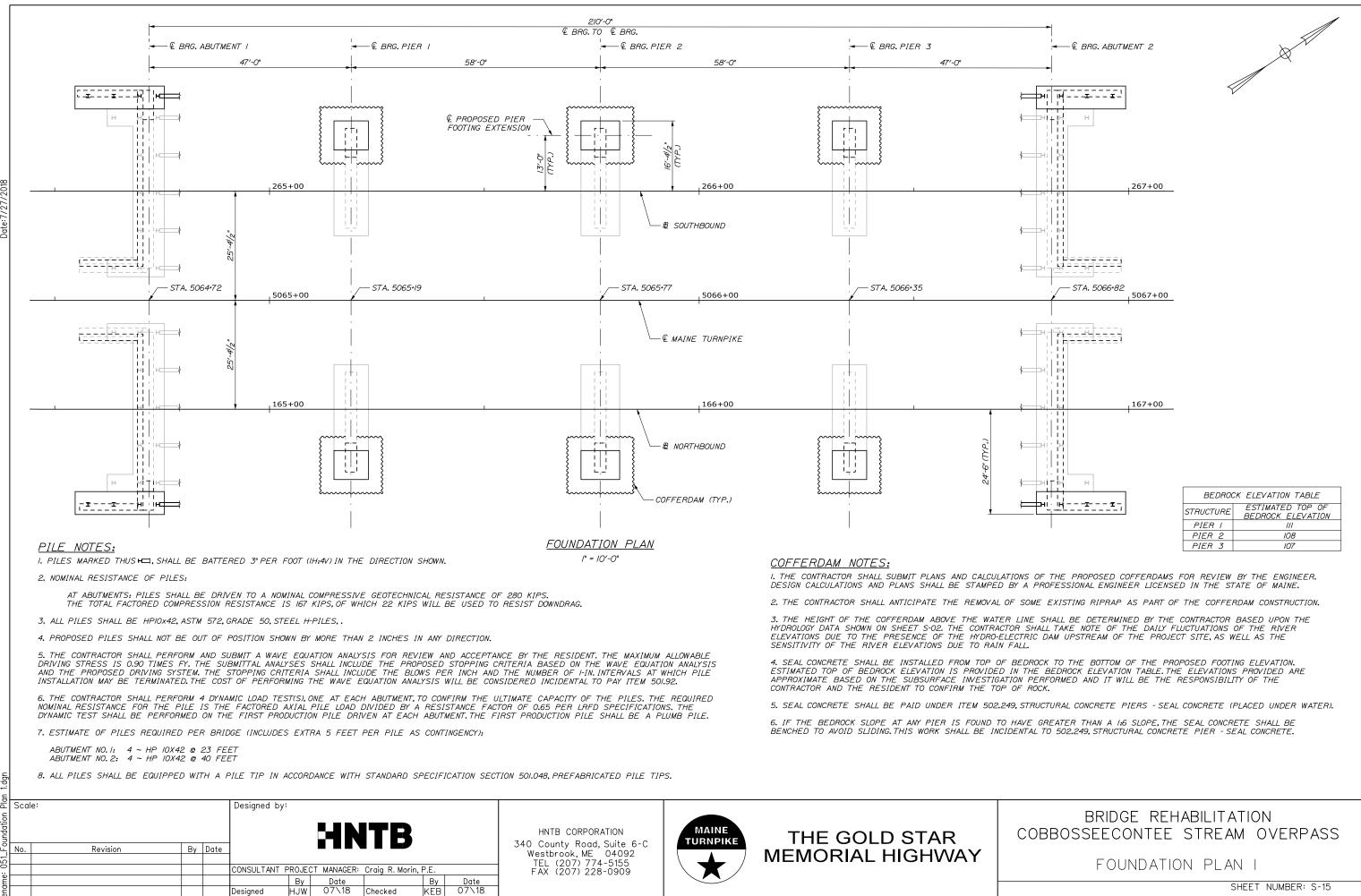
REPAIR OF VERTICAL SURFACES < 8 INCHES	S.F.*
REPAIR OF VERTICAL SURFACES < 8 INCHES - BELOW WATERLINE	15 S.F.*
EPOXY INJECTION CRACK REPAIR	13 L.F.**
EPOXY INJECTION CRACK REPAIR - BELOW WATERLINE	5 L.F.**

\boxtimes	LIMIT OF DEMOLITION
	LIMIT OF SURFACE PATCH REPAIR
#	SQUARE FOOT AREA OF REPAIR
#	LINEAR FOOT LENGTH OF CRACK
	EPOYY INJECTION CRACK REPAIR



REPAIR OF VERTICAL SURFACES < 8 INCHES	S.F.*
REPAIR OF VERTICAL SURFACES < 8 INCHES - BELOW WATERLINE	10 S.F.*
EPOXY INJECTION CRACK REPAIR	5 L.F.**
EPOXY INJECTION CRACK REPAIR - BELOW WATERLINE	5 L.F.**

\boxtimes	LIMIT OF DEMOLITION
	LIMIT OF SURFACE PATCH REPAIR
#	SQUARE FOOT AREA OF REPAIR
#	LINEAR FOOT LENGTH OF CRACK
~~~	EPOXY INJECTION CRACK REPAIR



Checked

07\18 In Charge of RAL 07\18

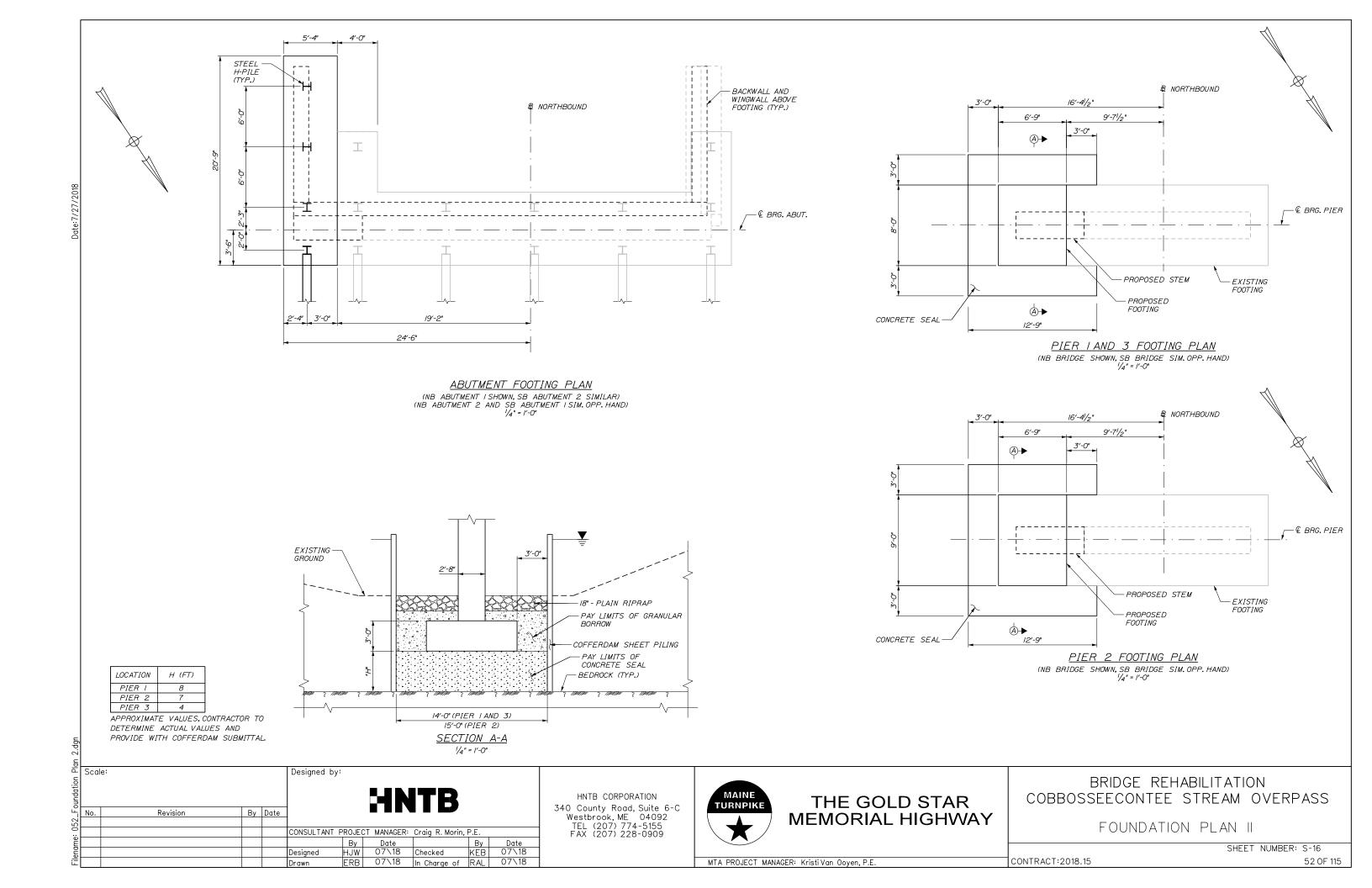
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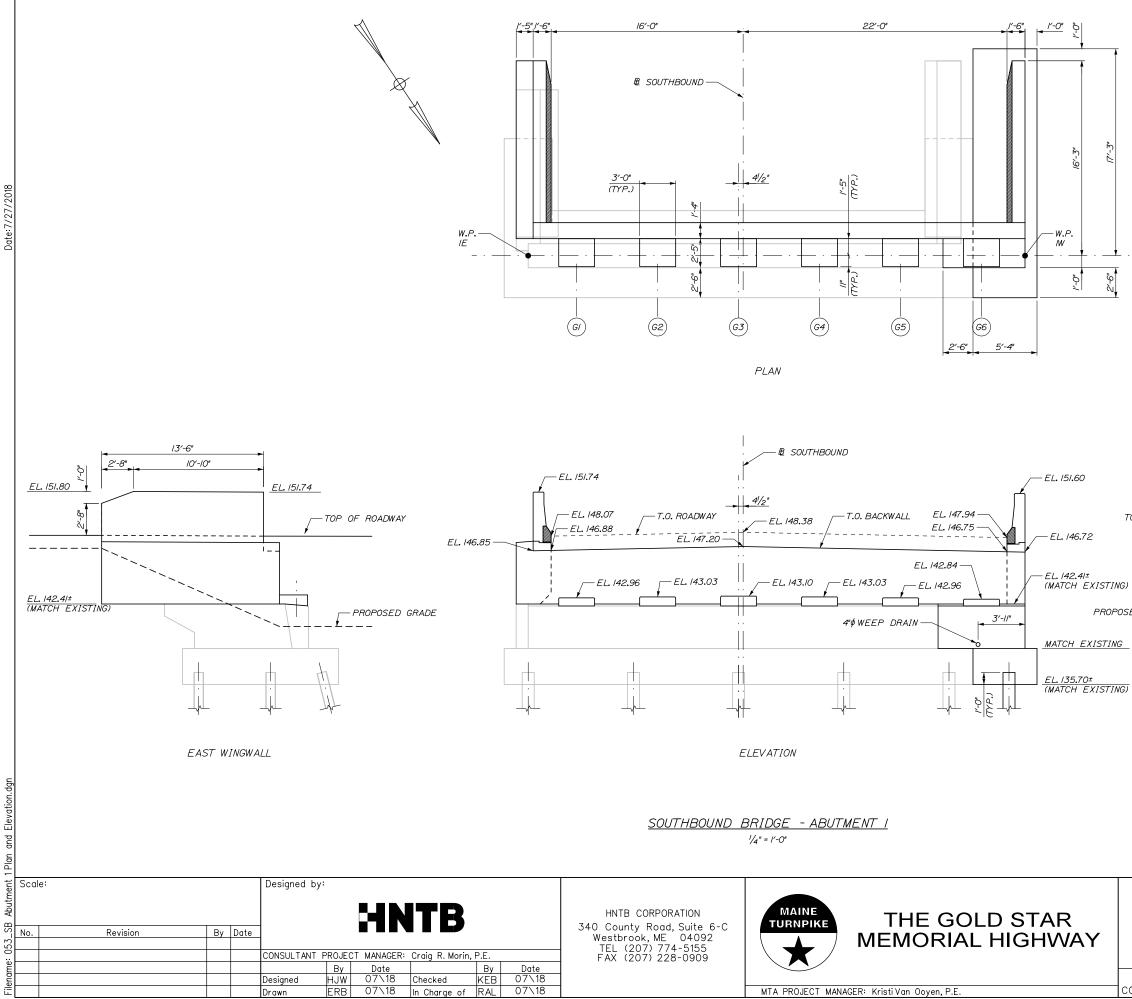
FRR

Designed

Drawn

KEB



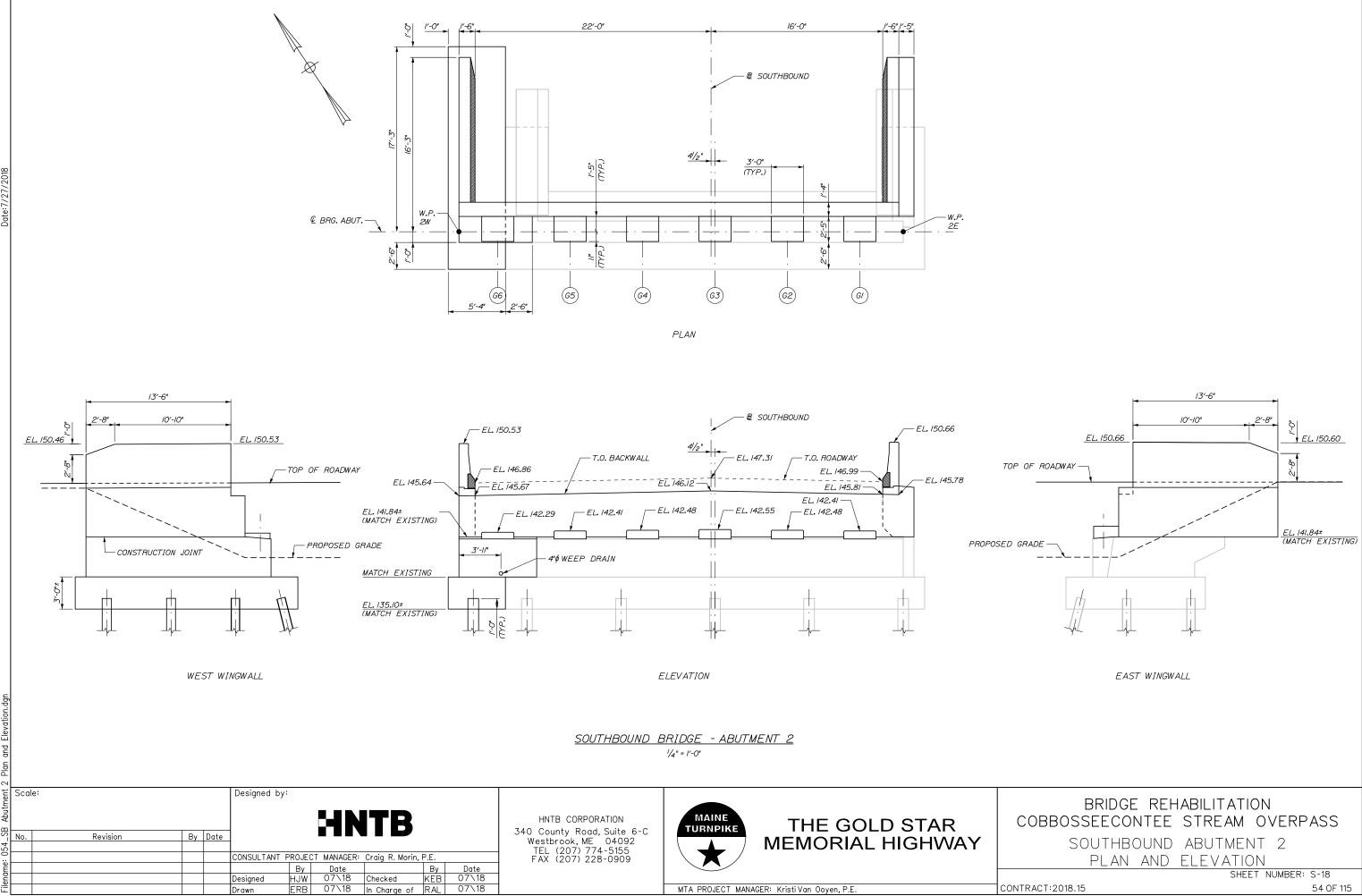


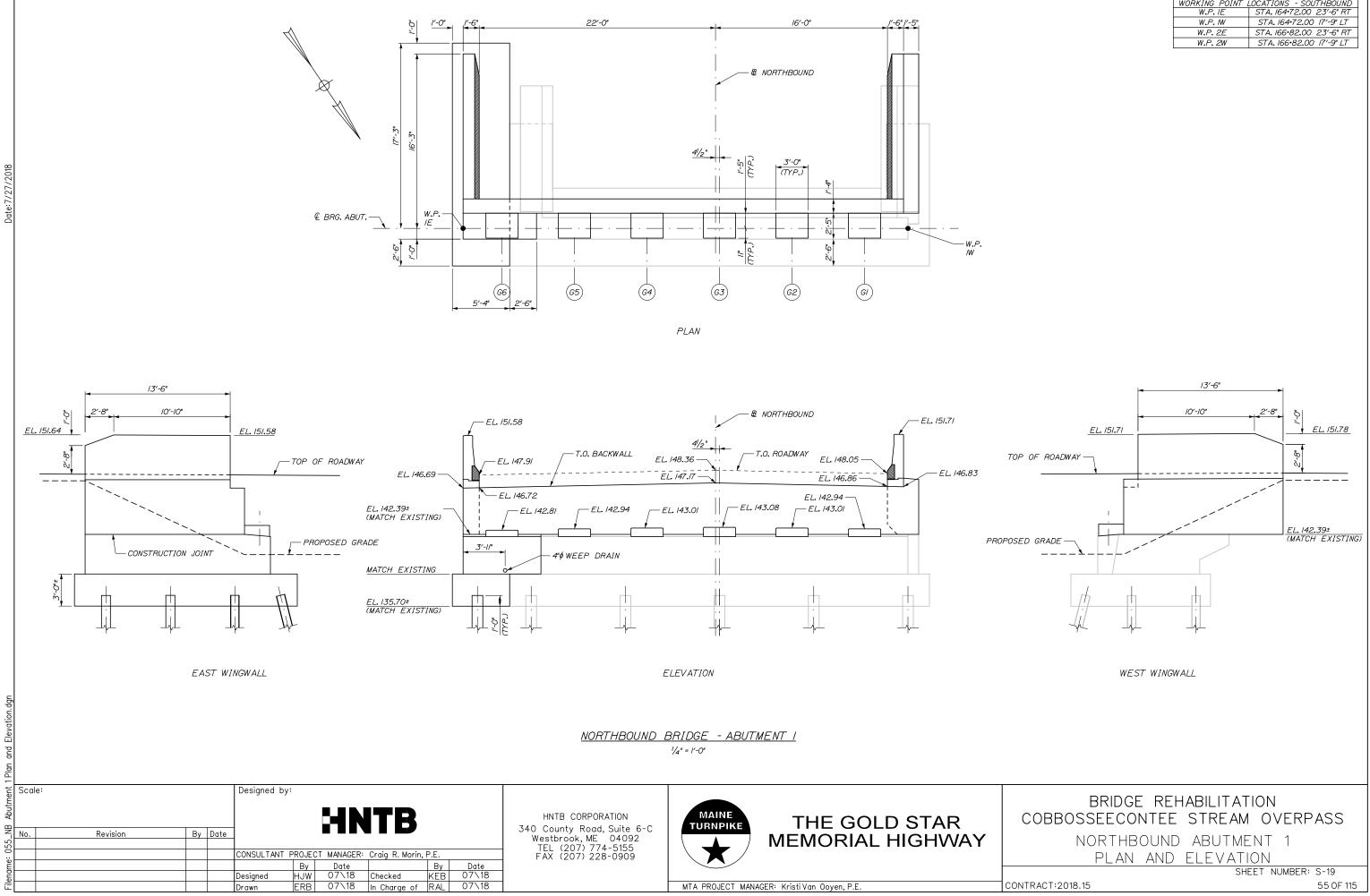
	WORKING POINT	LOCATIONS - SO	OUTHBOUND
	W.P. IE W.P. W	STA. 264+72.0 STA. 264+72.0	00 17'-9" RT
	W.P. 2E	STA. 266+82.0	00 17'-9" RT
	W.P. 2W	STA. 266+82.0	00 23'-6" LT
_			
↓ C BRG. ABUT.			
/			
	13′-6"		
	10'-10"	2'-8"	51
EL. 151.60		:	0 ↓ EL. 151.67
TOP OF ROADWAY -			-φ -ν
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SED GRADE	/	1	
co	NSTRUCTION JOINT	/	
, , , , , , , , , , , , , , , , , , , ,	i i		<i>+</i> ,
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	I		
WEST WING	WALL		

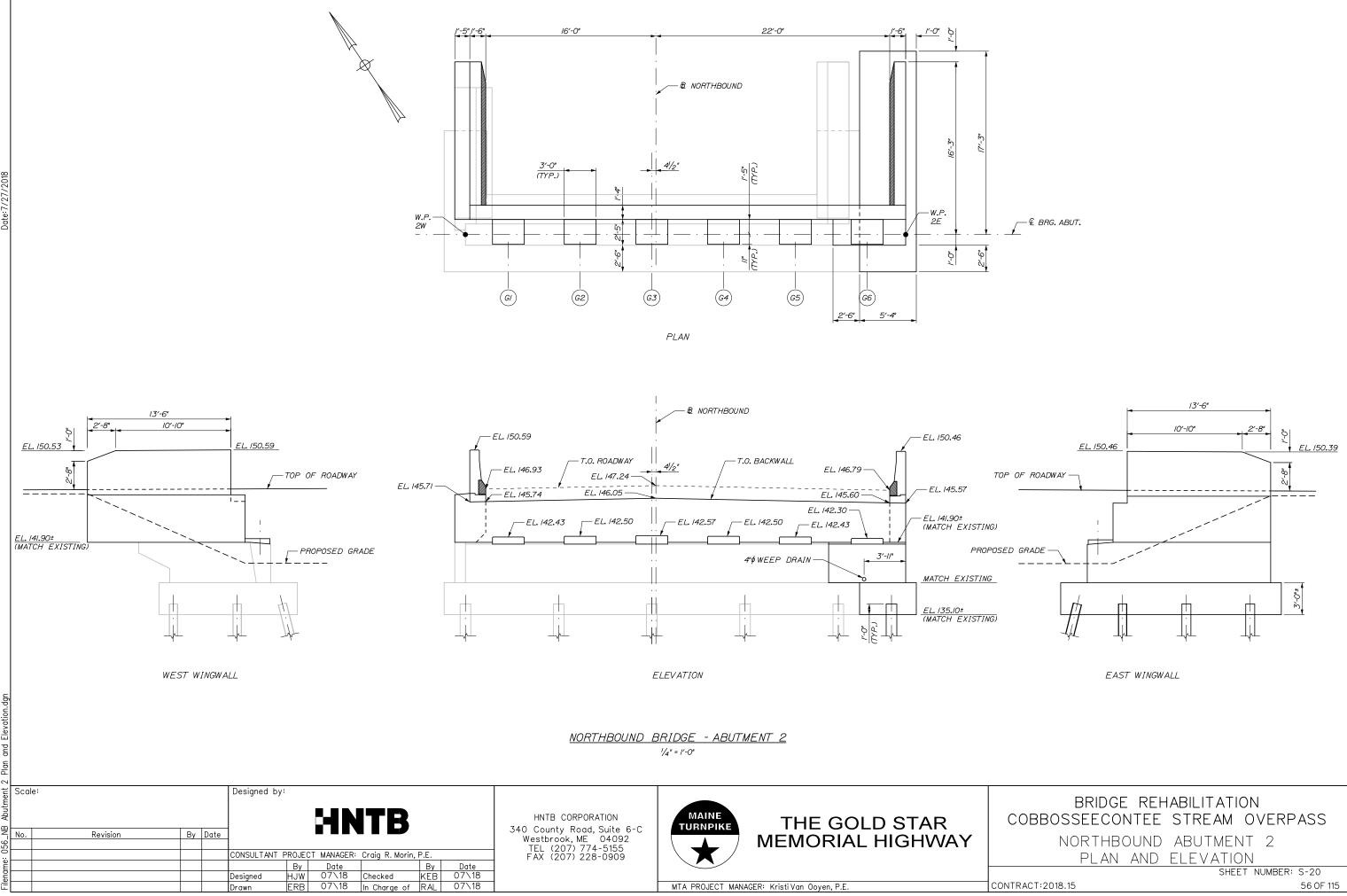
BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS SOUTHBOUND ABUTMENT 1 PLAN AND ELEVATION SHEET NUMBER: S-17

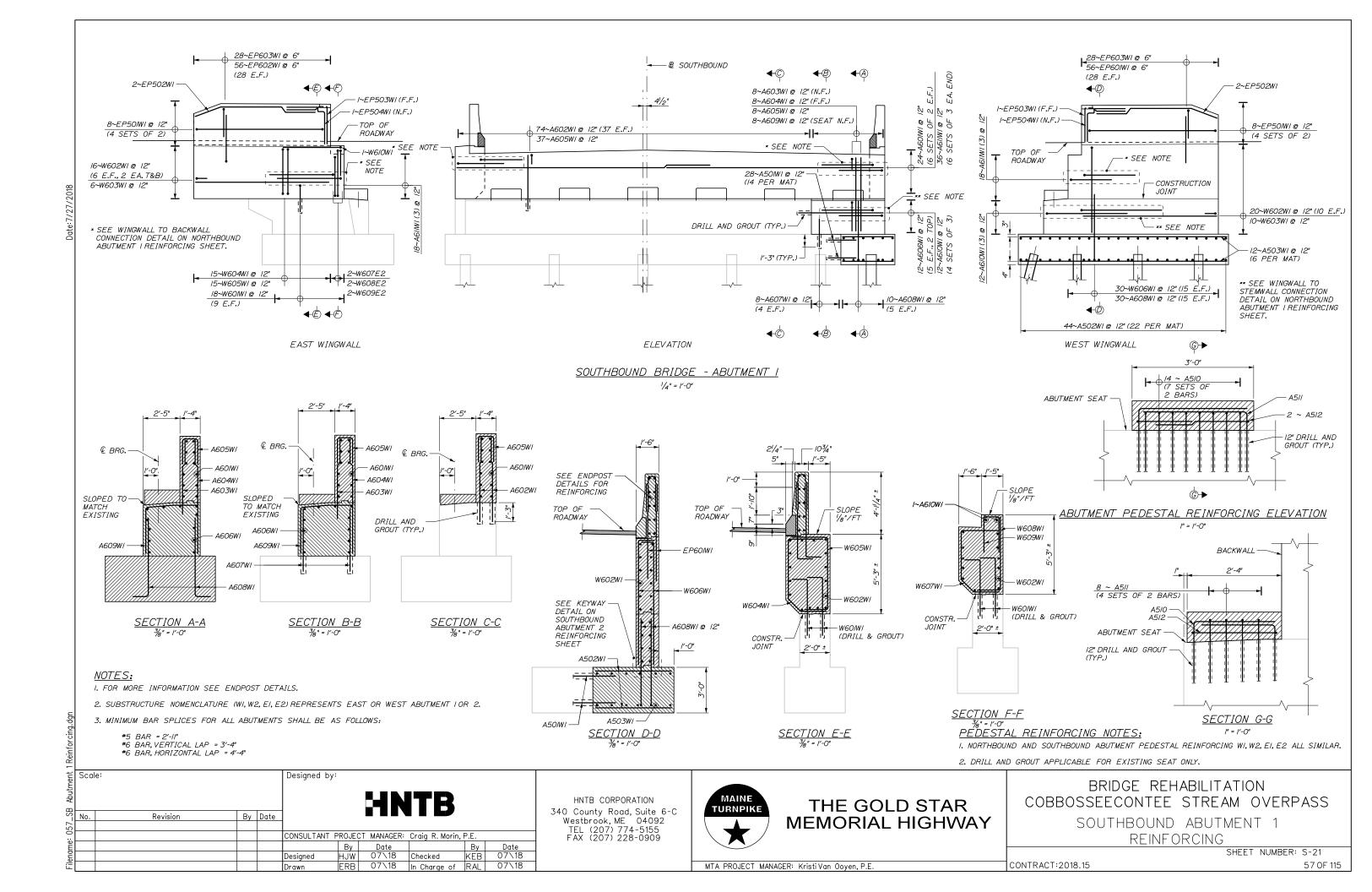
CONTRACT:2018.15

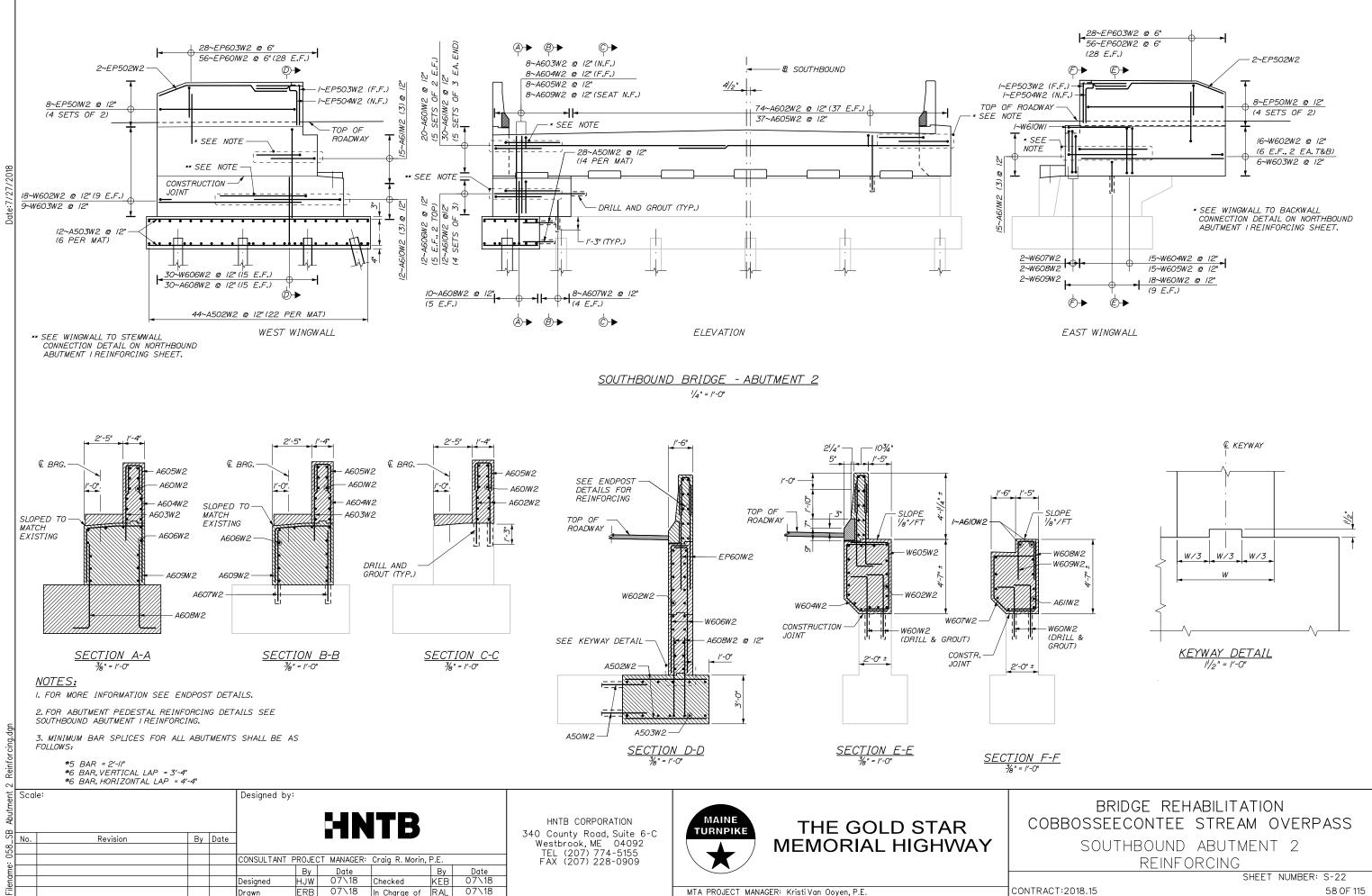
EET NUMBER: S-17 53 OF 115



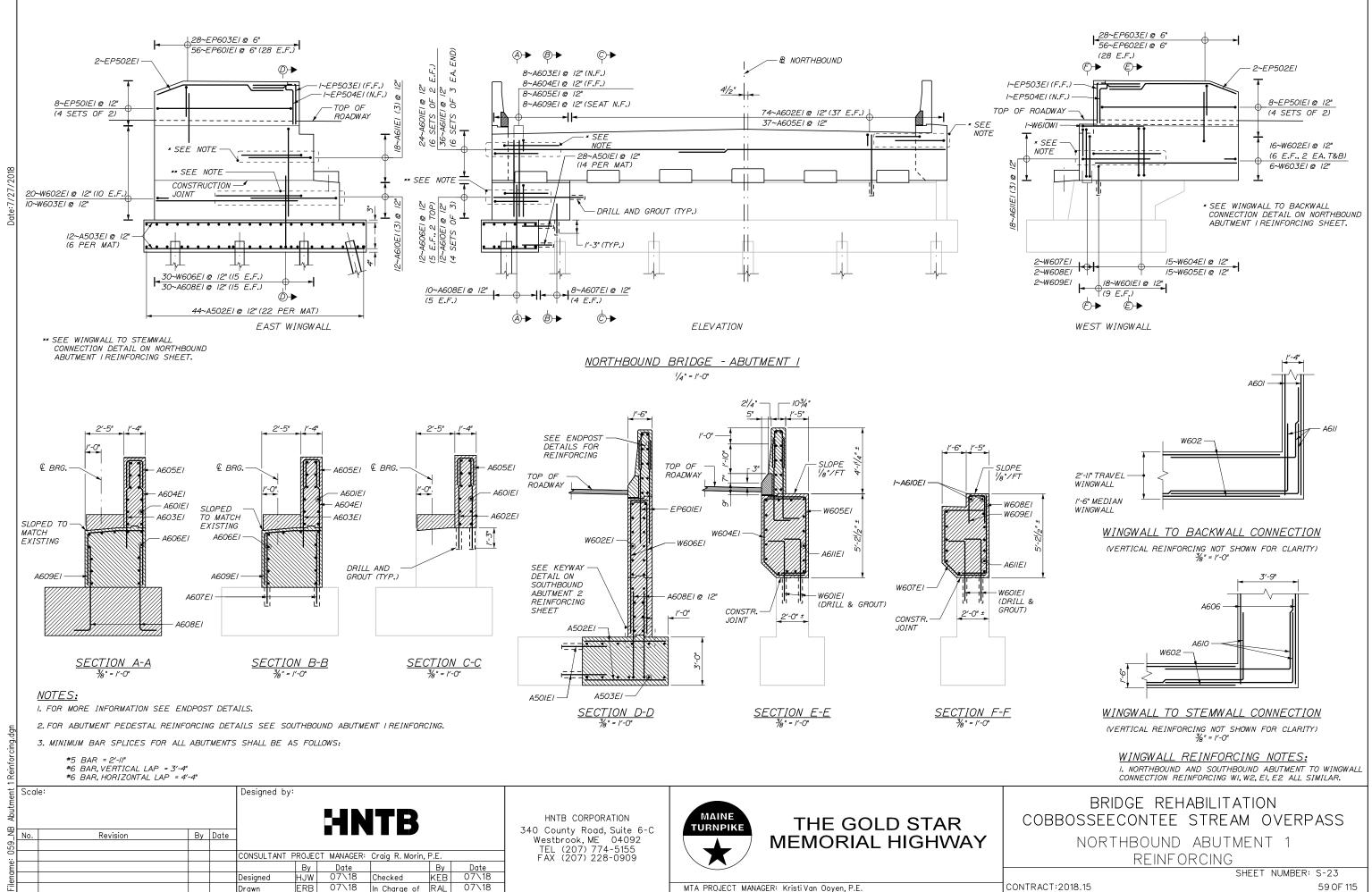




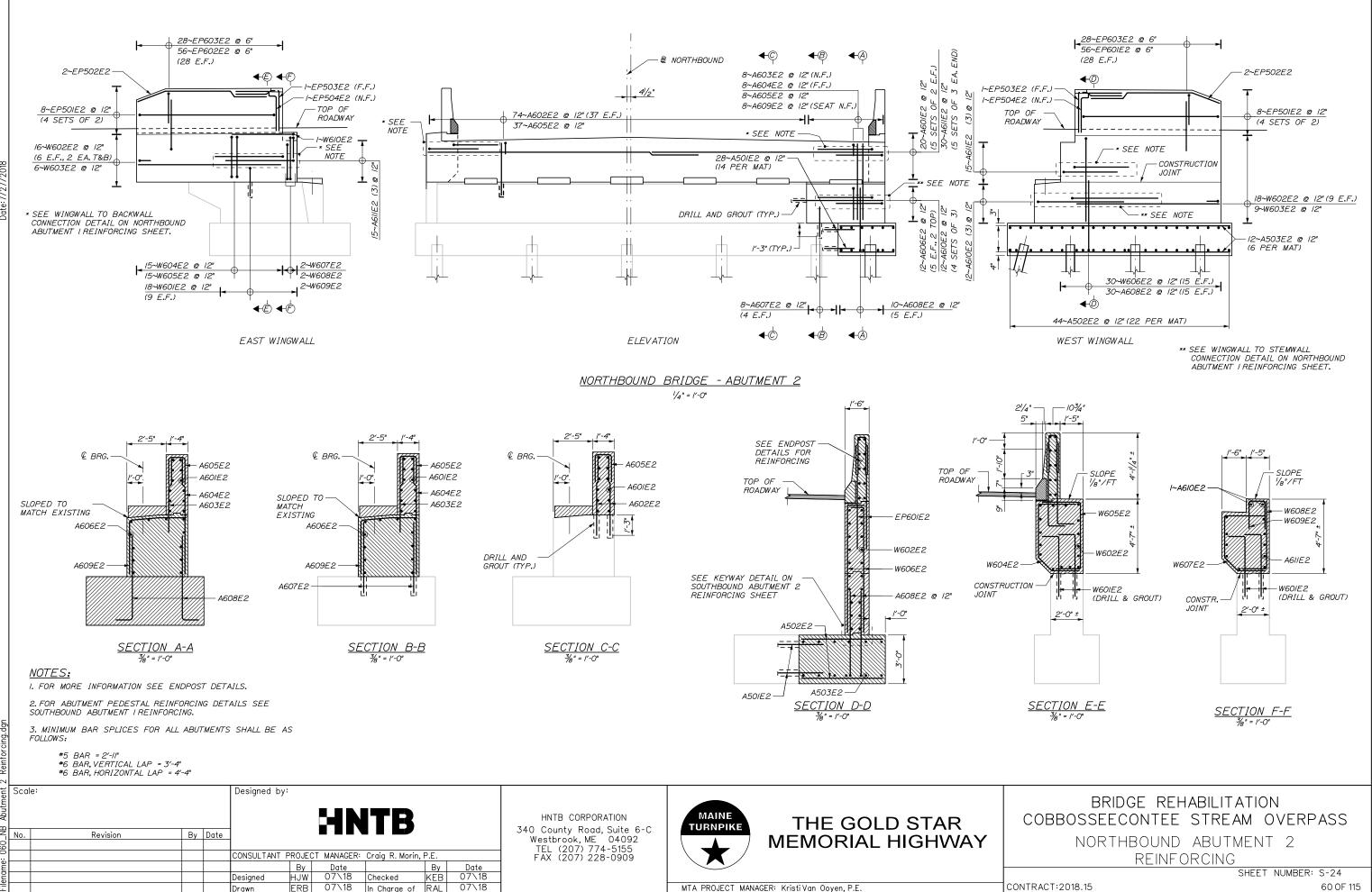




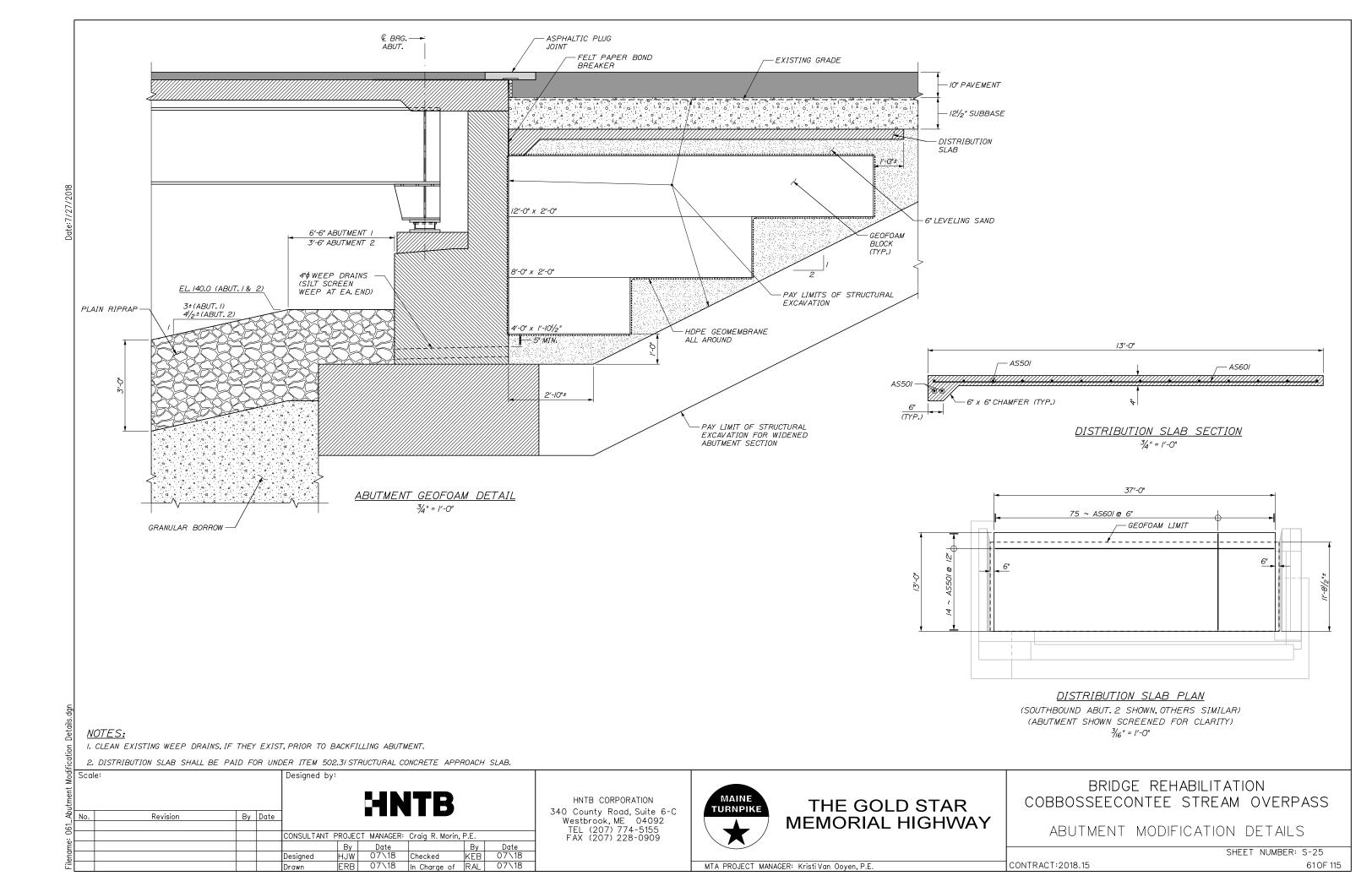
CONTRACT:2018.15

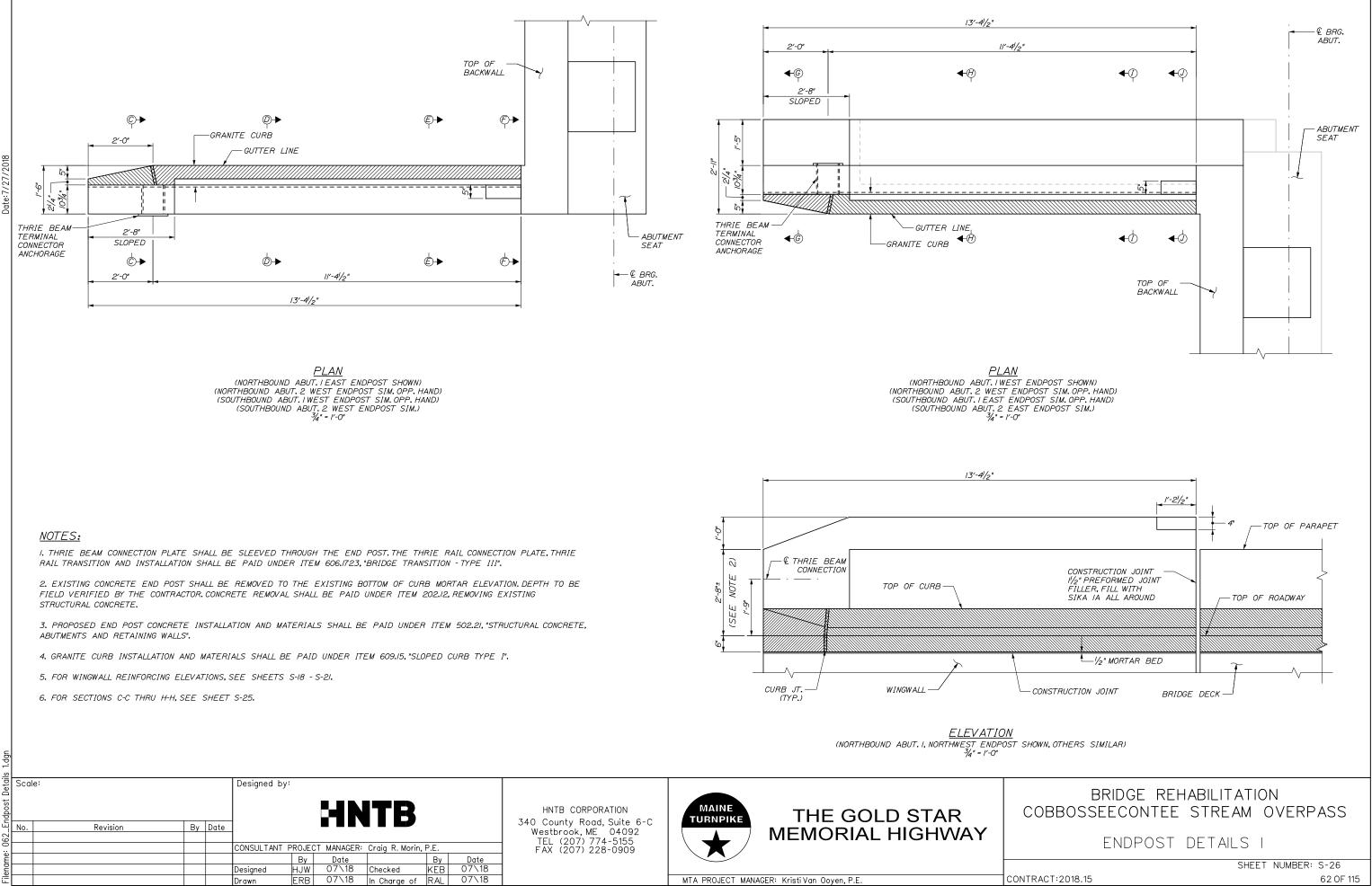


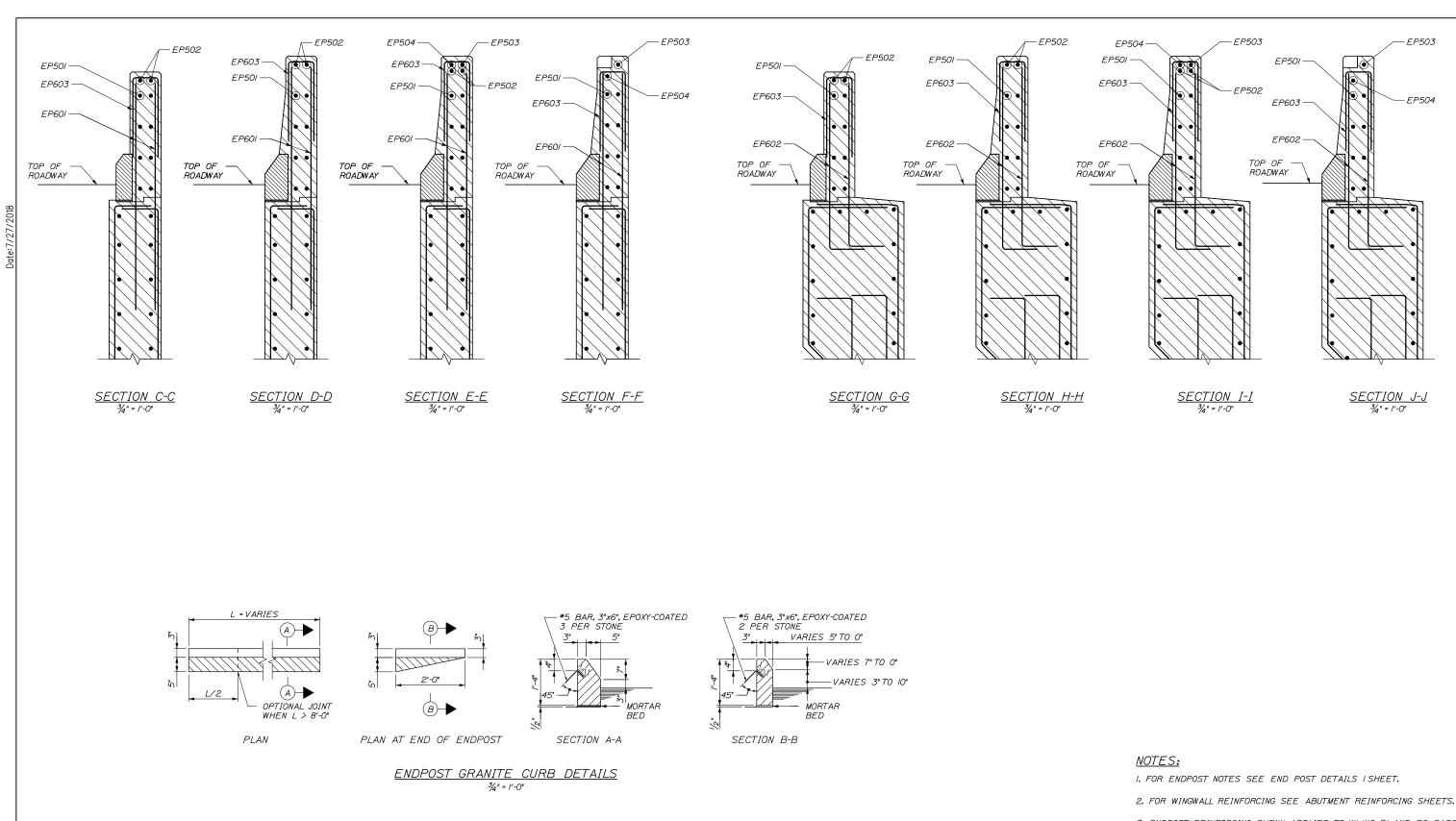
59 OF 115

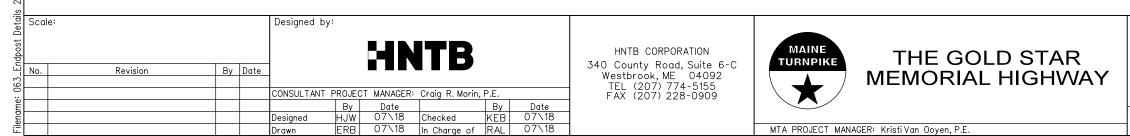


MTA PROJECT MANAGER: KristiVan Ooyen, P.E.









NOMENCLATURE FOR CLARITY (WI, W2, EI, E2).

3. ENDPOST REINFORCING SHOWN APPLIES TO WI, W2, EI, AND E2 BARS.

4. ENDPOST REINFORCING DOES NOT INCLUDE SUBSTRUCTURE UNIT

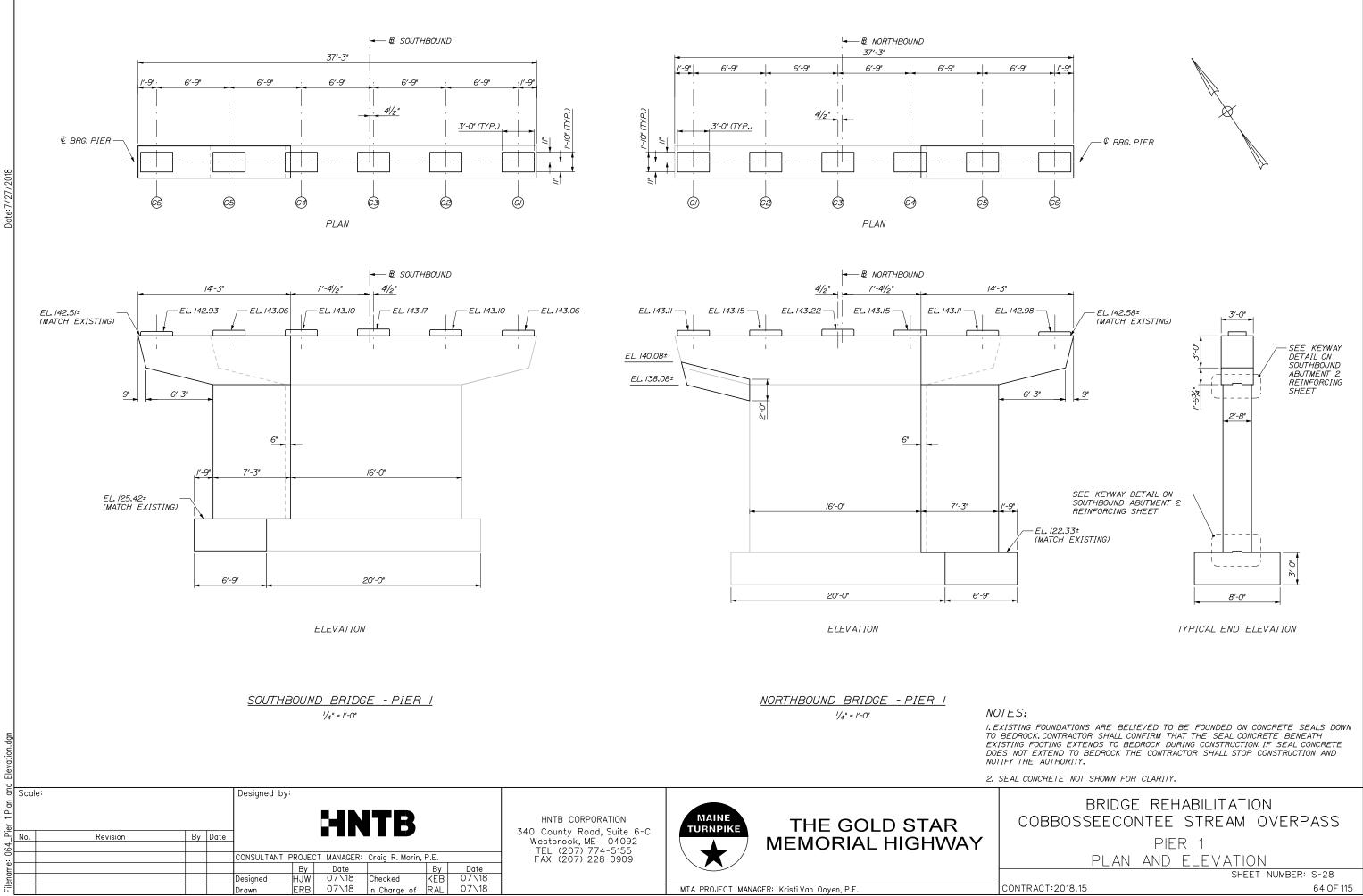
BRIDGE REHABILITATION

COBBOSSEECONTEE STREAM OVERPASS

ENDPOST DETAILS II

SHEET NUMBER: S-27

63 OF 115



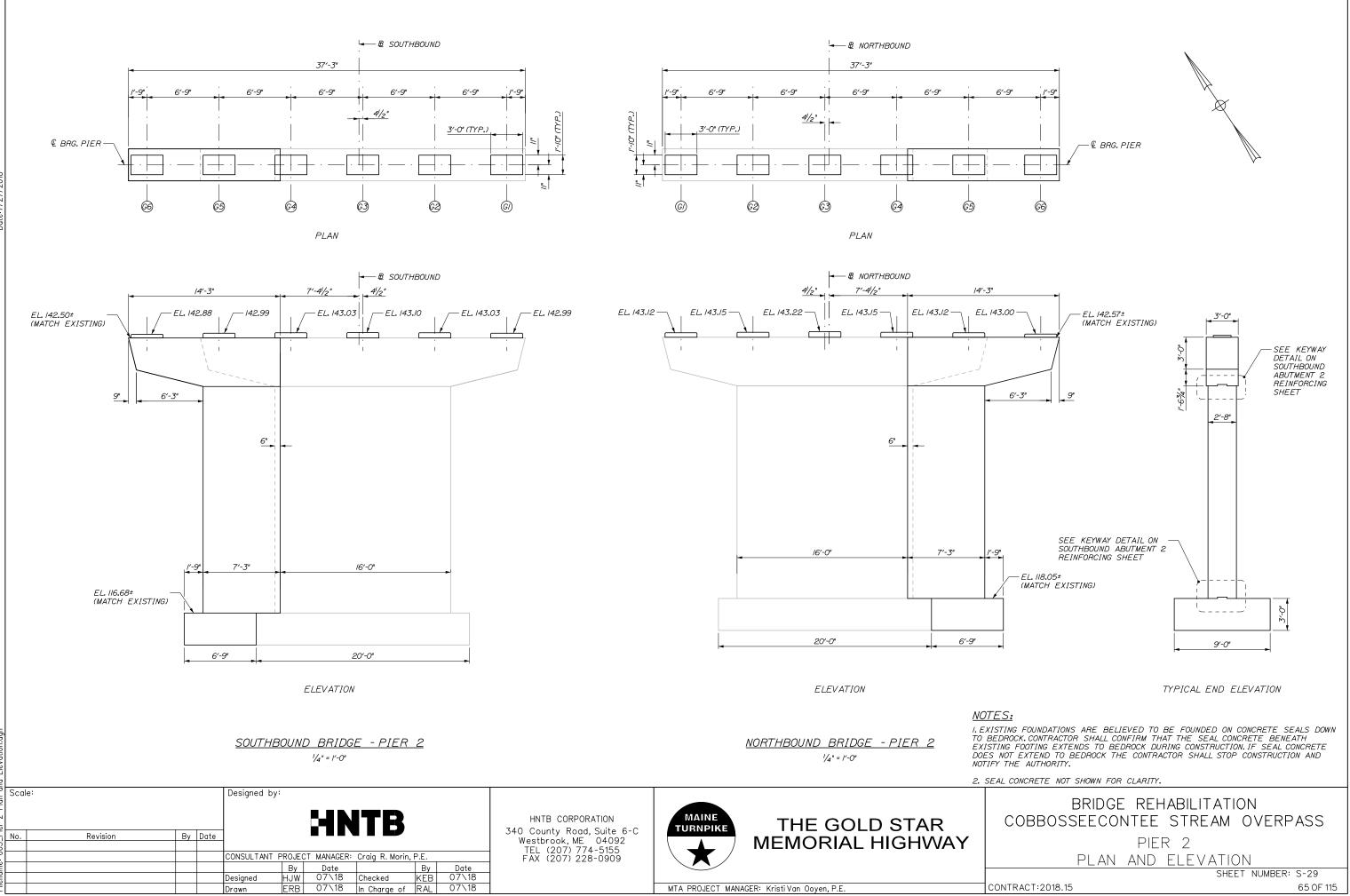
MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.

ERB

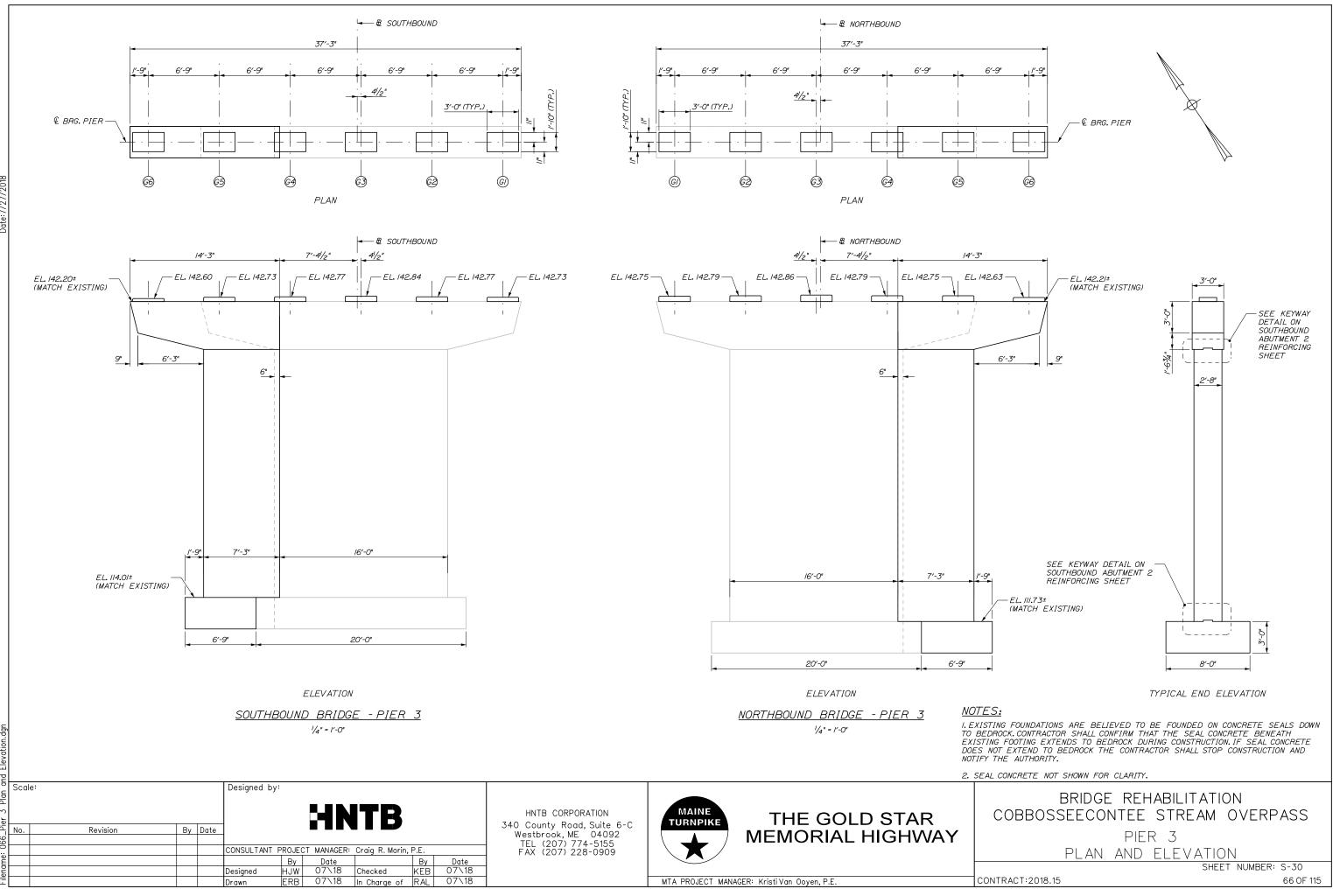
Drawn

CONTRACT:2018.15

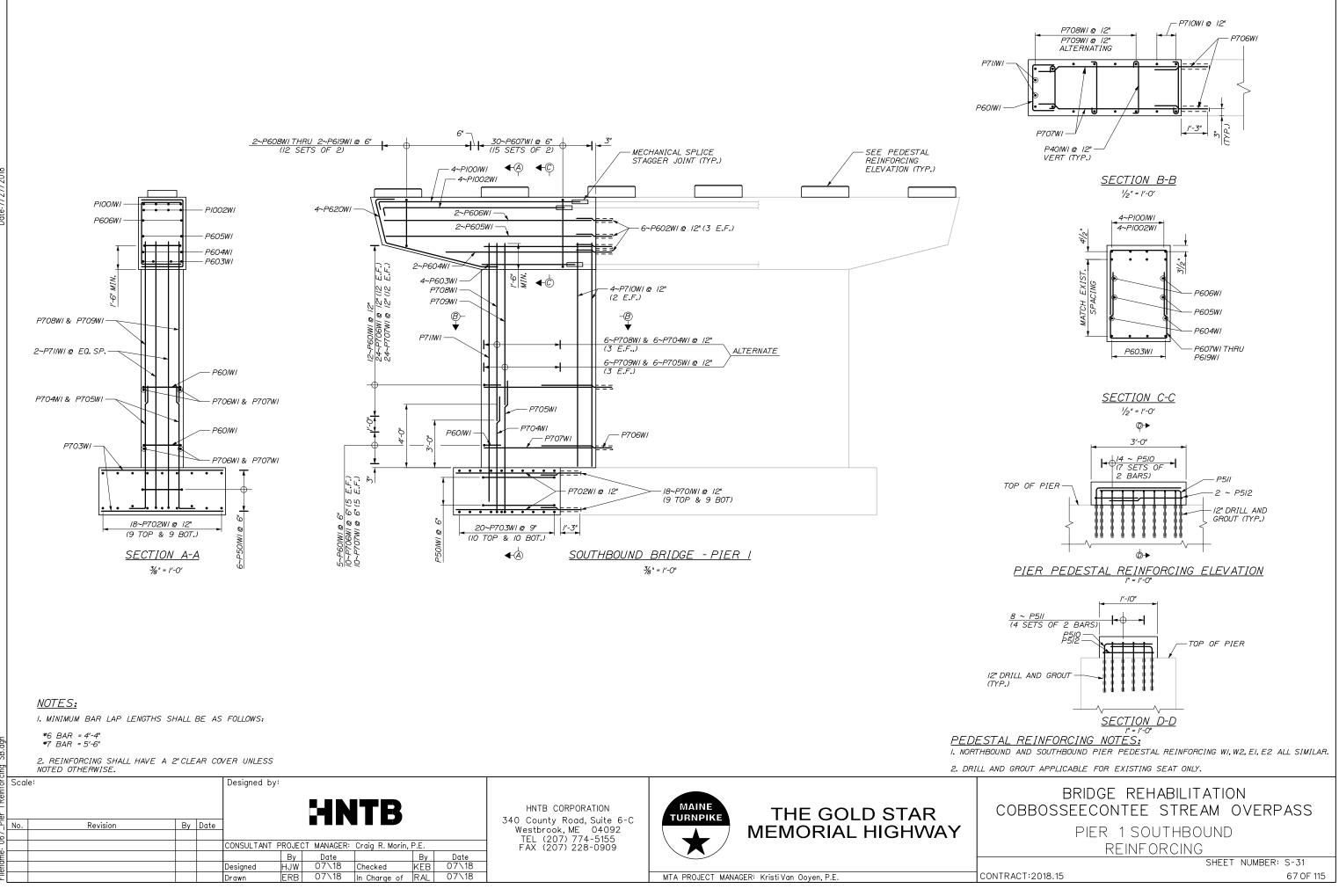
64 OF 115

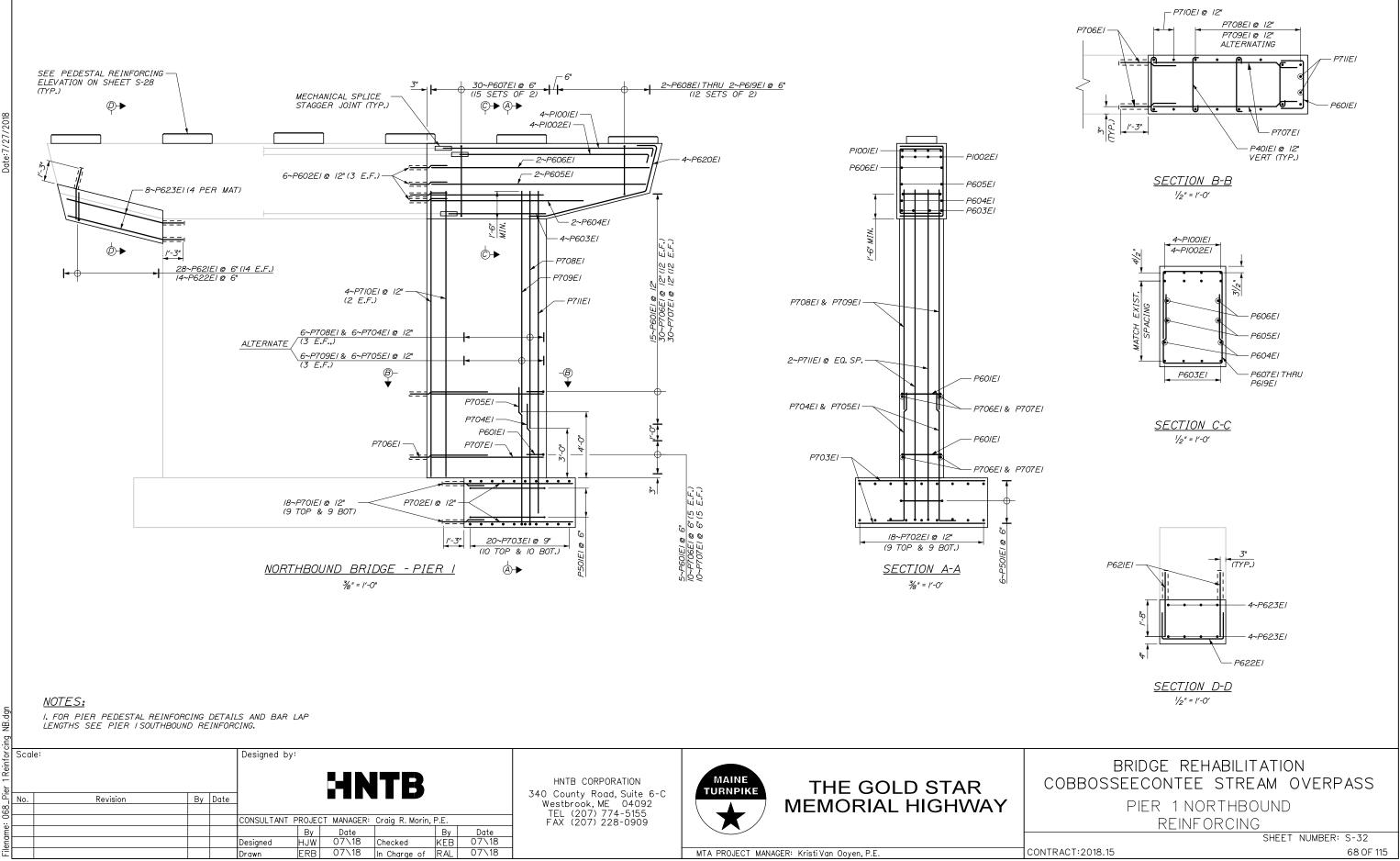


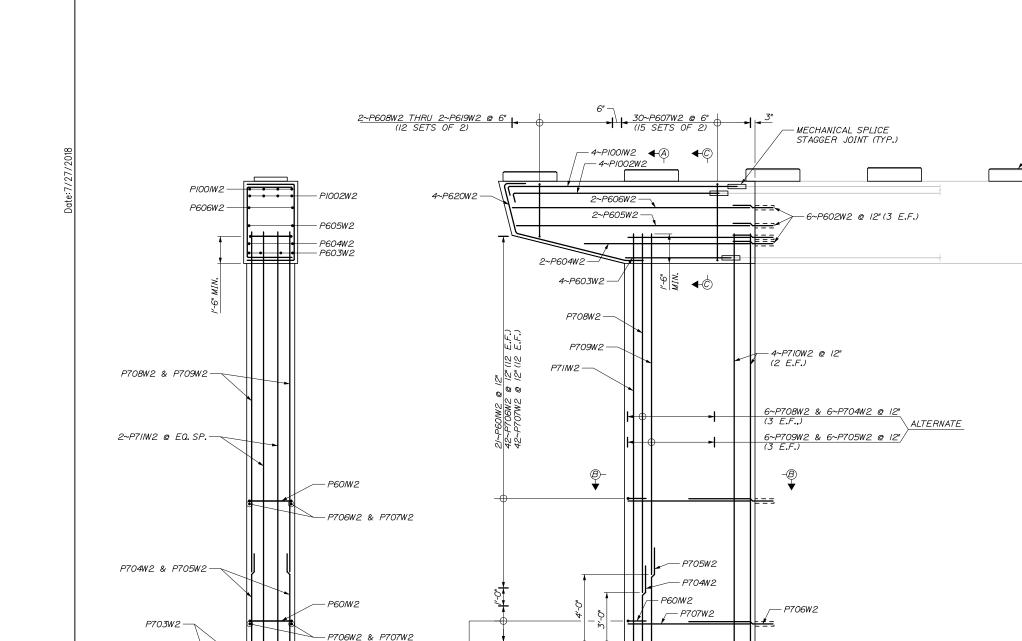
MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.











5--P60M2 & 6'' 10--P706M2 & 6''(5 E.F.) 10--P707W2 & 6''(5 E.F.) 3-+|

<u>NOTES:</u>

I. FOR PIER PEDESTAL REINFORCING DETAILS AND BAR LAP LENGTHS SEE PIER I SOUTHBOUND REINFORCING.

20~P702W2 @ 12"

(IO TOP & IO BOT.)

<u>SECTION A-A</u>

3/8" = 1'-0'

69_Pier 2 Reinforcin	Scale: No. Revision By	Date	Designed by:					HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092	MAINE TURNPIKE	THE GOLD STAR MEMORIAL HIGHWAY
8			CONSULTANT PROJE	CT MANAGER:	Craig R. Morin,	P.E.		TEL (207) 774-5155 FAX (207) 228-0909		
e E			By	Date		By	Date			
ene			Designed HJW	07\18	Checked	KEB	07\18			
iĒ[			Drawn ERB	07\18	In Charge of	RAL	07\18		MTA PROJECT MAN	AGER: KristiVan Ooyen, P.E.

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P50W2

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_ 38~P703W2 @ 4/2"

(19 TOP & 19 BOT.)

**€** 

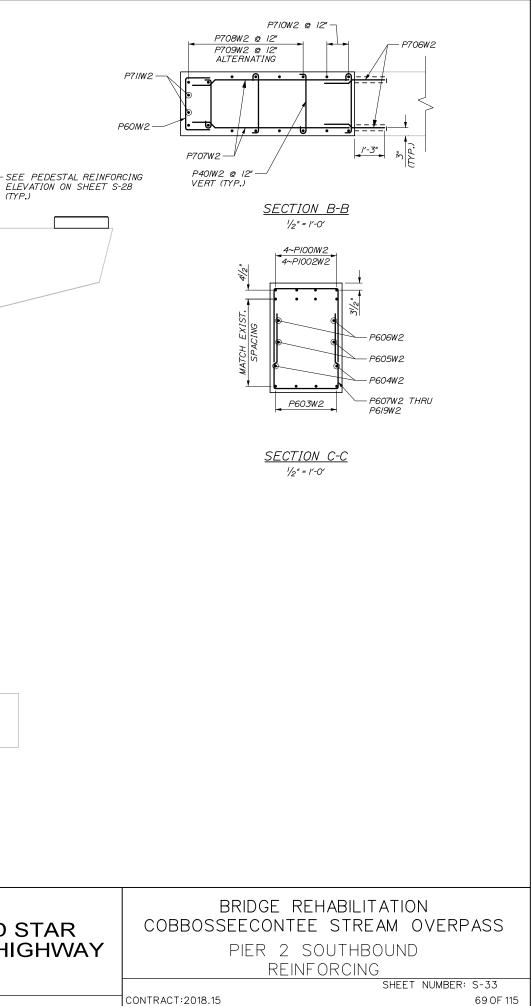
P702W2 @

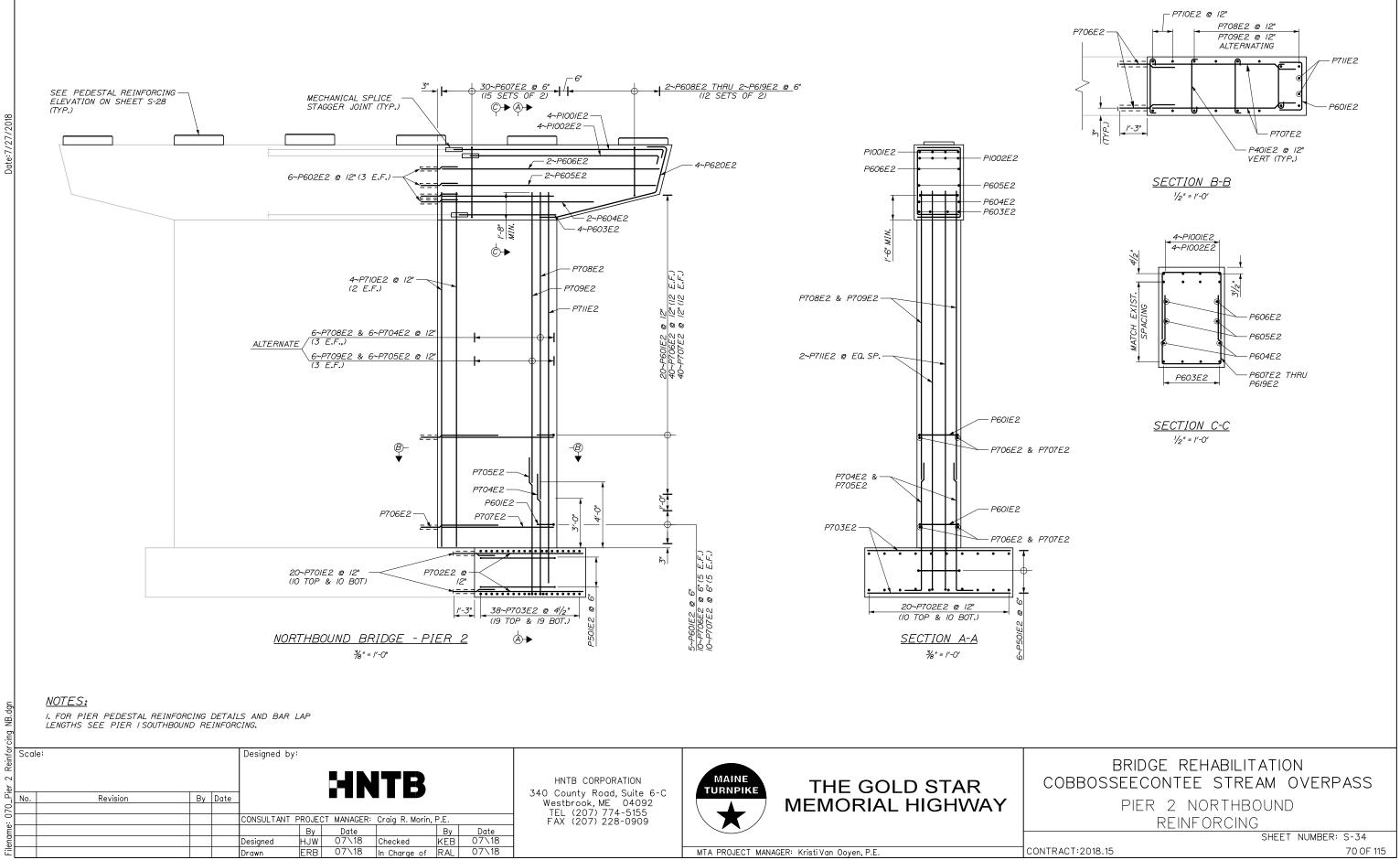
/'-3"

- 20~P701W2 @ 12" (10 TOP & 10 BOT)

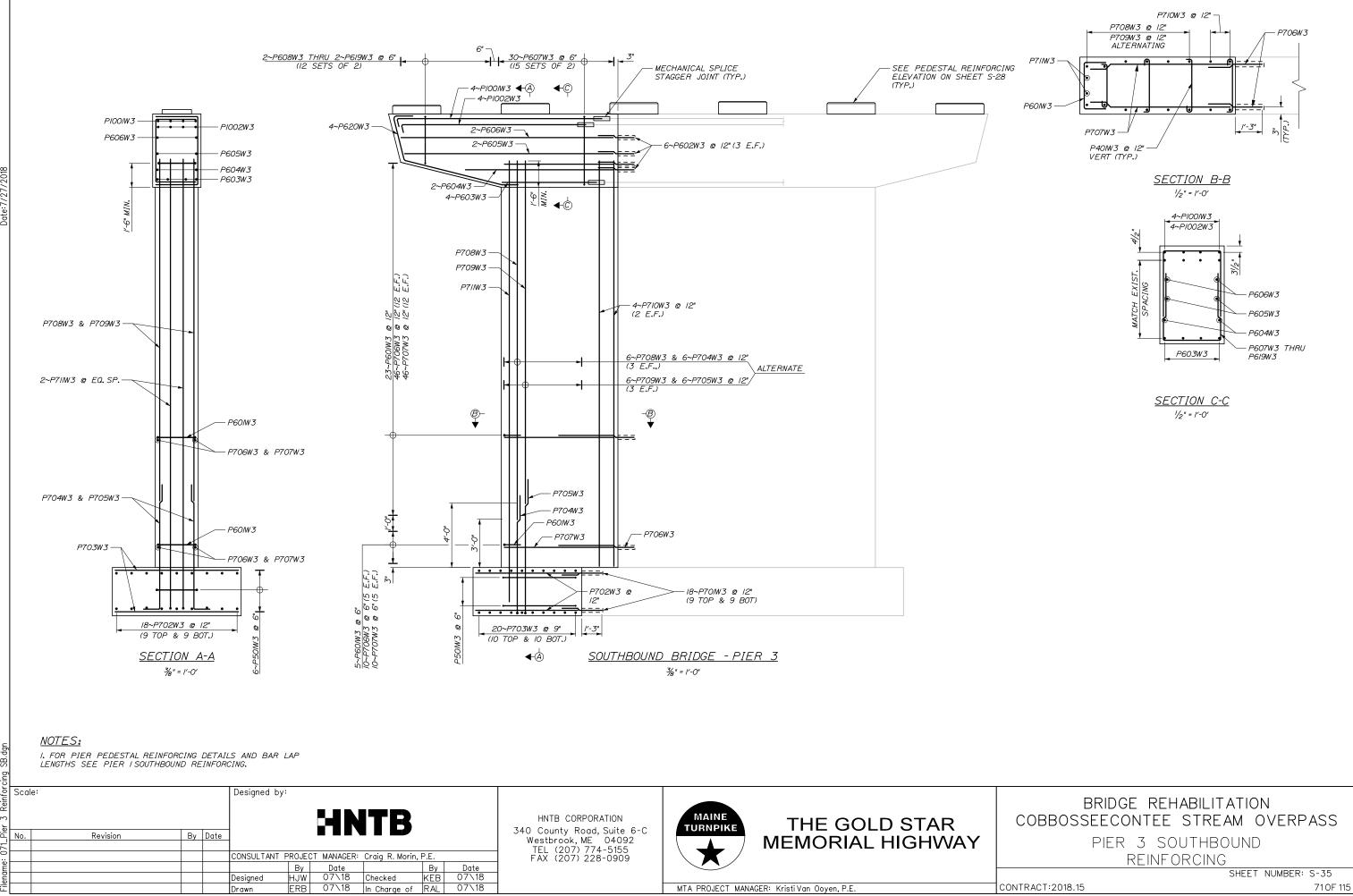
SOUTHBOUND BRIDGE - PIER 2

3⁄8" = 1'-0"

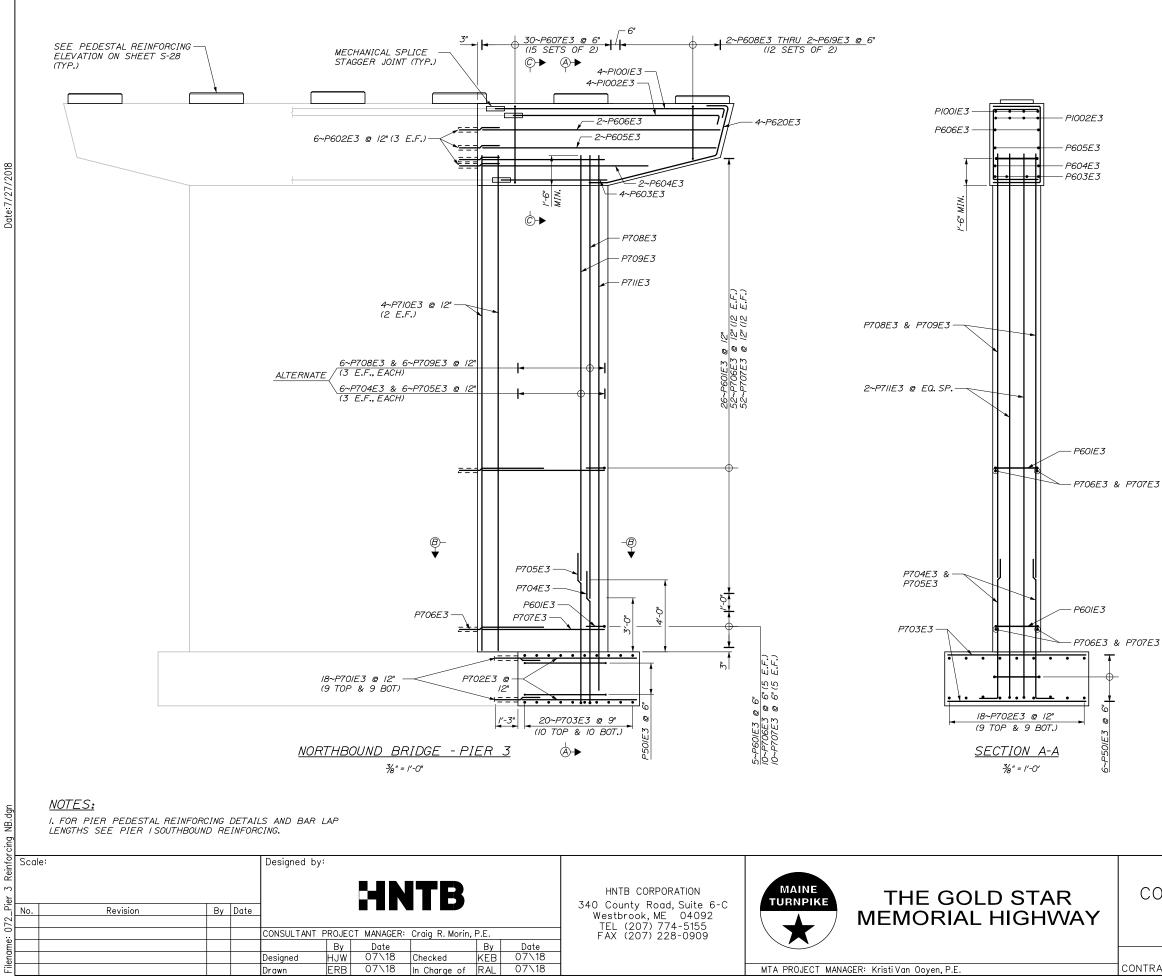




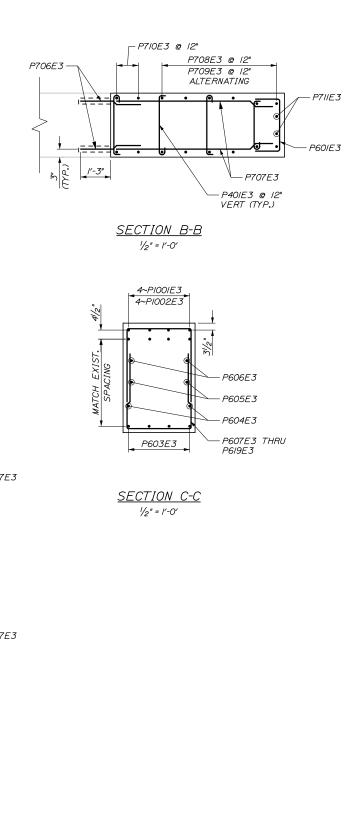
70 OF 115



Drawn

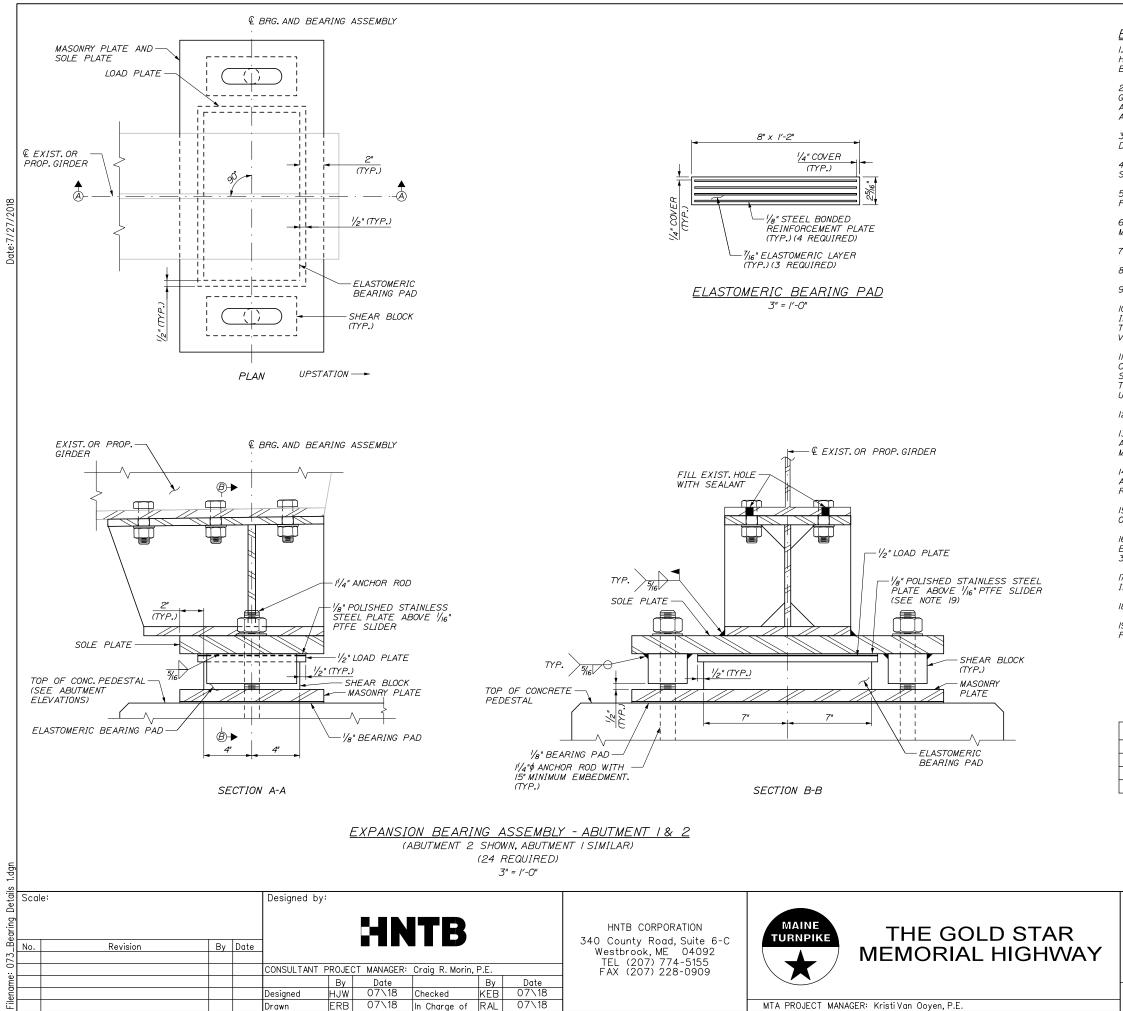


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BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS PIER 3 NORTHBOUND REINFORCING

CONTRACT:2018.15



#### BEARING NOTES:

I. ELASTOMER SHALL BE 100% POLYCHLOROPRENE (NEOPRENE) WITH DUROMETER HARDNESS OF 50. THE SHEAR MODULUS OF THE ELASTOMER SHALL BE BETWEEN 95 AND 135 PSI.

2. SOLE PLATE, MASONRY PLATE AND SHEAR BLOCK SHALL BE AASHTO M270 GRADE 36, WASHERS SHALL BE AASHTO F436, NUTS SHALL BE AASHTO A563. ALL STEEL COMPONENTS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153 AND A123 AS APPLICABLE.

3. CONTRACTOR SHALL RE-FINISH GALVANIZING AND GIRDER PAINT, AS DIRECTED BY ENGINEER, IN ACCORDANCE WITH ASTM A780 AFTER WELDING.

4. BEARINGS SHALL MEET THE REQUIREMENTS OF THE AASHTO M25/ SPECIFICATIONS.

5. BEARING ASSEMBLIES FOR EXISTING GIRDERS SHOWN. BEARING ASSEMBLIES FOR PROPOSED GIRDER SIMILAR.

6. VULCANIZING OF THE ELASTOMER TO THE SOLE PLATE OR LOAD PLATE, AND MASONRY PLATE, SHALL BE DONE DURING THE PRIMARY MOLD PROCESS.

7. ANCHOR RODS SHALL BE SET BEFORE INSTALLING STRUCTURAL STEEL.

8. UPSET THE THREADS ON THE ANCHOR RODS AFTER ASSEMBLY.

9. BEARINGS SHALL BE COVERED DURING TRANSIT.

IO. ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND SHALL BE VISIBLE AFTER THE BEARING IS INSTALLED.

II. ALL PRECAUTIONS NECESSARY SHALL BE TAKEN TO PROTECT BEARING COMPONENTS FROM FIELD WELD FLASH AND SPATTER. WELDING PROCEDURES SHALL BE ESTABLISHED BY THE CONTRACTOR TO RESTRICT THE MAXIMUM TEMPERATURE OF STEEL ADJACENT TO THE ELASTOMER TO 200'F THROUGH USE OF TEMPERATURE INDICATING CRAYONS OR OTHER SUITABLE MEANS.

12. BEARING PADS WERE DESIGNED USING "METHOD A".

I3. ALL STEEL REINFORCEMENT PLATES SHALL MEET THE REQUIREMENTS OF ASTM A36 UNLESS OTHERWISE NOTED AND SHALL BE DEBURRED PRIOR TO MOLDING THE BEARING.

14. ANCHOR RODS SHALL MEET THE REQUIREMENTS OF ASTM F1554, GRADE 55, AND SHALL BE SWEDGED OR THREADED ON THE EMBEDDED PORTION OF THE ROD.

15. ANCHOR ROD EMBEDMENT SHALL BE MEASURED FROM TOP OF PIER CAP OR ABUTMENT SEAT, NOT INCLUDING DEPTH OF PEDESTAL.

16. THE BEARINGS ARE DESIGNED SO THAT THE SUPERSTRUCTURE MAY BE ERECTED WHEN THE AMBIENT AIR TEMPERATURE IS WITHIN THE RANGE OF 30° F AND 90° F.

17. CONTRACTOR SHALL AVOID DAMAGING EXISTING REINFORCING DURING INSTALLATION OF ANCHOR RODS.

18. FOR BOLSTER DETAILS SEE S-42.

19. PTFE SLIDER SHALL BE RECESSED  $^{\prime}\!\!/_4"$  FROM THE EDGE OF THE LOAD PLATE ON ALL EDGES.

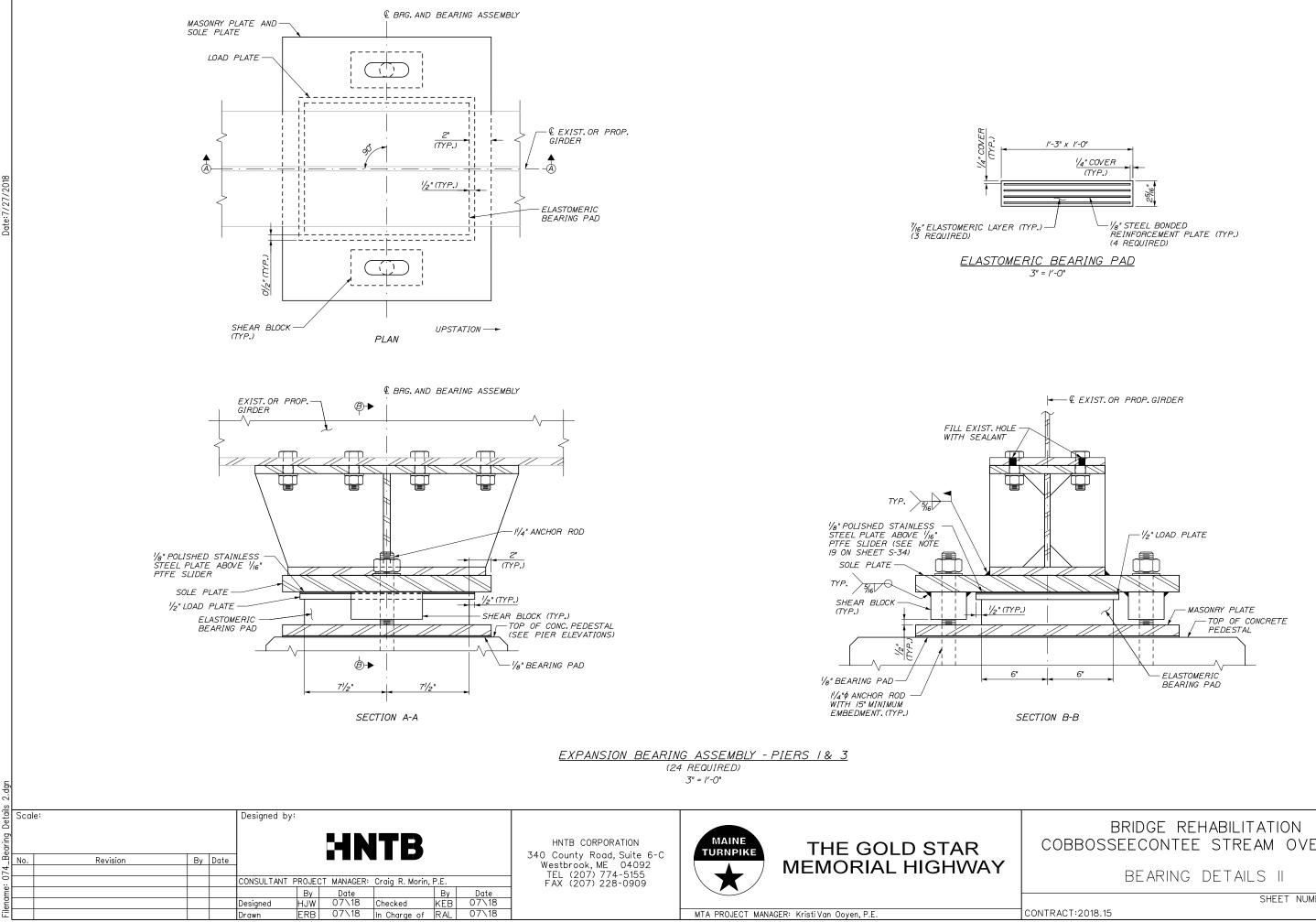
BE	ARING DESIGN	CRITERIA	
CRITERIA	ABUTMENTS	PIERS / & 3	PIER 2
UNFACTORED DEAD LOAD	26 KIPS	80 KIPS	80 KIPS
UNFACTORED LIVE LOAD	52 KIPS	80 KIPS	8I KIPS
MAX. LONGITUDINAL DISPL.	1.18 INCHES	0.65 INCHES	0.00 INCHES

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

BEARING DETAILS I

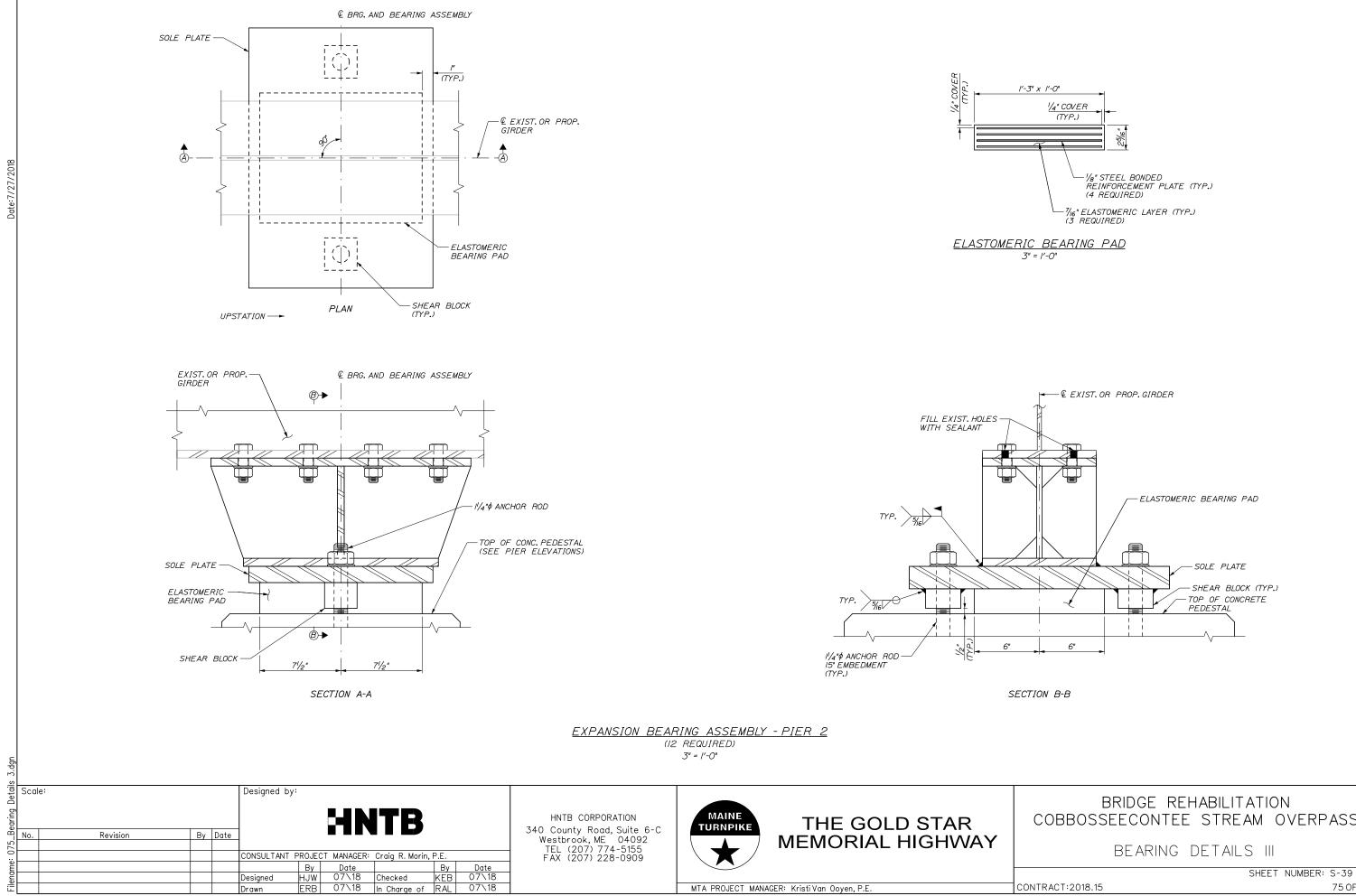
CONTRACT:2018.15

SHEET NUMBER: S-37



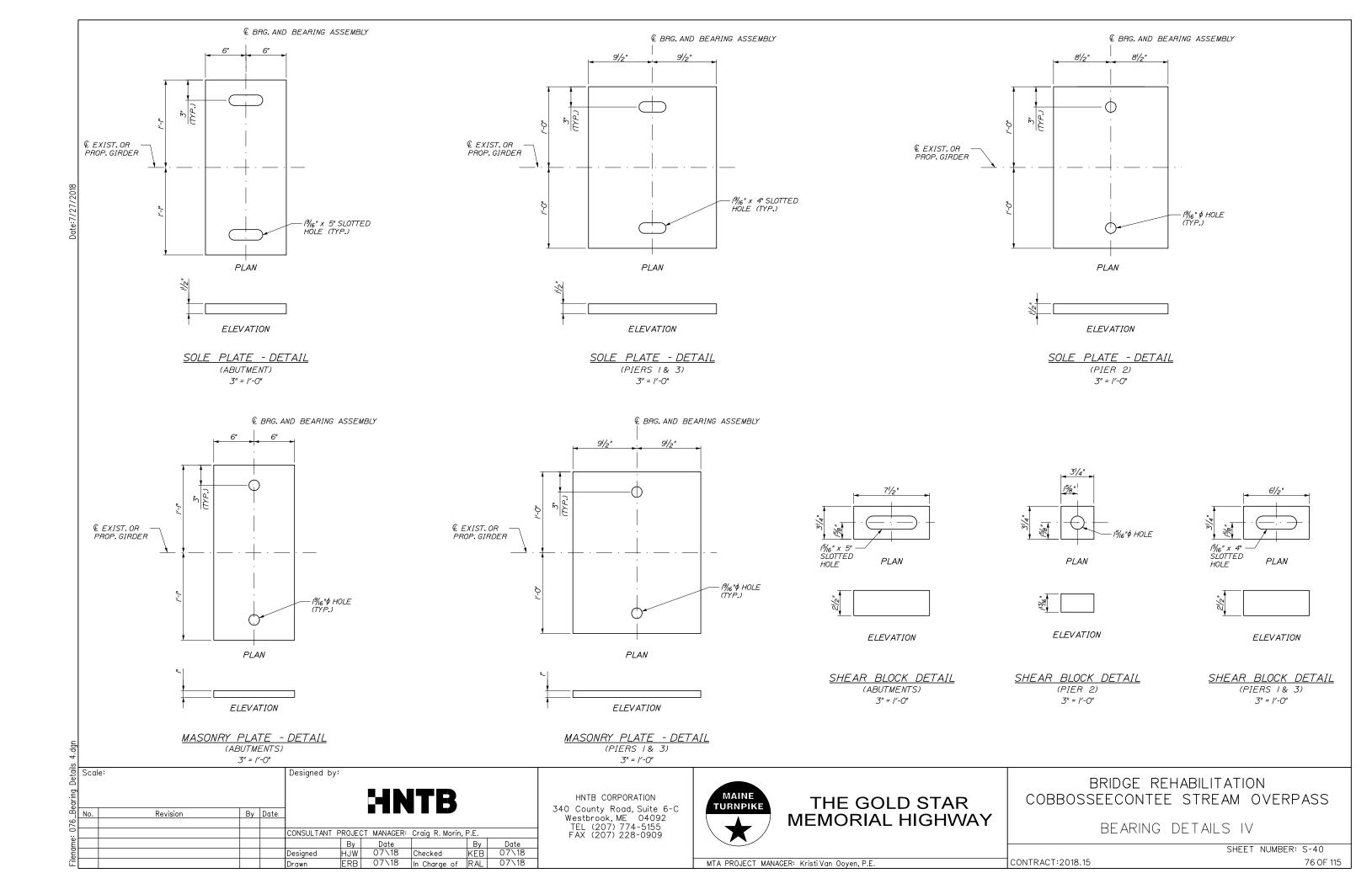
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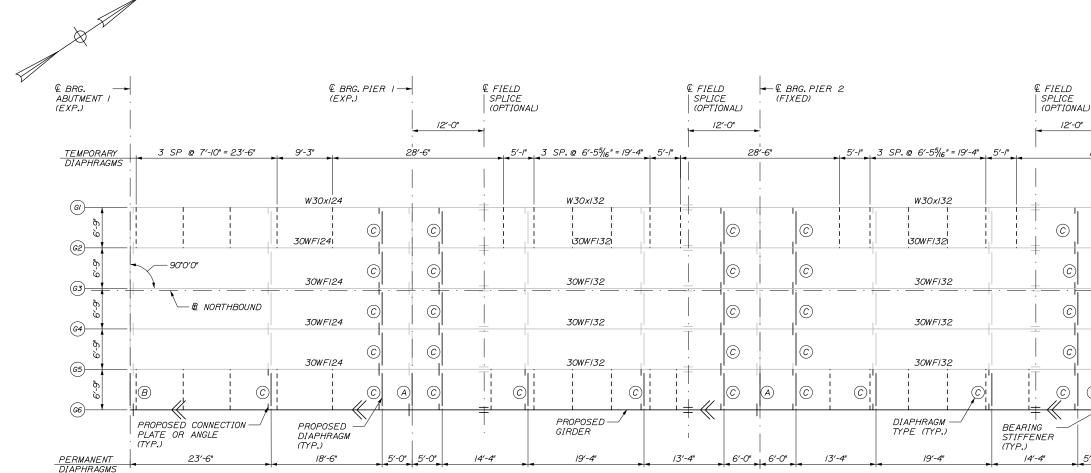
# COBBOSSEECONTEE STREAM OVERPASS



COBBOSSEECONTEE STREAM OVERPASS

CONTRACT:2018.15





#### NOTES:

I. CAMBER ORDINATES ARE COMPUTED TO COMPENSATE FOR ALL DEAD LOAD DEFLECTIONS.

2. NO TRANSVERSE BUTT-WELD SPLICES WILL BE ALLOWED IN THE FLANGE PLATES OR WEB PLATES WITHIN IO FEET OR IO% OF THE SPAN LENGTH (WHICHEVER IS GREATER) FROM THE POINTS OF MAXIMUM NEGATIVE MOMENT OR MAXIMUM POSITIVE MOMENT. BUTT-WELD SPLICES IN FLANGES SHALL BE NOT LESS THAN I FOOT FROM TRANSVERSE BUTT-WELDS IN THE WEB PLATES AND NO TRANSVERSE WEB OR FLANGE BUTT-WELDS SHALL BE LOCATED WITHIN 6 INCHES OF OTHER TRANSVERSE WELDS (E.G. CONNECTION PLATES TO WEB WELDS) ON EITHER FLANGE OR WEB.

3. SECTIONS OF FLANGE PLATES OR WEB PLATES BETWEEN TRANSVERSE SHOP SPLICES OR BETWEEN A TRANSVERSE SHOP SPLICE AND A FIELD SPLICE SHALL BE NOT LESS THAN 20 FEET IN LENGTH UNLESS OTHERWISE SHOWN ON THE PLANS.

4. BEARING STIFFENERS SHALL BE PLUMB AFTER ERECTION AND DEAD LOADING OF THE STRUCTURE.

5. ALL BOLTS SHALL BE  $7_8$ " HIGH STRENGTH BOLTS. HOLES SHALL BE  $15_{\rm M6}$ " UNLESS OTHERWISE NOTED. BOLTS SHALL BE INSTALLED WITH HEADS DOWN AT ALL BOTTOM FLANGE CONNECTIONS AND HEADS UP AT ALL TOP FLANGE CONNECTIONS.

6. DIAPHRAGM CONNECTION PLATES MAY BE EITHER PLUMB OR NORMAL TO THE TOP FLANGE.

7. BUTT WELDS AT WEB SPLICES AND FLANGE SPLICES SHALL BE GROUND FLUSH IN LONGITUDINAL DIRECTION OF GIRDER.

8. SHEAR STUDS SHALL EXTEND A MINIMUM OF 2" INTO THE SLAB.SEE "SHEAR CONNECTOR DETAIL" ON STEEL DETAILS I SHEET.

9. ALL FAYING SURFACES SHALL BE PAINTED OR SEALED IN ACCORDANCE WITH FHWA TECHNICAL ADVISORY T5140.22, SECTION 4.c.2.c. TO PREVENT THE FORMATION OF PACK RUST.

IO. PRIOR TO ERECTION OF STRUCTURAL STEEL THE CONTRACTOR SHALL SUBMIT A DETAILED ERECTION PLAN FOR APPROVAL.

II. WHERE PROPOSED DIAPHRAGMS ARE TO BE CONNECTED TO EXISTING GIRDERS, DIAPHRAGM SPACING, CONNECTION PLATE BOLT LAYOUT, BOLT PATTERN, ETC. SHALL BE FIELD VERIFIED BY THE CONTRACTOR.

FRAMING PLAN (NORTHBOUND SHOWN, SOUTHBOUND SIMILAR) 1/8" = 1'-0"

12. USE A SINGLE ROW OF SHEAR STUDS ON SPLICE PLATES TO AVOID INTERFERENCE WITH SPLICE BOLTS.

13. CONNECTION PLATES ON PROPOSED GIRDER SHALL BE FULL WEB DEPTH AND WELDED TO THE WEB AND FLANGES ON BOTH SIDES OF THE PLATES. WELDS SHALL TERMINATE  $\frac{1}{2}$ "  $\frac{1}{8}$ " FROM THE ENDS OF THE PLATES.

14. BEARING STIFFENERS SHALL BE MILL-TO-BEAR ON THE BOTTOM FLANGE AND TIGHT FIT TO THE TOP FLANGE. BEARING STIFFENERS USED AS CONNECTION PLATES SHALL BE DETAILED AS CONNECTION PLATES. BEARING STIFFENERS USED ON BOLSTERS SHALL BE MILL-TO-BEAR ON THE TOP AND BOTTOM FLANGE.

I5. ALL EXISTING BEARINGS SHALL BE REMOVED AND REPLACED WITH ELASTOMERIC BEARINGS. EXISTING STEEL BEARINGS IN SATISFACTORY CONDITION SHALL BE SALVAGED AS DESCRIBED IN SPECIAL PROVISION 202. BEARINGS NOT SALVAGED SHALL BE INCIDENTAL TO ITEM 202.19, REMOVING EXISTING STRUCTURE. FOR ESTIMATING PURPOSES IT IS ASSUMED THE BEARINGS UNDER INTERIOR BEAMS WILL BE SALVAGED AND THOSE UNDER FASCIA BEAMS WILL NOT.

I6. TEMPORARY DIAPHRAGMS SHALL BE INSTALLED BETWEEN GI-G2 AND G5-G6 ON BOTH NORTHBOUND AND SOUTHBOUND BRIDGES PRIOR TO PLACING CONCRETE DECK. FOR TEMPORARY DIAPHRAGM DETAILS, SEE SHEET S-39. APPROXIMATE TEMPORARY DIAPHRAGM LOCATIONS SHOWN ON THE FRAMING PLAN. A TEMPORARY DIAPHRAGM IS REQUIRED AT EACH PERMANENT DIAPHRAGM LOCATION AND MAY BE OFFSET UP TO '-O' IN EITHER DIRECTION TO AVOID INTERFERENCE.

I7. CONTRACTOR SHALL SUBMIT A TEMPORARY DIAPHRAGM INSTALLATION PROCEDURE AND SHOP DRAWINGS FOR REVIEW. RESIDENT ENGINEER SHALL INSPECT TEMPORARY DIAPHRAGM INSTALLATION PRIOR TO PLACING THE DECK.

Scale Designed by: ТВ No. Revision By Date CONSULTANT PROJECT MANAGER: Craig R. Morin, P.E Date 07∖18 Date 07∖18 Βv Bу KEB Checked HJW Jesigned 07\18 In Charge of RAL 07\18 FRR Drawn

HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155 FAX (207) 228-0909



CONTRACT:2018.15

66

65-

G4-

G3-

G2-

GI

G2-

G4

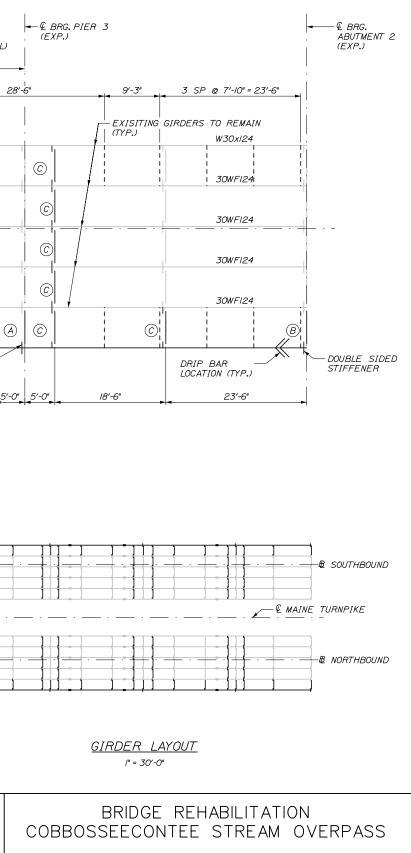
65-

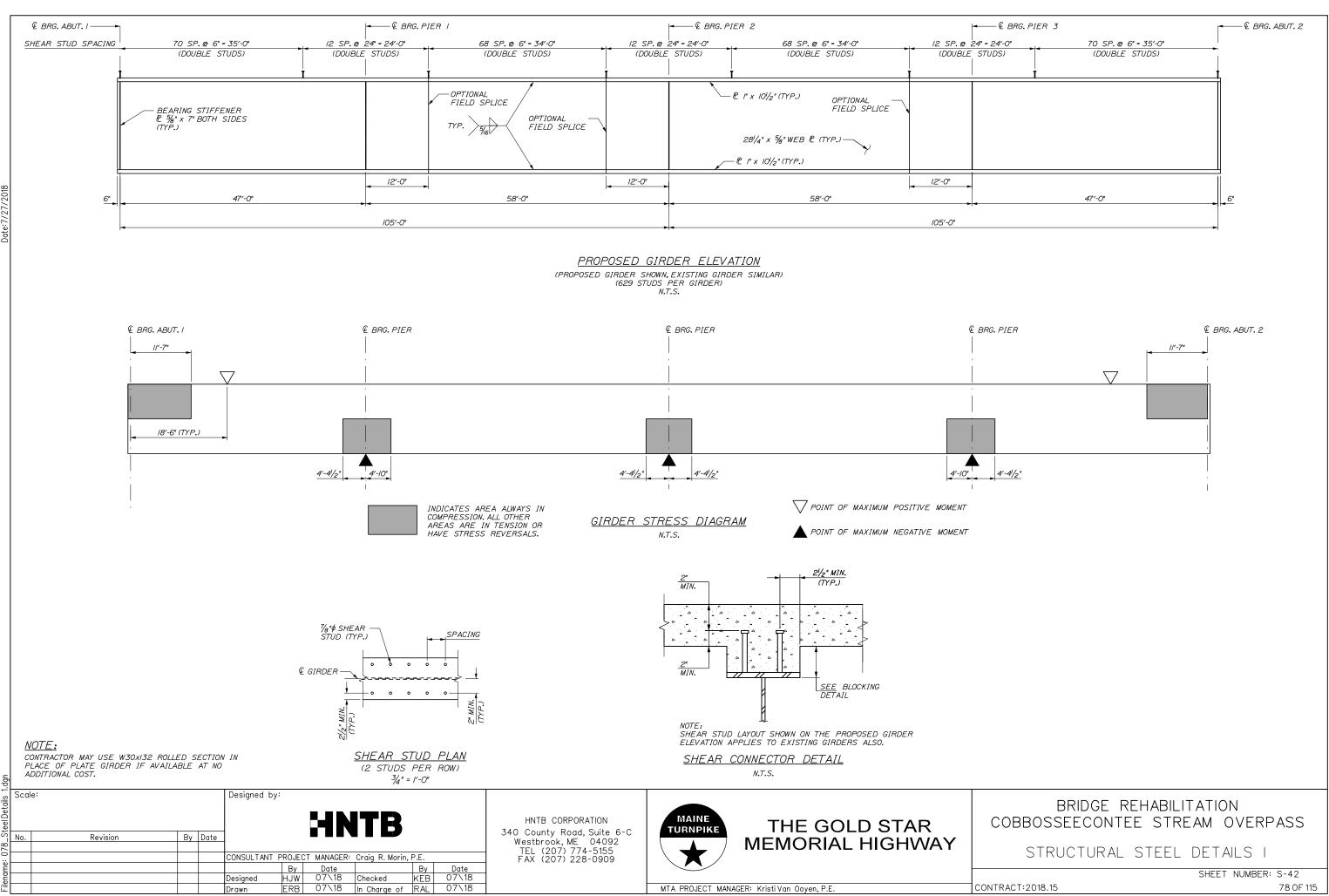
66

G3-

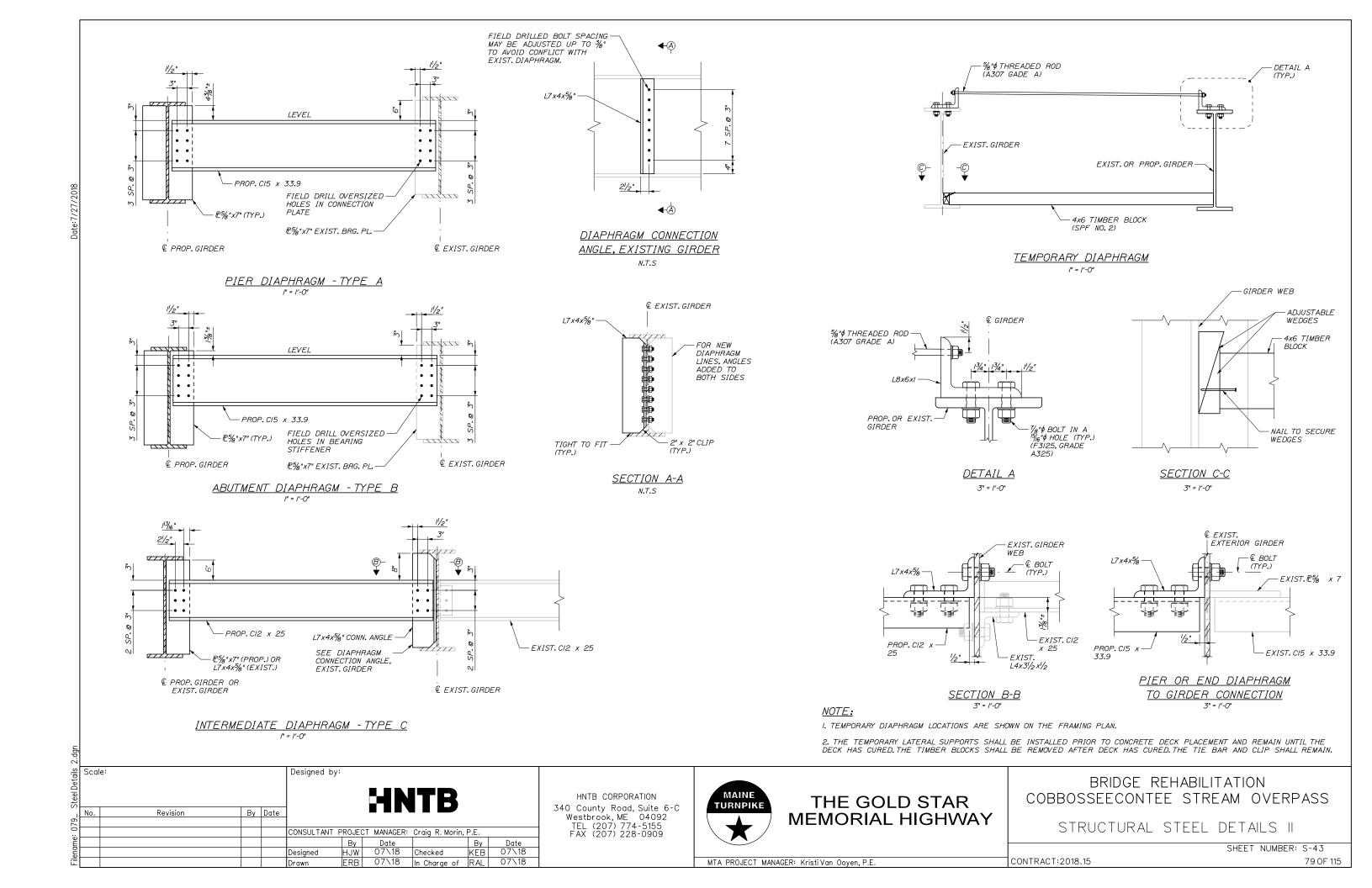
SHEET NUMBER: S-41 77 OF 115

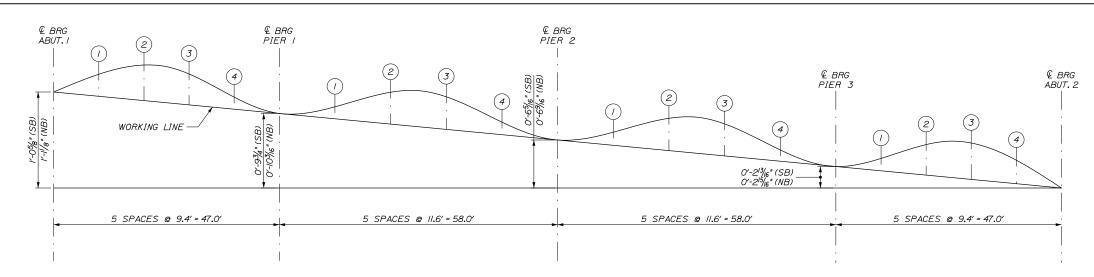
## FRAMING PLAN





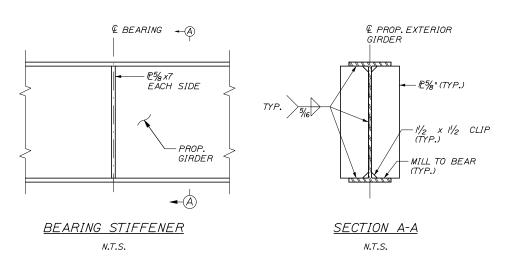
CONTRACT:2018.15





								ΤC	DTAL N	ORTHB	OUND	CAMBE	ER VA	LUES (	IN)							
		<i>€ BRG</i>		SPAN I = ·	47 <b>.</b> 0′		<i>€ BRG</i>	S	SPAN 2 =	58 <b>.</b> 0′		<i>€ BRG</i>	S	PAN 3 =	58.0′		€ BRG	5	SPAN 4 =	47.0′		<i>€ BRG</i>
		ABUT. I	/	2	3	4	PIER /	1	2	3	4	PIER 2	1	2	3	4	PIER 3	1	2	3	4	ABUT. 2
	DISTANCE FROM ABUT. I (FT)	0.000	9.400	18.800	28.200	37.600	47.000	58.600	70.200	81.800	93.400	105.000	116.600	128.200	139.800	151.400	/63.000	172.400	181.800	191.200	200.600	210.000
	STEEL DEAD LOAD	0.000	-0.020	-0.040	-0.040	-0.0/0	0.000	-0.020	-0.050	-0.050	-0.020	0.000	-0.020	-0.05	-0.050	-0.020	0.000	-0.0/0	-0.040	-0.040	-0.020	0.000
G6	CONCRETE DEAD LOAD	0.000	-0,170	-0.250	-0.200	-0.080	0.000	-0.120	-0.250	-0.240	-0,//0	0.000	-0.110	-0.240	-0,250	-0,/20	0.000	-0.080	-0.200	-0.250	-0./70	0.000
	SUPERIMPOSED DEAD LOAD	0.000	-0.040	-0.060	-0.050	-0.020	0.000	-0.020	-0.060	-0.060	-0.020	0.000	-0.020	-0.060	-0.060	-0.020	0.000	-0.020	-0.050	-0.000	-0.040	0.000
	TOTAL	0.000	0.227	0.345	0.284	0.//4	0.000	0./65	0.360	0.340	0./52	0.000	0.148	.0344	0.360	0./59	0.000	0.//4	0.280	0.339	0.229	0.000

								ΤC	DTAL S	OUTHE	OUND	CAMBE	R VA	UES (	(IN)							
		€ BRG		SPAN /=·	47 <b>.</b> 0′		€ BRG	S	SPAN 2 =	58 <b>.</b> 0′		<i>€ BRG</i>	S	PAN 3 =	58.0′		€ BRG	¢.	SPAN 4 =	47.0′		<i>€ BRG</i>
		ABUT. I	1	2	3	4	PIER /	1	2	3	4	PIER 2	1	2	3	4	PIER 3	/	2	3	4	ABUT.2
	DISTANCE FROM ABUT. I (FT)	0.000	9.400	18.800	28.200	37.600	47.000	58.600	70.200	81.800	93.400	105.000	//6.600	128.200	139.800	151.400	163.000	172.400	181.800	191.200	200.600	210.000
	STEEL DEAD LOAD	0.000	-0.020	-0.040	-0.040	-0.0/0	0.000	-0.020	-0.050	-0.050	-0.020	0.000	-0.020	-0.05	-0.050	-0.020	0.000	-0.0/0	-0.040	-0.040	-0.020	0.000
G6	CONCRETE DEAD LOAD	0.000	-0.170	-0.250	-0.200	-0.080	0.000	-0.120	-0.250	-0.240	-0.//0	0.000	-0.//0	-0.240	-0.250	-0./20	0.000	-0.080	-0.200	-0.250	-0./70	0.000
	SUPERIMPOSED DEAD LOAD	0.000	-0.040	-0.060	-0.050	-0.020	0.000	-0.020	-0.060	-0.060	-0.020	0.000	-0.020	-0.060	-0.060	-0.020	0.000	-0.020	-0.050	-0.000	-0.040	0.000
	TOTAL	0.000	0.228	0.348	0.288	0.120	0.000	0,/68	0.360	0,348	0,/56	0.000	0./56	0.348	0.360	0./68	0.000	0,120	0,288	0.348	0.228	0.000



ails 3.6	ale:		Designed by	/:							
080_ SteelDeta	. Revision	By Date				ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155		THE GOLD STAR MEMORIAL HIGHWAY
			CONSULTANT	PROJEC	T MANAGER:	Craig R. Mori	n, P.E.		FAX (207) 228-0909		
Ĕ				By	Date		By	Date			
suc			Designed	HJW	07\18	Checked	KEB	07\18			
Ĩ.			Drawn	ERB	07\18	In Charge of	RAL	07\18	]	MTA PROJECT MANA	GER: KristiVan Ooyen, P.E.

## BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

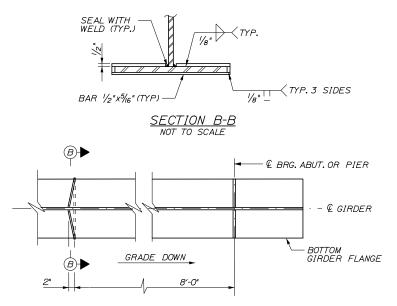
STRUCTURAL STEEL DETAILS III

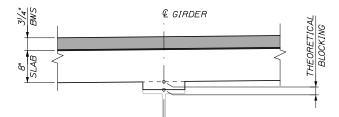
CONTRACT:2018.15

SHEET NUMBER: S-44

					NC	ORTHBO	DUND I	воттол	I OF .	SLAB	ELEVA	TIONS	AT BL	OCKIN	g poii	VTS					
	<i>€ BRG</i>		SPAN /= 4	47 <b>.</b> 0′		€ BRG	S	PAN 2 =	58.0′		<i>€ BRG</i>	S	SPAN 3 =	58 <b>.</b> 0′		<i>€ BRG</i>	S	SPAN 4 =	47.0′		€ BRG
	ABUT. I	0.2	0.4	0.6	0.8	PIER I	0.2	0.4	0.6	0.8	PIER 2	0.2	0.4	0.6	0.8	PIER 3	0.2	0.4	0.6	0.8	ABUT.
G/	147.13	147.10	147.06	147.00	146.94	146.88	146.84	146.79	146.73	/46.65	146.58	146.53	146.49	146.43	146.35	146.28	146.24	146.20	146.16	146.10	146.04
G2	147.26	147.23	147.19	147.14	147.08	147.02	146.97	146,92	146.86	146.79	146.72	/46.67	146.62	146.56	/46.49	146.42	146.38	146.34	146.29	146.24	146.17
G3	147.40	147.37	147.32	147.27	147.21	147.15	147.10	147.06	147.00	146.92	146.85	146.80	146.75	146.69	146.62	146.55	146.51	146.47	/46.43	146.37	146.3
G4	147.28	147.25	147.20	147.15	147.09	147.03	146.98	146.94	/46.88	146.80	146.73	/46.68	/46.63	146.57	146.50	146.43	/46.39	/46.35	146.31	/46.25	146.19
G5	147.14	47.	147.07	147.02	/46.96	146.90	146.85	146.80	146.74	146.67	146.60	/46.55	/46.50	146.44	146.37	146.30	146.26	146.22	146.17	146.12	146.05
G6	147.01	146.98	/46.94	146.88	146.82	146.76	146.72	146.67	146.61	146.53	/46.46	146.41	146.37	146.31	146.23	146.16	146.12	146.08	146.04	145.98	145.9

					SC	DUTHBO	DUND I	BOTTON	1 OF .	SLAB .	ELEVA	TIONS	AT BL	OCKIN	G POII	VTS					
	<i>€ BRG</i>		SPAN /=	47 <b>.</b> 0′		<i>€ BRG</i>	S	PAN 2 =	58 <b>.</b> 0′		∉ BRG	S	PAN 3 =	58.0′		<i>€ BRG</i>	Ś	SPAN 4 =	47.0′		<i>€ BRG</i>
	ABUT. I	0.2	0.4	0.6	0.8	PIER I	0.2	0.4	0.6	0.8	PIER 2	0.2	0.4	0.6	0.8	PIER 3	0.2	0.4	0.6	0.8	ABUT.2
GI	147.15	147.12	147.09	147.03	146.98	146.92	146.87	146.83	146.77	146.70	146.63	146.58	146.54	146.48	146.41	146.34	146.30	146.27	146.22	146.17	146.11
G2	147.29	147,26	147.22	147.17	47.	147.06	147.01	146.96	146.90	146.83	146.76	146.72	146.67	146.61	146.54	146.47	146.44	146,40	146.36	146.30	146.24
G3	147.42	147.39	147.35	147.30	147.24	147.19	147.14	147.10	147.04	146.97	146.90	146.85	146.81	146.75	/46.68	146.61	146.57	146.53	/46.49	146.44	146.38
G4	147.30	147.27	147.23	147.18	147.12	147.07	147.02	146.98	146.92	146.85	146.78	146.73	/46.69	146.63	/46.56	146.49	146.45	146.41	146.37	146.32	146.25
G5	147.17	147.14	147.10	147.05	146.99	146.93	146.89	/46.84	146.78	146.71	/46.64	146.60	146.55	146.49	146.42	/46.35	146.32	146.28	146.24	146.18	146.12
G6	147.03	147.00	146.97	146.91	146.86	146.80	146.75	146.71	/46.65	146.58	146.51	146.46	146.42	/46.36	146.29	146.22	146.18	146.15	146.10	146.05	145.98





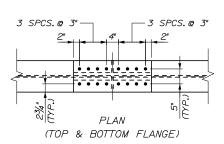
BLOCKING DETAIL THEORETICAL BLOCKING VARIES (DO NOT USE THEORETICAL BLOCKING TO SET FORMS) I" = I'-O"

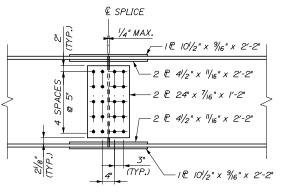
I. PRIOR TO PROFILING OF THE GIRDERS THE CONTRACTOR SHALL HAVE SET THE GIRDERS TO THEIR FINAL ELEVATION AND INSTALLED ALL NECESSARY DIAPHRAGMS AND/OR TEMPORARY BRACES NECESSARY TO HOLD THE GIRDERS IN THEIR FINAL

LINES OR SHIELDING PRIOR TO PROFILING THE GIRDERS.

2. THE CONTRACTOR SHALL INSTALL PROTECTIVE SHIELDING AND ACCEPTABLE SAFETY

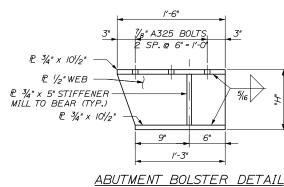
3. THEORETICAL BLOCKING HEIGHTS ARE TAKEN FROM BOTTOM OF SLAB TO TOP OF TOP FLANGE ALONG THE CENTERLINE OF GIRDER FOR ALL GIRDERS. INCLUDING THE



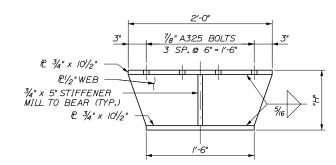


ELEVATION

OPTIONAL FIELD SPLICE 3/4 = 1'-0"



 $|l_{2}'' = l' - O''$ 



PIER BOLSTER DETAIL  $l'/_{2}" = l' - O"$ 

4.d													
ails	Sca	le:			Designed by:								
31_ SteelDet	No.	Revision	Ву	Date			ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092	MAINE TURNPIKE	THE GOLD STAR MEMORIAL HIGHWAY	
õ					CONSULTANT PROJEC	T MANAGER:	Craig R. Morin,	P.E.		TEL (207) 774-5155 FAX (207) 228-0909			
Ĕ					By	Date		By	Date				
enc					Designed HJW	07\18	Checked	KEB					
Ē					Drawn ERB	07\18	In Charge of	RAL	07\18		MTA PROJECT MAN	AGER: Kristi Van Ooyen, P.E.	]00

NOTES:

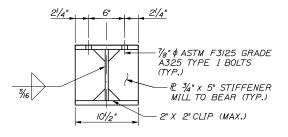
PLUMB POSITION.

NEW PLATE GIRDERS.





E	BOLSTE	R HE	IGHT T	ABLE	
٨	IORTHBOU	ND BRID	GE ("H")(	INCHES)	
GIRDER	ABUT. I	PIER I	PIER 2	PIER 3	ABUT.2
GI	/3	8	6	5	6
G2	/3	8	6	5	6
G3	/3	8	6	5	6
G4	/3	8	6	5	6
G5	/3	8	6	5	6
G6	/3	8	6	5	6
Ś	SOUTHBOU	ND BRID	GE ("H")(	INCHES)	
GIRDER	ABUT. I	PIER I	PIER 2	PIER 3	ABUT.2
GI	/3	9	8	6	7
G2	/3	9	8	6	7
G3	/3	9	8	6	7
G4	/3	9	8	6	7
G5	/3	9	8	6	7
<i>G6</i>	/3	9	8	6	7



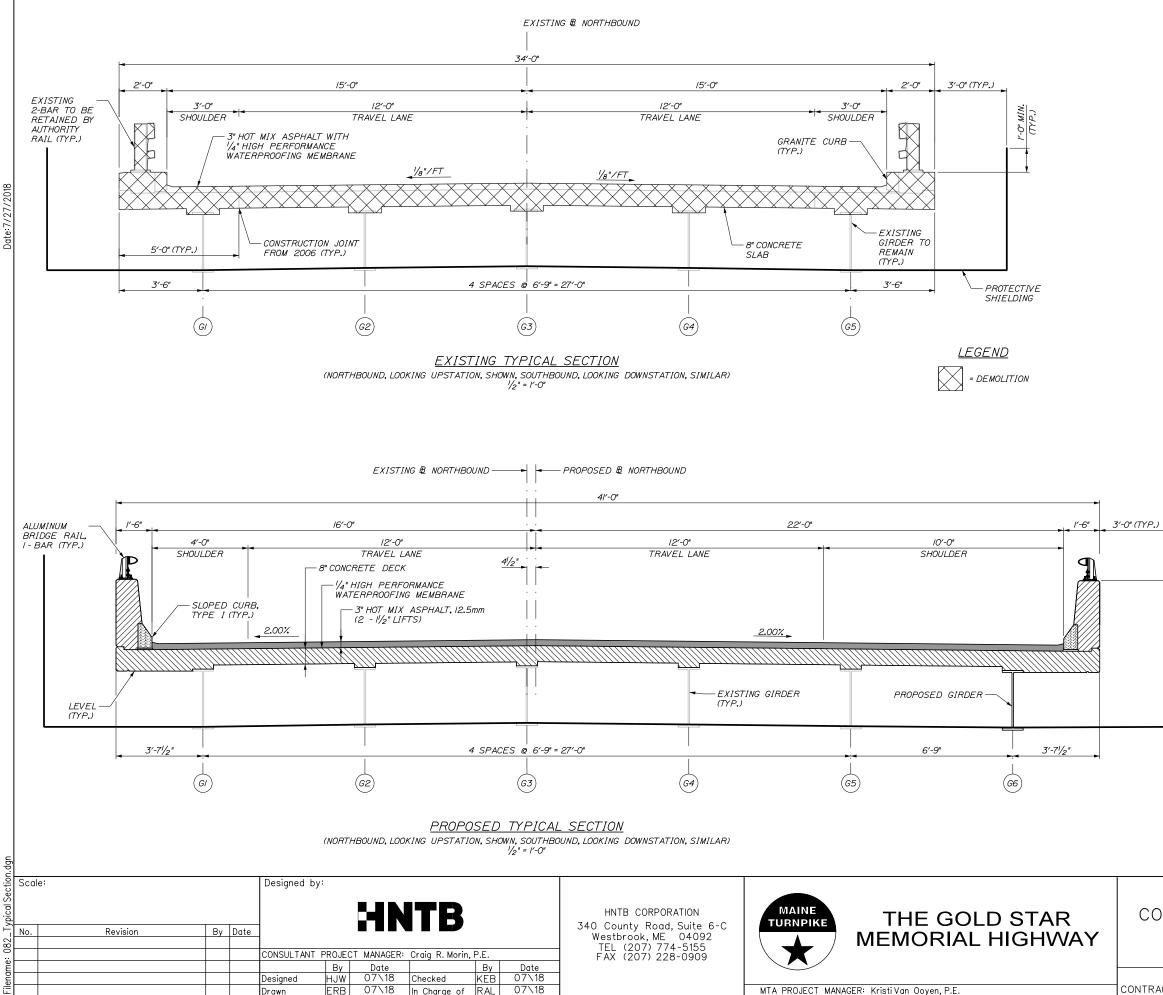
BOLSTER SECTION |//2" = |'-0"

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

STRUCTURAL STEEL DETAILS IV

CONTRACT:2018.15

SHEET NUMBER: S-45



SUPERSTRUCTURE NOTES:

I. THE USE OF PRECAST DECK PANELS IS PROHIBITED.

2. MORTAR FOR BEDDING AND FOR JOINTS IN THE GRANITE CURB SHALL CONTAIN A NON-SHRINK ADDITIVE.

3. CLEAR PROTECTIVE COATING FOR CONCRETE SURFACE SHALL BE APPLIED TO THE FOLLOWING AREAS: PARAPET SURFACES, FASCIA DOWN TO DRIP NOTCH AND ALL EXPOSED CONCRETE SURFACES ON THE END POSTS.

4. ALL BRIDGE PARAPET CONCRETE, INCLUDING INSIDE FACE, TOP FACE AND END POSTS SHALL HAVE A RUBBED FINISH PRIOR TO THE APPLICATION OF THE CLEAR PROTECTIVE COATING FOR CONCRETE SURFACE.

5. THE CONCRETE DECK SHALL BE GIVEN A SMOOTH BULL FLOAT OR WOOD FLOAT FINISH.

6. GRANITE CURB JOINTS SHALL LINE UP WITH PARAFFIN AND DUMMY JOINTS.

7. SHOP DRAWINGS FOR BAR CHAIRS USED WITH REINFORCING STEEL IN SLAB CONSTRUCTION SHALL BE SUBMITTED WITH REQUIRED SPACING TO THE RESIDENT FOR APPROVAL BAR CHAIRS SHALL BE EPOXY-COATED OR PLASTIC PROTECTED.

8. PROTECTIVE SHIELDING SHALL EXTEND LONGITUDINALLY FOR THE FULL LENGTH OF THE STRUCTURE. THE WIDTH OF THIS SHIELDING SHALL BE EQUAL TO THE TOTAL WIDTH OF THE NEW STRUCTURE PLUS THREE FEET BEYOND THE FASCIA LINES ON EACH SIDE OF THE STRUCTURE.

9. PRIOR TO INSTALLING THE PROPOSED SHEAR STUDS THE CONTRACTOR SHALL CLEAN THE GIRDER TOP FLANGE SO THAT IT IS FREE OF PAINT, INCLUDING LEAD BASED, CONCRETE DEBRIS, RUST, SCALE, OIL AND OTHER CONTAMINATES THAT WOULD ADVERSELY AFFECT THE WELDING OPERATION. ALL GRINDING SHALL BE PERFORMED IN THE LONGITUDINAL DIRECTION OF THE BEAM. PAYMENT FOR PREPARING GIRDER TOP FLANGE FOR INSTALLATION OF PROPOSED SHEAR STUDS SHALL BE INCIDENTAL TO ITEM 505.08 SHEAR CONNECTORS, REMOVAL OF THE LEAD BASED PAINT SHALL BE IN ACCORDANCE WITH SP 105.2.4.2.

IO.THE APPROXIMATE SHIELDING QUANTITIES REPRESENT THE TOTAL QUANTITY OF SHIELDING REQUIRED TO COMPLETE THE WORK, INCLUDING INITIAL INSTALLATION, REMOVAL, AND RESETTING OF SHIELDING.

II. FORM V-GROOVE ON THE FASCIA AT THE HORIZONTAL JOINT BETWEEN THE PARAPET AND SLAB.

I2. EXISTING 2-BAR RAIL SHALL BE SALVAGED AND RETAINED BY THE AUTHORITY PER SPECIAL PROVISION 202.13.



	DEMOLITION	CONSTRUCTION	TOTAL
APPROX.WIDTH OF			
NEW SHIELDING	40 FT	47 FT	87 FT
INSTALLED (FT)			
APPROX. AREA OF			
SHIELDING INSTALLED	1035 SY	1215 SY	2250 SY
(SY)			
* SEE NOTES 8 AND 10			

## BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

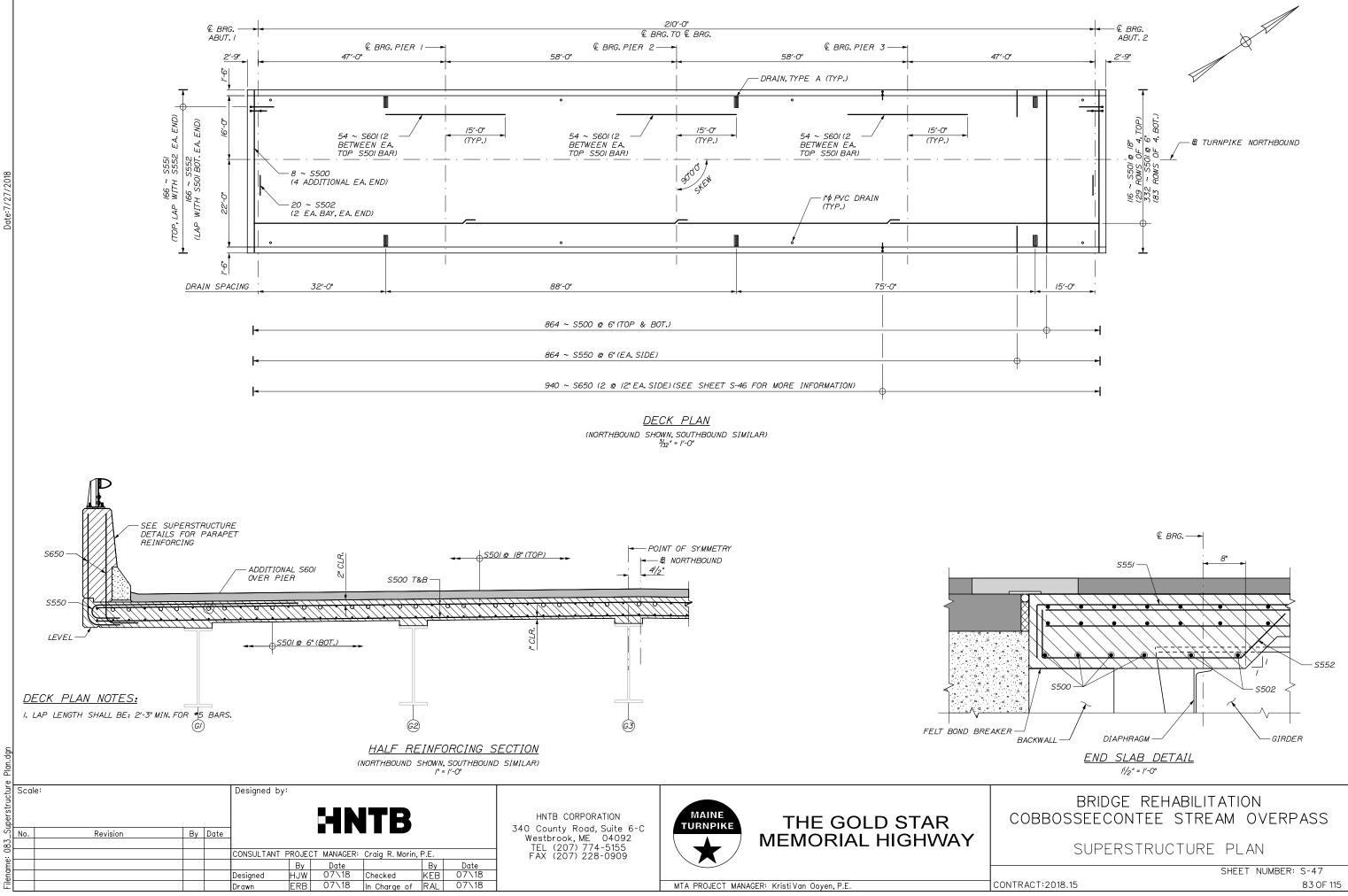
## TYPICAL SECTION

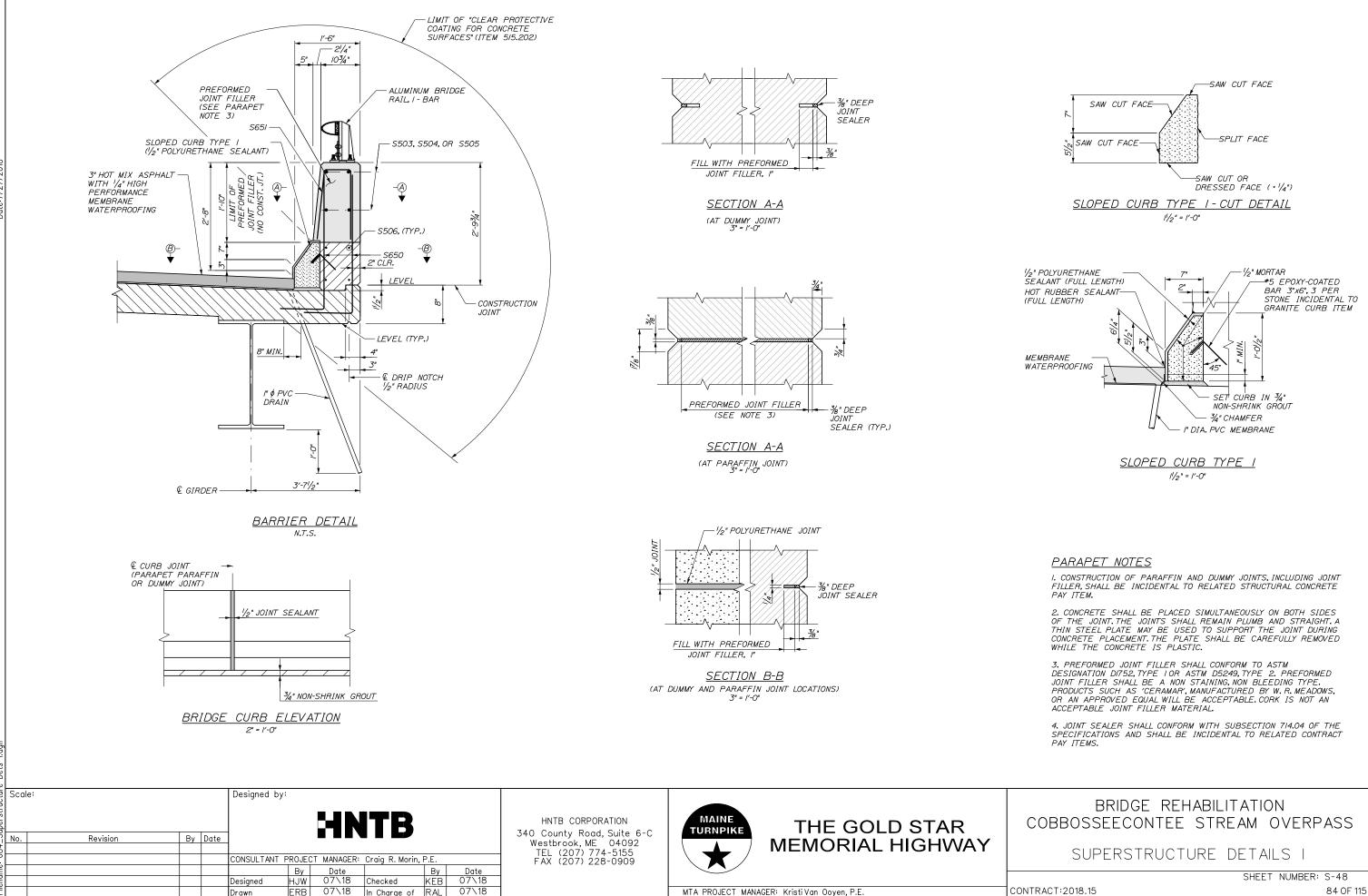
CONTRACT:2018.15

(.TYP.)

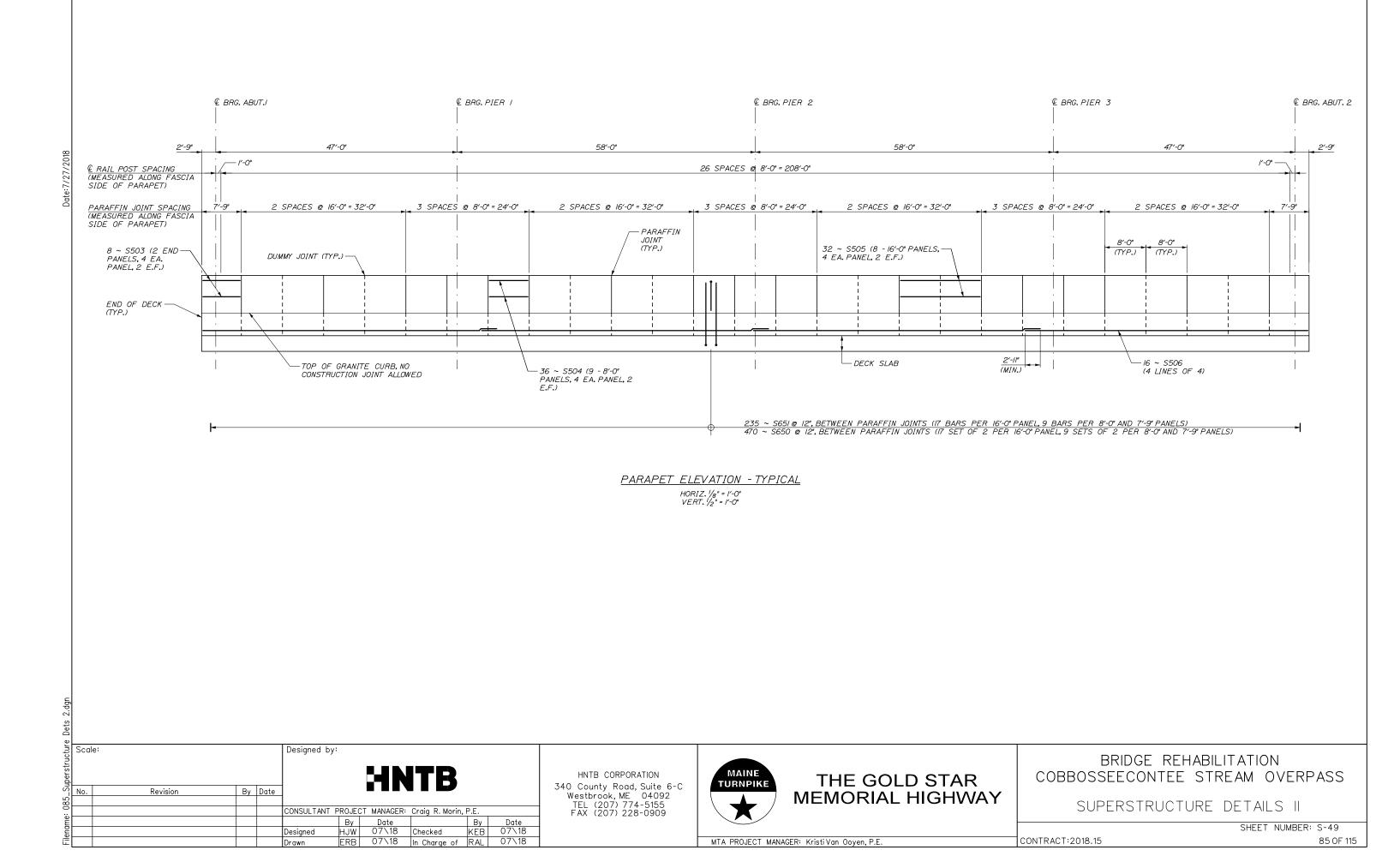
- PROTECTIVE SHIELDING

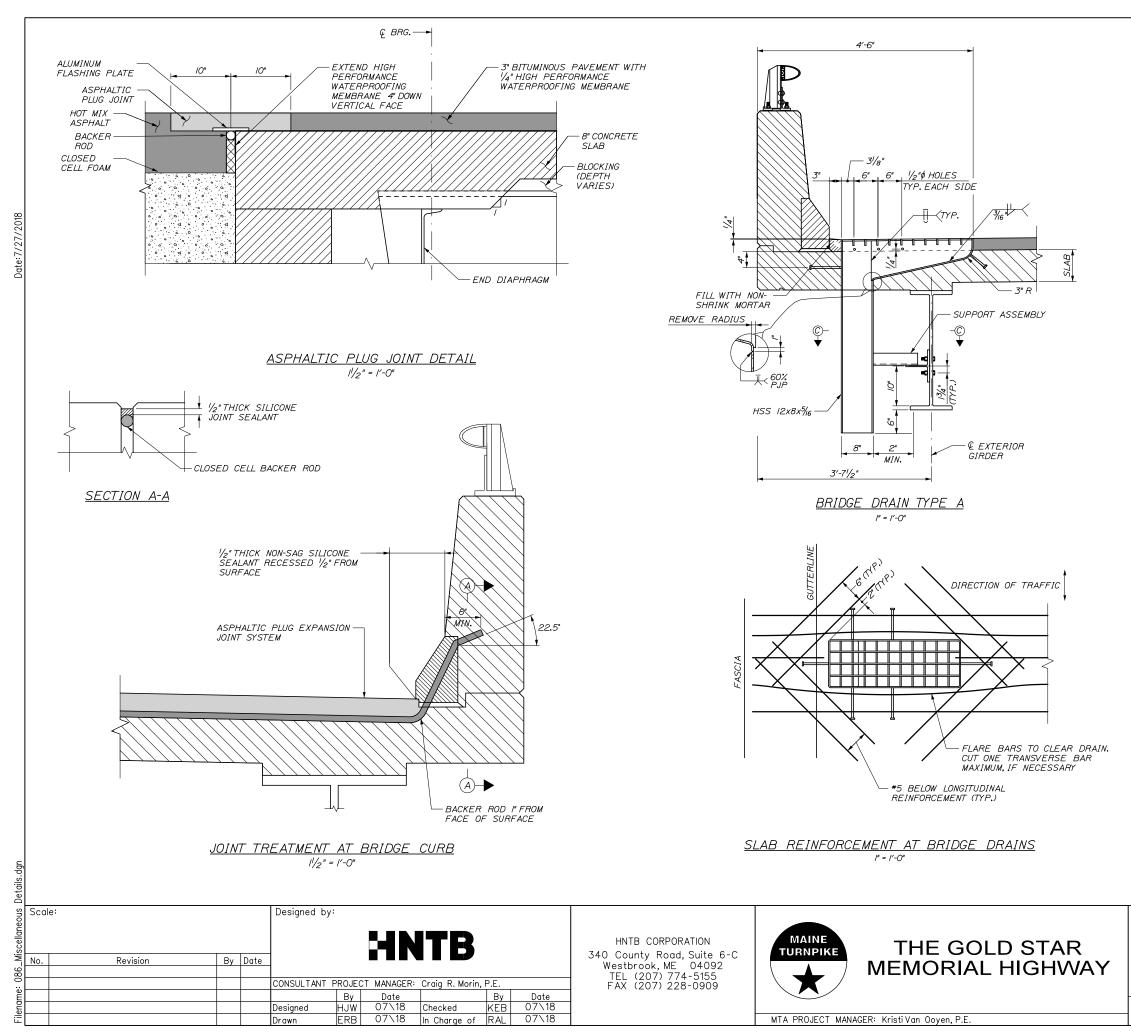
SHEET NUMBER: S-46





CONTRACT:2018.15





### NOTES:

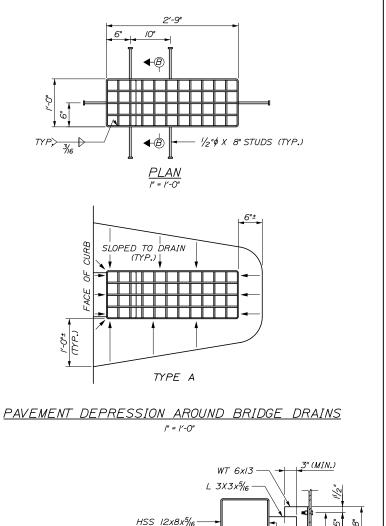
I. ASPHALTIC PLUG JOINTS SHALL BE INSTALLED AFTER THE SURFACE PAVEMENT ON THE BRIDGE AND THE APPROACHES IS COMPLETE.

2. THE ASPHALTIC PLUG JOINT SHALL BE INSTALLED AT AN AIR TEMPERATURE BETWEEN 40°F AND 60°F.

3. DECK AND BACKWALL REINFORCING NOT SHOWN FOR CLARITY.

4. THE ASPHALTIC PLUG JOINTS SHALL BE ONE OF THE SYSTEMS LISTED IN THE PROJECT SPECIFICATIONS AND SHALL BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. EQUIVALENT JOINT SYSTEMS MAY BE USED WITH APPROVAL OF THE ENGINEER.

5. IGD GALVANIZED CENTERING NAILS SHALL BE SPACED AT 12" O.C. MAXIMUM AND PLACED 2" FROM JOINTS IN THE ALUMINUM FLASHING PLATE.



BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS MISCELLANEOUS DETAILS

CONTRACT:2018.15

-11/2"x5/6" BEARING BARS

-¾" CROSS BARS

<u>SECTION B-B</u>

2" = 1'-0"

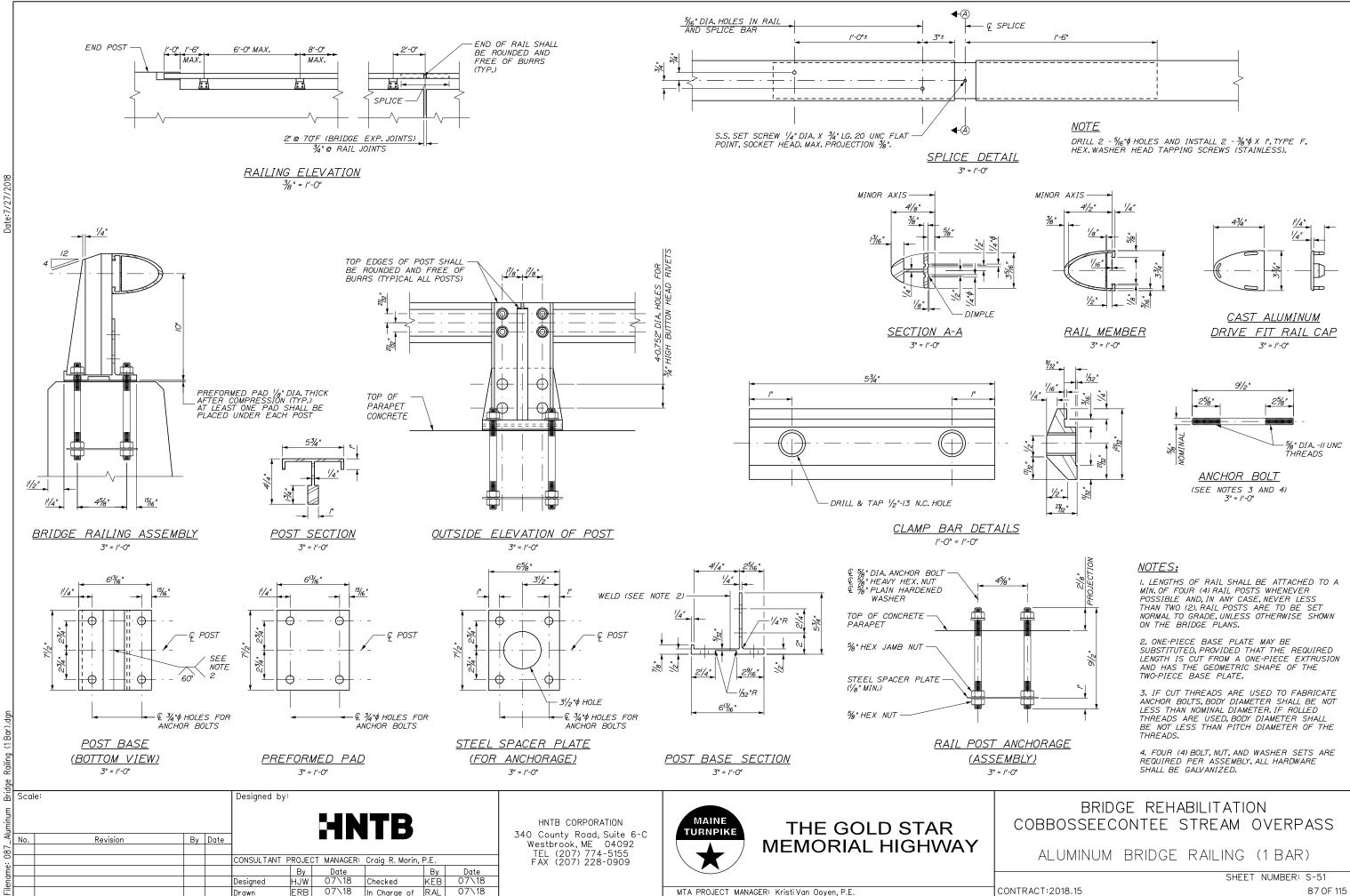
SHEET NUMBER: S-50

SECTION C-C

2" = 1'-0"

1/2"

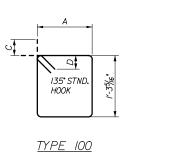
4 ~ % BOLTS



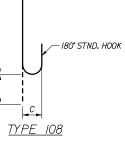
CONTRACT:2018.15

MARK           AB UTMENT           AS01W1           AS02W1           AS10W1           AS11W1           AS11W1           AS12W1           AS10W1           AS11W1           AS12W1           AS03W1           AS01W1           A601W1           A602W1           A603W1           A604W1           A604W1           A604W1           A604W1           A604W1           A604W1           A609W1           A609W1           A610W1	SIZE           NO. 1           5           5           5           5           5           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6           6	NO.           28           44           12           84           12           24           74           8           45           12           8           45           12	LENGTH 4' - 4" 5' - 0" 20' - 5" 2' - 8.5" 3' - 10.5" 5' - 10" 23' - 3" 5' - 5" 7' - 11" 7' - 9" 5' - 0" 7' - 11"	TYPE           STR           STR           118           118           129           STR           STR           STR           STR           STR           STR           STR           STR           STR           STR	A 1'-6" 2'-8" 2'-0"	B 1'-2.5" 1'-2.5" 2'-2"	С	D	E	F	REMARKS Abutment Footing (D+G) Abutment Footing Transverse
AS02W1 AS03W1 AS03W1 AS10W1 AS11W1 AS12W1 AS02W1 AS02W1 AS03W1 AS03W1 AS03W1 AS05W1 AS06W1 AS07W1 AS07W1 AS08W1 AS09W1 AS09W1 AS09W1	5 5 5 5 6 6 6 6 6 6 6 6 6 6	44 12 84 48 12 24 74 8 8 45 12 8 40	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	STR           STR           118           129           STR           STR           STR           STR           STR           STR           STR	2'-8"	1'-2.5"					•
x503W1 x510W1 x511W1 x512W1 x601W1 x602W1 x603W1 x603W1 x605W1 x605W1 x607W1 x607W1 x608W1 x609W1 x609W1 x609W1 x610W1	5 5 5 6 6 6 6 6 6 6 6 6 6 6 6	12 84 48 12 24 74 8 8 45 12 8 40	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	STR           118           118           129           STR           STR           STR           STR           STR	2'-8"	1'-2.5"					•
AS10W1 AS11W1 AS12W1 AS01W1 AS02W1 AS03W1 AS04W1 AS05W1 AS05W1 AS07W1 AS07W1 AS08W1 AS09W1 AS09W1 AS09W1 AS09W1 AS09W1	5 5 6 6 6 6 6 6 6 6 6 6 6	84 48 12 24 74 8 8 45 12 8 40	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	118 118 129 STR STR STR STR STR STR	2'-8"	1'-2.5"					
AS11W1 AS12W1 A601W1 A602W1 A602W1 A604W1 A605W1 A605W1 A607W1 A607W1 A609W1 A609W1 A609W1 A609W1	5 5 6 6 6 6 6 6 6 6 6 6 6	48 12 24 74 8 8 45 12 8 40	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	118 129 STR STR STR STR STR	2'-8"	1'-2.5"					Abutment Footing Longitudinal
A512W1 A601W1 A602W1 A602W1 A603W1 A604W1 A605W1 A605W1 A607W1 A608W1 A608W1 A609W1 A609W1	5 6 6 6 6 6 6 6 6 6 6	12 24 74 8 8 45 12 8 40	5         -         10"           23         -         3"           5'         -         5"           7'         -         11"           7'         -         9"           5'         -         0"           7'         -         11"	129 STR STR STR STR STR							Pedestal Longitudinal (D+G)
A601W1 A602W1 A603W1 A604W1 A605W1 A605W1 A606W1 A607W1 A608W1 A609W1 A610W1	6 6 6 6 6 6 6 6 6	24 74 8 8 45 12 8 40	23' - 3" 5' - 5" 7' - 11" 7' - 9" 5' - 0" 7' - 11"	STR STR STR STR	2'-0"	2'-2"					Pedestal Transverse (D+G)
A602W1           A603W1           A604W1           A605W1           A605W1           A606W1           A607W1           A608W1           A609W1           A609W1	6 6 6 6 6 6 6 6	74 8 8 45 12 8 40	5' - 5'' 7' - 11'' 7' - 9'' 5' - 0'' 7' - 11''	STR STR STR							Pedestal Stimp
A602W1           A603W1           A604W1           A605W1           A606W1           A606W1           A608W1           A608W1           A609W1           A609W1	6 6 6 6 6 6 6 6	74 8 8 45 12 8 40	5' - 5'' 7' - 11'' 7' - 9'' 5' - 0'' 7' - 11''	STR STR STR							Abutment Backwall Horizontal
A603W1 A604W1 A605W1 A606W1 A607W1 A607W1 A608W1 A609W1 A609W1	6 6 6 6 6 6 6 6	8 8 45 12 8 40	7' - 11" 7' - 9" 5' - 0" 7' - 11"	STR STR							Abutment Backwall Vertical (D+G)
A604W1 A605W1 A606W1 A607W1 A607W1 A609W1 A609W1 A610W1	6 6 6 6 6 6	8 45 12 8 40	7' - 9" 5' - 0" 7' - 11"	STR							Abutment Backwall Vertical
A606VV1 A607VV1 A608VV1 A609VV1 A609VV1	6 6 6 6	12 8 40	7' - 11"	120							Abutment Backwall Vertical
A607VV1 A608VV1 A609VV1 A610VV1	6 6 6	8 40		1 129	1'-0"	2'-0"					Abutment Backwall Top Stirrup
A608VV1 A609VV1 A610VV1	6 6 6	40		STR							Abutment Seat Horizontal (D+G)
A609VV1 A610VV1	6 6		4' - 9"	STR							Abutment Wingwall and Seat Vertical (D+G)
610W1	6	-	7' - 3"	118	6'-3"	1'-0"					Abutment Wingwall and Seat Vertical
		8	6' - 9"	118	3'-4"	3'-5"					Abutment Seat Vertical & Top
161110/1	6	12	12' - 5"	118	8'-1"	4'-4"					Abutment Seat to Wingwall Horizontal
NOTIVI	0	36	10' - 0"	118	5'-8"	4'-4"					Abutment Backwall to Wingwall Horizontal
AS501W1	5	16	36' - 8"	STR							Approach Slab Transverse
AS601W1	6	75	12' - 8"	STR							Approach Slab Longitudinal
W601W1	6	18	3' - 8"	118	2'-8"	1'-0"					Wingwall Vertical (D+G)
N602W1	6	36	13' - 2"	STR	41.07	41.07					Wingwall Horizontal
N603W1	6	16	3' - 2"	129	1'-2"	1'-0"	41.0.5"	41.0"	01.4.4"	01.44"	Wingwall Horizontal End Stirrups
N604W1	6	15	9' - 5"	122	2'-7" 4'-9"	3'-10.5"	1'-3.5"	1'-8"	0'-11"	0'-11"	Wingwall Corbel
W605W1 W606W1	6 6	15 30	8' - 1" 9' - 6"	129 118	4 -9 8'-4"	1'-8" 1'-2"					Wingwall Vertical Stirrup Wingwall Vertical
N607W1	6	2	8' - 7"	110	0-4 2'-7"	3'-0.5"	1'-3.5"	1'-8"	0'-11"	0'-11"	Wingwall Corbel
N608W1	6	2	6' - 5.5"	118	1'-8"	4'-9.5"	1-3.5	1-0	0-11	0-11	WingwallCorbel
N609W1	6	2	4' - 9"	129	1'-1"	1'-10"					Wingwall Corbel
W610W1	6	1	12' - 5"	120	1'-1"	5'-8"					Wingwall Top Horizontal
EP501W1	5	16	14' - 10"	129	13'-2"	0'-10"					Endpost Horizontal Stirrup
EP502W1	5	4	12' - 3"	119	9'-7"	2'-8.25"	2'-6"				Endpost Horizontal, Top
EP503W1	5	2	7' - 10"	118	4'-0"	3'-10"					Endpost Horizontal, Top
EP504W1	5	2	7' - 10"	138	2'-11"	0'-4"	1'-1"	3'-6"			Endpost Horizontal, Top
EP601W1	6	56	7' - 3"	STR							Endpost Vertical
EP602W1	6	56	6' - 6"	118	5'-6"	1'-0"					Endpost Vertical
EP603W1	6	56	5' - 6.75"	129	0'-6.75"	2'-6"					Endpost Top Stirrup
BUTMENT		00	41 411	OTD							
1501W2	5 5	28 44	4' - 4'' 5' - 0''	STR							Abutment Footing (D+G) Abutment Footing Transverse
4502W2 4503W2	5	44 12	5' - 0" 20' - 5"	STR STR							Abutment Footing Transverse Abutment Footing Longitudinal
4503VV2 4510W2	5	84	20 - 5"	118	1'-6"	1'-2.5"					Pedestal Longitudinal (D+G)
A510W2	5	48	3' - 10.5"	118	2'-8"	1'-2.5"					Pedestal Transverse (D+G)
4512W2	5	12	5' - 10"	129	2'-0"	2'-2"					Pedestal Stirup
A601W2	6	20	23' - 3"	STR							Abutment Backwall Horizontal
A602W2	6	74	4' - 11"	STR							Abutment Backwall Vertical (D+G)
603W2	6	8	7' - 4"	STR							Abutment Backwall Vertical
A604W2	6	8	7' - 3"	STR							Abutment Backwall Vertical
A605W2	6	45	5' - 0''	129	1'-0"	2'-0"					Abutment Backwall Top Stirrup
A606W2	6	12	7' - 11"	STR							Abutment Seat Horizontal (D+G)
A607W2	6	8	4' - 9''	STR							Abutment Wingwall and Seat Vertical (D+G)
A608W2	6	40	7' - 3"	118	6'-3"	1'-0"					Abutment Wingwall and Seat Vertical
A609W2	6	8	6' - 9''	118	3'-4"	3'-5"					Abutment Seat Vertical & Top
A610W2	6	12	12' - 5"	118	8'-1"	4'-4"					Abutment Seat to Wingwall Horizontal
A611W2	6	30	10' - 0"	118	5'-8"	4'-4"					Abutment Backwall to Wingwall Horizontal
AS501W2	5	16	36' - 8"	STR							Approach Slab Transverse
AS601W2	6	75	12' - 8"	STR							Approach Slab Longitudinal

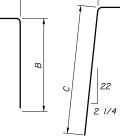
MARK	SIZE	NO.	LENGTH	TYPE	Α	в	с	D	Е	F	REMARKS
BUTMEN		110.	LENGTH				•	5			KEIIAKKO
W601W2	6	18	3' - 8"	118	2'-8"	1'-0"					Wingwall Vertical (D+G)
W602W2	6	34	13' - 2"	STR							Wingwall Horizontal
W603W2	6	15	3' - 2"	129	1'-2"	1'-0"					Wingwall Horizontal End Stirrups
W604W2	6	15	8' - 10.5"	122	2'-7"	3'-4"	1'-3.5"	1'-8"	0'-11"	0'-11"	WingwallCorbel
W605W2	6	15	7' - 7"	129	4'-3"	1'-8"					Wingwall Vertical Stirrup
W606W2	6	30	9' - 1"	118	7'-11"	1'-2"					Wingwall Vertical
W607W2	6	2	8' - 0.5"	122	2'-7"	2'-6"	1'-3.5"	1'-8"	0'-11"	0'-11"	WingwallCorbel
W608W2	6	2	5' - 11"	118	1'-8"	4'-3"					Wingwall Corbel
W609W2	6	2	4' - 9''	129	1'-1"	1'-10"					Wingwall Corbel
W610W2	6	1	12' - 5"	129	1'-1"	5'-8"					Wingwall Top Horizontal
											× .
EP501W2	5	16	14' - 10"	129	13'-2"	0'-10"					Endpost Horizontal Stirrup
EP502W2	5	4	12' - 3"	119	9'-7"	2'-8.25"	2'-6"				Endpost Horizontal, Top
EP503W2	5	2	7' - 10"	118	4'-0"	3'-10"					Endpost Horizontal, Top
EP504W2	5	2	7' - 10"	138	2'-11"	0'-4"	1'-1"	3'-6"			Endpost Horizontal, Top
EP601W2	6	56	7' - 3"	STR							Endpost Vertical
EP602W2	6	56	6' - 6"	118	5'-6"	1'-0"					Endpost Vertical
EP603W2	6	56	5' - 6.75"	129	0'-6.75"	2'-6"					Endpost Top Stirrup
SUPERSTR	RUCTURE										
S500W	5	872	40' - 8"	STR							Deck Transverse Reinforcing
S501W	5	448	55' - 6"	STR							Deck Longitudinal Reinforcing
S502W	5	20	5' - 7"	STR							Deck End Transverse Reinforcing, Thickened Slab
S503W	5	16	7' - 5"	STR							End Parapet Longitudinal Reinforcing
S504W	5	72	7' - 8''	STR							8' Parapet Longitudinal Reinforcing
S505W	5	64	15' - 8''	STR							16' Parapet Longitudinal Reinforcing
S506W	5	32	56' - 0''	STR							Deck Curb Longitudinal Reinforcing
S550W	5	864	7' - 1"	108	6'-6"	0'-7"	0'-5"				Deck Transverse Reinforcing
S551W	5	166	3' - 10"	118	3'-3"	0'-7"					Deck Thickened Slab Reinforcing
	5	166	4' - 6''	109	3'-1"	0'-7"	0'-7"	0'-10"	0'-7"		Deck Thickened Slab Reinforcing
S552W											
S552W				OTO	1						Deck Longitudinal Reinforcing
S552W	6	162	30' - 0"	STR							Deek Longittenin Reinoreng
	6	162 940	30' - 0'' 4' - 4''	118	3'-4"	1'-0"					Parapet Vertical Reinforcing



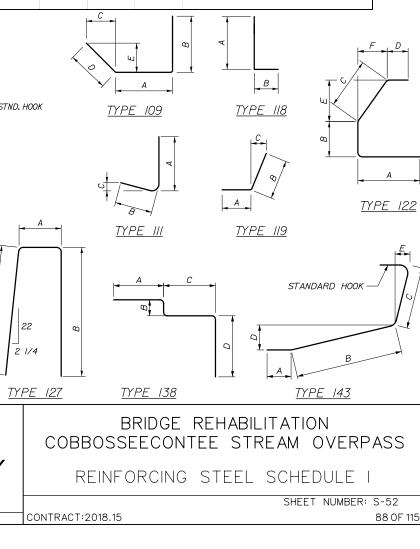
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0 В S — 180° STND. HOOK



teel S													<u>TYPE 126</u>	<u>TYPE 129</u>	<u>TY</u>
88_Reinforcing St	Scal No.	e: Revision	Ву	Date	Designed by			ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155			THE GOLD STAR MEMORIAL HIGHWA	Y
0					CONSULTANT	PROJEC	T MANAGER:	Craig R. Morin	n, P.E.		FAX (207) 228-0909		$\setminus \mathbf{X}$		
me						By	Date		By	Date			$\checkmark$		
oue					Designed	HJW	07\18	Checked	KEB	07\18					
Ē					Drawn	ERB	07\18	In Charge of	RAL	07\18		N	MTA PROJECT MANAG	GER: Kristi Van Ooyen, P.E.	



MARK PIER 1 P401W1 P501W1 P510W1 P511W1 P512W1	SIZE 4		LENGTH	TYPE		в	с	D	Е	REMARKS
2501W1 2510W1 2511W1	4	NO.	LENGTH	TTPE	A	В	C	U	E	REMARKS
2510W1 2511W1		60	3' - 6"	126	0'-6"	2'-4"	0'-8"			Pier Stem Horizontal Ties
P511W1	5	6	14' - 11"	100	4'-8"	2'-4"	0'-5.5"			Pier Footing Horizontal Stirrups
	5	84	2' - 8.5"	118	1'-6"	1'-2.5"				Pedestal Longitudinal (D+G)
P512W1	5	48	3' - 10.5"	118	2'-8"	1'-2.5"				Pedestal Transverse (D+G)
	5	12	5' - 10"	129	1'-6"	2'-2"				Pedestal Stirrup
P601W1	6	17	11' - 0"	129	2'-4"	4'-4"				Pier Stem Horizontal End Stirrups
P602W1	6	6	5' - 9"	STR						Pier Cap Horizontal Dowel
P603W1	6	4	7' - 3"	STR						Pier Hammerhead Cap Horizontal
P604W1	6	2	10' - 8''	STR						Pier Hammerhead Cap Horizontal
P605W1	6	2	13' - 3"	STR						Pier Hammerhead Cap Horizontal
P606W1	6	2	13' - 6"	STR	01.01					Pier Hammerhead Cap Horizontal
P607W1	6	30	10' - 10"	129	2'-8"	4'-1"				Pier Hammerhead Vertical Stirrups
P608W1	6	2	10' - 7" 10' - 4"	129	2'-8"	3'-11.5"				Pier Hammerhead Vertical Stirrups
P609W1	6	2		129	2'-8"	3'-10"				Pier Hammerhead Vertical Stirrups
P610W1	6 6	2	10' - 0'' 9' - 9''	129 129	2'-8" 2'-8"	3'-8"			+	Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
P611W1 P612W1	6	2	9' - 9'' 9' - 6''	129	2'-8"	3'-6.5" 3'-5"			<u> </u>	Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
P612W1 P613W1	6	2	9' - 6" 9' - 4"	129	2'-8"	3'-5" 3'-4"			<u>├</u>	Pier Hammenhead Vertical Stirrups Pier Hammenhead Vertical Stirrups
P613W1 P614W1	6	2	9 - 4	129	2-8	3-4			<u>├</u> ───	Pier Hammerhead Vertical Stirrups
P614W1 P615W1	6	2	9 - 0 8' - 9"	129	2-0	3'-0.5"			+	Pier Hammerhead Vertical Stirrups
P616W1	6	2	8' - 7"	129	2-8	2'-11.5"			+	Pier Hammerhead Vertical Stirrups
P617W1	6	2	8' - 4''	129	2-8	2-11.5			+	Pier Hammerhead Vertical Stirrups
P618W1	6	2	8' - 0''	129	2'-8"	2'-10			+	Pier Hammerhead Vertical Stirrups
P619W1	6	2	7' - 8''	120	2'-8"	2'-6"				Pier Hammerhead Vertical Stirrups
P620W1	6	4	10' - 9"	143	1'-0"	6'-3"	2'-6"	1'-6"	0'-7.5"	Pier Hammerhead End Bars
P701W1	7	18	6' - 11"	OTD.						Pier Footing Transverse Dowel
P701W1 P702W1	7	18	6' - 5"	STR STR						Pier Footing Transverse
P702W1 P703W1	7	20	7' - 8"	STR						Pier Footing Longitudinal
P704W1	7	6	12' - 4"	118	11'-2"	1'-2"				Pier Footing Vertical Hook
P705W1	7	6	13' - 4"	118	12'-2"	1'-2"				Pier Footing Vertical Hook
P706W1	7	34	6' - 11"	STR	12-2	1-2				Pier Stem Horizontal Dowel
P707W1	7	34	7' - 7"	STR						Pier Stem Horizontal
P708W1	7	6	11' - 0"	STR						Pier Stem Vertical
P709W1	7	6	10' - 0''	STR						Pier Stem Vertical
P710W1	7	4	13' - 11"	STR						Pier Stem Vertical
P711W1	7	2	13' - 8"	STR						Pier Stem Vertical
P1001W1	10	4	14' - 10"	111	13'-0"	1'-10"	5.5"			Pier Hammerhead Horizontal
P1002W1	10	4	13' - 8.5"	111	11'-10.5"	1'-10"	5.5"			Pier Hammerhead Horizontal
PIER 2										
P401W2	4	96	3' - 6"	126	0'-6"	2'-4"	0'-8"			Pier Stem Horizontal Ties
P501W2	5	6	14' - 11"	100	4'-8"	2'-4"	0'-5.5"			Pier Footing Horizontal Stimups
P510W2	5	84	2' - 8.5"	118	1'-6"	1'-2.5"	0-0.0			Pedestal Longitudinal (D+G)
P511W2	5	48	3' - 10.5"	118	2'-8"	1'-2.5"				Pedestal Transverse (D+G)
P512W2	5	12	5' - 10"	129	1'-6"	2'-2"				Pedestal Stirup
260114/2		26	11' - 0''	100	01.4"	A' A''				Pier Stem Horizontal End Stirrups
P601W2 P602W2	6 6	26 6	5' - 9"	129 STR	2'-4"	4'-4"			+	Pier Stem Horizontal End Startups Pier Cap Horizontal Dowel
P602VV2	6	4	5 - 9 7' - 3"	STR					+	Pier Cap Horizontal Dowel Pier Hammerhead Cap Horizontal
	6	4	7 - 3 10' - 8''	STR					+ +	Pier Hammerhead Cap Horizontal
	6	2	13' - 3"	STR					+	Pier Hammerhead Cap Horizontal
	6	2	13' - 6"	STR					<u> </u>	Pier Hammerhead Cap Horizontal
P605W2	6	30	10' - 10"	129	2'-8"	4'-1"				Pier Hammerhead Vertical Stirrups
P605W2 P606W2		2	10' - 7"	129	2'-8"	3'-11.5"				Pier Hammerhead Vertical Stirrups
P605W2 P606W2 P607W2	6	2	10' - 4''	129	2'-8"	3'-10"				Pier Hammerhead Vertical Stirrups
P605W2 P606W2 P607W2 P608W2	6		10' - 0''	129	2'-8"	3'-8"				Pier Hammerhead Vertical Stirrups
P605W2 P606W2 P607W2 P608W2 P608W2		2	10 - 0		2'-8"	3'-6.5"				Pier Hammerhead Vertical Stirrups
P604W2 P605W2 P606W2 P607W2 P608W2 P609W2 P609W2 P610W2 P611W2	6	2	9' - 9"	129	2-0		1	1	1	
P605W2 P606W2 P607W2 P608W2 P609W2 P609W2 P610W2	6 6			129 129	2-0	3'-5"				Pier Hammerhead Vertical Stirrups
P605W2 P606W2 P607W2 P608W2 P609W2 P609W2 P610W2 P611W2	6 6 6	2	9' - 9''			3'-5" 3'-4"				Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
Peoo5W2           Peoo6W2           Peoo7W2           Peoo8W2           Peoo9W2           Peoo9W2 <td< td=""><td>6 6 6 6</td><td>2 2</td><td>9' - 9'' 9' - 6''</td><td>129</td><td>2'-8"</td><td></td><td></td><td></td><td></td><td>· · · · · · · · · · · · · · · · · · ·</td></td<>	6 6 6 6	2 2	9' - 9'' 9' - 6''	129	2'-8"					· · · · · · · · · · · · · · · · · · ·
P605W2 P606W2 P607W2 P608W2 P609W2 P609W2 P610W2 P611W2 P612W2	6 6 6 6 6	2 2 2	9' - 9'' 9' - 6'' 9' - 4''	129 129	2'-8" 2'-8"	3'-4"				Pier Hammerhead Vertical Stirrups
2605W2 2606W2 2607W2 2608W2 2609W2 2610W2 2611W2 2611W2 2612W2 2613W2 2614W2	6 6 6 6 6	2 2 2 2	9' - 9" 9' - 6" 9' - 4" 9' - 0"	129 129 129	2'-8" 2'-8" 2'-8"	3'-4" 3'-2"				Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
2605W2 2606W2 2607W2 2608W2 2609W2 2610W2 2611W2 2611W2 2612W2 2613W2 2613W2 2614W2 2615W2	6 6 6 6 6 6	2 2 2 2 2 2	9'       -       9''         9'       -       6''         9'       -       4''         9'       -       0''         8'       -       9''	129 129 129 129	2'-8" 2'-8" 2'-8" 2'-8"	3'-4" 3'-2" 3'-0.5"				Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
>>>>>>>>>>>>>>>>>>>>>>>>>>>>	6 6 6 6 6 6 6	2 2 2 2 2 2 2 2 2	9'       -       9''         9'       -       6''         9'       -       4''         9'       -       0''         8'       -       9''         8'       -       7''	129 129 129 129 129 129	2'-8" 2'-8" 2'-8" 2'-8" 2'-8"	3'-4" 3'-2" 3'-0.5" 2'-11.5"				Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups

MARK	SIZE	NO.	LENGTH	TYPE		в	с	D	E	DEMARKS
PIER 2	SIZE	NU.	LENGIN	TTPE	Α	D	ر.	U	<b>E</b>	REMARKS
P701W2	7	20	6' - 11"	STR						Pier Footing Transverse Dowel
202W2	7	20	6' - 5"	STR						Pier Footing Transverse
P703W2	7	38	8' - 8''	STR						Pier Footing Longitudinal
704W2	7	6	12' - 4"	118	11'-2"	1'-2"				Pier Footing Vertical Hook
P705W2	7	6	13' - 4"	118	12'-2"	1'-2"				Pier Footing Vertical Hook
P706W2	7	52	6' - 11"	STR						Pier Stem Horizontal Dowel
2707W2	7	52	7' - 7"	STR						Pier Stem Horizontal
P708W2	7	6	19' - 10''	STR						Pier Stem Vertical
P709W2	7	6	18' - 10''	STR						Pier Stem Vertical
710W2	7	4	22' - 8"	STR						Pier Stem Vertical
711W2	7	2	22' - 6"	STR						Pier Stem Vertical
P1001W2	10	4	14' - 10''	111	13'-0"	1'-10"	5.5"			Pier Hammerhead Horizontal
P1002W2	10	4	13' - 8.5"	111	11'-10.5"	1'-10"	5.5"			Pier Hammerhead Horizontal
PIER 3										
P401W3	4	104	3' - 6"	126	0'-6"	2'-4"	0'-8"			Pier Stem Horizontal Ties
P501W3	5	6	14' - 11"	100	4'-8"	2'-4"	0'-5.5"			Pier Footing Horizontal Stirrups
P510W3	5	84	2' - 8.5"	118	1'-6"	1'-2.5"				Pedestal Longitudinal (D+G)
P511W3	5	48	3' - 10.5"	118	2'-8"	1'-2.5"				Pedestal Transverse (D+G)
P512W3	5	12	5' - 10"	129	1'-6"	2'-2"				Pedestal Stimp
P601W3	6	28	11' - 0"	129	2'-4"	4'-4"				Pier Stem Horizontal End Stirrups
P602W3	6	6	5' - 9''	STR						Pier Cap Horizontal Dowel
P603W3	6	4	7' - 3"	STR						Pier Hammerhead Cap Horizontal
2604W3	6	2	10' - 8''	STR						Pier Hammerhead Cap Horizontal
2605W3	6	2	13' - 3"	STR						Pier Hammerhead Cap Horizontal
P606W3	6	2	13' - 6"	STR						Pier Hammerhead Cap Horizontal
P607W3	6	30	10' - 10''	129	2'-8"	4'-1"				Pier Hammerhead Vertical Stirrups
P608W3	6	2	10' - 7''	129	2'-8"	3'-11.5"				Pier Hammerhead Vertical Stirrups
P609W3	6	2	10' - 4''	129	2'-8"	3'-10"				Pier Hammerhead Vertical Stirrups
P610W3	6	2	10' - 0''	129	2'-8"	3'-8"				Pier Hammerhead Vertical Stirrups
P611W3	6	2	9' - 9''	129	2'-8"	3'-6.5"				Pier Hammerhead Vertical Stirrups
P612W3	6	2	9' - 6''	129	2'-8"	3'-5"				Pier Hammerhead Vertical Stirrups
P613W3	6	2	9' - 4''	129	2'-8"	3'-4"				Pier Hammerhead Vertical Stirrups
P614W3	6	2	9' - 0''	129	2'-8"	3'-2"				Pier Hammerhead Vertical Stirrups
P615W3	6	2	8' - 9''	129	2'-8"	3'-0.5"				Pier Hammerhead Vertical Stirrups
P616W3	6	2	8' - 7''	129	2'-8"	2'-11.5"				Pier Hammerhead Vertical Stirrups
2617W3	6	2	8' - 4''	129	2'-8"	2'-10"			L	Pier Hammerhead Vertical Stirrups
2618W3	6	2	8' - 0''	129	2'-8"	2'-8"			L	Pier Hammerhead Vertical Stirrups
P619W3	6	2	7' - 8"	129	2'-8"	2'-6"				Pier Hammerhead Vertical Stirrups
2620W3	6	4	10' - 9"	143	1'-0"	6'-3"	2'-6"	1'-6"	0'-7.5"	Pier Hammerhead End Bars
	<u> </u>								L	
P701W3	7	18	6' - 11"	STR						Pier Footing Transverse Dowel
P702W3	7	18	6' - 5"	STR					L	Pier Footing Transverse
2703W3	7	20	7' - 8"	STR						Pier Footing Longitudinal
2704W3	7	6	12' - 4"	118	11'-2"	1'-2"				Pier Footing Vertical Hook
705W3	7	6	13' - 4"	118	12'-2"	1'-2"			<b>↓</b>	Pier Footing Vertical Hook
706W3	7	56	6' - 11"	STR						Pier Stem Horizontal Dowel
707W3	7	56	7' - 7"	STR					<u>↓</u>	Pier Stem Horizontal
708W3	7	6	22' - 2"	STR					L	Pier Stem Vertical
709W3	7	6	21' - 2"	STR						Pier Stem Vertical
P710W3	7	4	25' - 0"	STR					L	Pier Stem Vertical
P711W3	7	2	24' - 10"	STR						Pier Stem Vertical
					ļ					
P1001W3	10	4	14' - 10''	111	13'-0"	1'-10"	5.5"			Pier Hammerhead Horizontal
P1002W3	10	4	13' - 8.5"	111	11'-10.5"	1'-10"	5.5"		1	Pier Hammerhead Horizontal

ing Steel Schedule 2.dgn

0ate:7/27/2018

Sca	le:				Designed b	y:					
No.		Revision	Ву	Date	-			ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092
					CONSULTANT	PROJEC	T MANAGER:	Craig R. Morin,	P.E.		TEL (207) 774-5155 FAX (207) 228-0909
						By	Date		By	Date	
					Designed	HJW	07\18	Checked	KEB	07\18	
					Drawn	ERB	07\18	In Charge of	RAL	07\18	



## THE GOLD STAR MEMORIAL HIGHWAY

MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

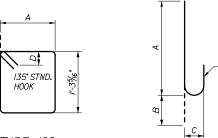
REINFORCING STEEL SCHEDULE II

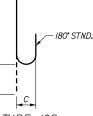
CONTRACT:2018.15

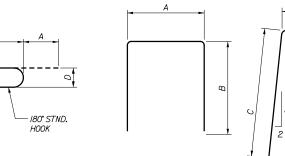
SHEET NUMBER: S-53

MARK	SIZE	NO.	LENGTH	TYPE	А	В	с	D	E	F	REMARKS
ABUTMENT	NO. 1										
A501E1	5	28	4' - 4"	STR							Abutment Footing (D+G)
A502E1	5	44	5' - 0"	STR							Abutment Footing Transverse
A503E1	5	12	20' - 5"	STR							Abutment Footing Longitudinal
A510E1	5	84	2' - 8.5"	118	1'-6"	1'-2.5"					Pedestal Longitudinal (D+G)
A511E1	5	48	3' - 10.5"	118	2'-8"	1'-2.5"					Pedestal Transverse (D+G)
A512E1	5	12	5' - 10"	129	2'-0"	2'-2"					Pedestal Stirrup
A601E1	6	24	23' - 3"	STR							Abutment Backwall Horizontal
A602E1	6	74	5' - 5"	STR							Abutment Backwall Vertical (D+G)
A603E1	6	8	7' - 11"	STR							Abutment Backwall Vertical
A604E1	6	8	7' - 9"	STR							Abutment Backwall Vertical
A605E1	6	45	5' - 0"	129	1'-0"	2'-0"					Abutment Backwall Top Stirrup
A606E1	6	12	7' - 11"	STR							Abutment Seat Horizontal (D+G)
A607E1	6	8	4' - 9"	STR							Abutment Wingwall and Seat Vertical (D+G)
A608E1	6	40	7' - 3"	118	6'-3"	1'-0"					Abutment Wingwall and Seat Vertical
A609E1	6	8	6' - 9"	118	3'-4"	3'-5"					Abutment Seat Vertical & Top
A610E1	6	12	12' - 5"	118	8'-1"	4'-4"					Abutment Seat to Wingwall Horizontal
A611E1	6	36	10' - 0"	118	5'-8"	4'-4"					Abutment Backwall to Wingwall Horizontal
AS501E1	5	16	36' - 8"	STR							Approach Slab Transverse
AS601E1	6	75	12' - 8"	STR							Approach Slab Longitudinal
W601E1	6	18	3' - 8"	118	2'-8"	1'-0"					Wingwall Vertical (D+G)
W602E1	6	36	13' - 2"	STR							Wingwall Horizontal
N603E1	6	16	3' - 2"	129	1'-2"	1'-0"					Wingwall Horizontal End Stirrups
N604E1	6	15	9' - 5"	122	2'-7"	3'-10.5"	1'-3.5"	1'-8"	0'-11"	0'-11"	Wingwall Corbel
W605E1	6	15	8' - 1"	129	4'-9"	1'-8"					Wingwall Vertical Stimup
W606E1	6	30	9' - 6"	118	8'-4"	1'-2"					Wingwall Vertical
W607E1	6	2	8' - 7"	122	2'-7"	3'-0.5"	1'-3.5"	1'-8"	0'-11"	0'-11"	Wingwall Corbel
W608E1	6	2	6' - 5.5"	118	1'-8"	4'-9.5"					Wingwall Corbel
W609E1	6	2	4' - 9"	129	1'-1"	1'-10"					Wingwall Corbel
W610E1	6	1	12' - 5"	129	1'-1"	5'-8"					Wingwall Top Horizontal
EP501E1	5	16	14' - 10''	129	13'-2"	0'-10"					Endpost Horizontal Stirrup
EP502E1	5	4	12' - 3"	119	9'-7"	2'-8.25"	2'-6"				Endpost Horizontal, Top
EP503E1	5	2	7' - 10"	118	4'-0"	3'-10"					Endpost Horizontal, Top
EP504E1	5	2	7' - 10"	138	2'-11"	0'-4"	1'-1"	3'-6"			Endpost Horizontal, Top
EP601E1	6	56	7' - 3"	STR							Endpost Vertical
EP602E1	6	56	6' - 6"	118	5'-6"	1'-0"					Endpost Vertical
EP603E1	6	56	5' - 6.75"	129	0'-6.75"	2'-6"					Endpost Top Stirrup
											* * *
		200	4' - 4"	CTD.							Abut $E_{2} = f_{12} = f_{12}$
A501E2 A502E2	5 5	28 44	<u>4 - 4</u> 5' - 0"	STR STR						├	Abutment Footing (D+G) Abutment Footing Transverse
4502E2 4503E2	5	12	20' - 5"	STR						├	Abutment Footing Longitudinal
4503E2 4510E2	5	12 84	20' - 5"	118	1'-6"	1'-2.5"					Pedestal Longitudinal (D+G)
A510E2	5	48	2' - 8.5" 3' - 10.5"	118	2'-8"	1'-2.5"					Pedestal Longmianal (D+G) Pedestal Transverse (D+G)
A512E2	5	12	5' - 10"	129	2'-0"	2'-2"					Pedestal Stirup
A601E2	6	20	23' - 3"	STR							Abutment Backwall Horizontal
4602E2	6	74	4' - 9"	STR							Abutment Backwall Vertical (D+G)
4603E2	6	8	7' - 3"	STR							Abutment Backwall Vertical
4604E2	6	8	7' - 2"	STR							Abutment Backwall Vertical
4605E2	6	45	5' - 0"	129	1'-0"	2'-0"					Abutment Backwall Ventear Abutment Backwall Top Stirrup
4606E2	6	12	7' - 11"	STR	1-0	2-0					Abutment Seat Horizontal (D+G)
4607E2	6	8	4' - 9"	STR							Abument Wingwall and Seat Vertical (D+G)
4608E2	6	40	7' - 3"	118	6'-3"	1'-0"					Abutment Wingwall and Seat Vertical
A609E2	6	8	6' - 10"	118	3'-5"	3'-5"					Abument Seat Vertical & Top
4610E2	6	12	12' - 5"	118	8'-1"	4'-4"					Abutment Seat to Wingwall Horizontal
A611E2	6	30	10' - 0"	118	5'-8"	4-4				+	Abutment Backwall to Wingwall Horizontal
											v
AS501E2	5	16	36' - 8"	STR							Approach Slab Transverse
AS601E2	6	75	12' - 8"	STR	-						Approach Slab Longitudinal

$\frac{VTMET W 0}{VTMET W 0} \frac{1}{VT} = \frac{1}{VT} $	COBBOSSEECONTEE NB BRIDGE WIN# 2018.15											
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	MARK	SIZE	NO.	LENGTH	TYPE	А	в	с	D	Е	F	REMARKS
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		NO. 2			440		41.01					
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		-				2'-8"	1'-0"					
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	/603E2					1'-2"	1'-0"					
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	604E2				-			1'-3.5"	1'-8"	0'-11"	0'-11"	<u> </u>
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	605E2				129							Wingwall Vertical Stirup
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	/606E2											
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	607E2							1'-3.5"	1'-8"	0'-11"	0'-11"	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		-										
Site												6
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				12 = 5	123	1.51	5-0					
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	501E2	5	16	14' - 10''	129	13'-2"	0'-10"					Endpost Horizontal Stirrup
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	502E2	5	4	12' - 3"	119	9'-7"	2'-8.25"	2'-6"				
Bit D       Bit D       Str       Bit D       Str       Bit D	503E2	5	2	7' - 10''	118	4'-0"	3'-10"					Endpost Horizontal, Top
NUMBER       0       80       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0 </td <td>504E2</td> <td>5</td> <td>2</td> <td>7' - 10"</td> <td>138</td> <td>2'-11"</td> <td>0'-4"</td> <td>1'-1"</td> <td>3'-6"</td> <td></td> <td></td> <td>Endpost Horizontal, Top</td>	504E2	5	2	7' - 10"	138	2'-11"	0'-4"	1'-1"	3'-6"			Endpost Horizontal, Top
Desc       0       0       0'''       110       5'''       1'''       1'''       1'''       1''''       1'''''       1''''''''''''''''''''''''''''''''''''		-										
$\begin{array}{c c c c c c c c c c c c c c c c c c c $						51.01	41.01					<u>^</u>
PERSUCTIVE       Image: Construction of the second of the se												*
etc       5       0.82       0.97       0.97       0.97       0.06       Dock Long-Mail Reducting         best       5       0.9       0.77       578       1       1       Dock Long-Mail Reducting         best       5       1.9       5.7.7       578       1       1       Dock Long-Mail Reducting         best       5       1.9       7.7.9       578       1       1       Dock Long-Mail Reducting         best       5       1.9       5.7.9       578       1       1       Dock Long-Mail Reducting         best       5       1.9       5.7.9       578       1       0.77       0.77       0.77       Dock Long-Mail Reducting         best       5       1.9       5.7.9       178       0.77       0.77       0.77       Dock Long-Mail Reducting         best       5       1.9       5.7.9       118       5.7.9       1.9       0.7       Dock Long-Mail Reducting         best Dist       5.7.9       1.9       7.7       0.7       0.7       0.7       Dock Long-Mail Reducting       Dock Long-Mail Reducting         best Dist       5.7.9       1.9       0.7       0.7       0.7       Dock Long-Mail Reducting       Do	03E2	6	50	5 - 0.75	129	0-6.75	2-6					Endpost Top Stirnip
OF       5       642       57       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67       67 <th< td=""><td>PERSTR</td><td>UCTURE</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></th<>	PERSTR	UCTURE										
VIE       S       440       SS ¹ (-P)       STR       Image: Complete Redexcination         SSR       6       6       7: -9       STR       Image: Complete Redexcination         SSR       6       7: -9       STR       Image: Complete Redexcination         SSR       6       6       7: -9       STR       Image: Complete Redexcination         SSR       6       6       7: -9       STR       Image: Complete Redexcination         SSR       6       6       7: -9       STR       Image: Complete Redexcination         SSR       6       6       9: -9       STR       Image: Complete Redexcination         SSR       6       6       7: -9       STR       Image: Complete Redexcination         SSR       6       6       0       7: -9       STR       Image: Complete Redexcination         SSR       7: -9       118       3: -9       1: -9       0: -9       Image: Complete Red       Str         SSR       100       7: -9       118       3: -9       1: -9       0: -9       Image: Complete Red       The Complete Red         SSR       100       112       1: -9       0: -9       1: -9       1: -9       1: -9       1: -9				40' - 8"	STR							Deck Transverse Reinforcing
REF       S       DO       SI-IT       SIRE       Description       Des												
$\frac{38}{160} + \frac{1}{12} + \frac{1}{12$												
GER       5       64       10: 0*       517       10       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0*       10: 0* <td></td> <td>· · · · ·</td>												· · · · ·
$\frac{88}{100} = \frac{5}{3}  \frac{32}{40}  \frac{37}{5} \cdot \frac{17}{10}  \frac{378}{100}  \frac{1}{600}  \frac{37}{5} \cdot \frac{17}{100}  \frac{1}{100}  \frac{37}{6}  \frac{9}{0} \cdot \frac{7}{0}  \frac{9}{0} \cdot \frac{7}{0}  \frac{1}{0}  \frac{1}{100}  \frac{1}{0} \cdot \frac{1}{100}  \frac{1}{0} \cdot \frac{1}{100}  \frac{1}{0} \cdot \frac{1}{100}  \frac{1}{0} \cdot \frac{1}{0}  \frac{1}{0} \cdot \frac{1}{100}  \frac{1}{0} \cdot \frac{1}{0}  \frac{1}{0} \cdot \frac{1}{0}  \frac{1}{0} \cdot \frac{1}{100}  \frac{1}{0} \cdot \frac{1}{0}  \frac{1}{0} \cdot \frac{1}{0}  \frac{1}{0} \cdot \frac{1}{0}  1$	04E											
$\frac{96}{28} \frac{5}{5} \frac{864}{100} \frac{7}{7} \frac{17}{10} \frac{100}{100} \frac{8}{9} \frac{4}{9} \frac{7}{10} \frac{7}{10} \frac{9}{7} \frac{9}{0} \frac{7}{0} \frac{10}{7} \frac{9}{0} \frac{7}{0} \frac{10}{7} \frac{9}{0} \frac{10}{7} \frac{10}{0} \frac{10}{7} \frac{10}{0} \frac{10}{7} \frac{10}{10} \frac{10}$	05E											
$\frac{116}{100} \frac{5}{100} \frac{110}{100} \frac{116}{100} \frac{3}{3} \frac{1}{10} \frac{0.7}{0.7} \frac{0.7}{0.7} \frac{0.7}{0.10} \frac{1}{0.7} \frac{1}$		-				01.0"	0.7"					
$\frac{587}{100} = 5  100  100  3.4^{\circ}  0.7  0.7  0.7  0.7  0.7  0.6^{\circ}  0.7  0.6^{\circ}  0.7  0.6^{\circ}  0.7  0.6^{\circ}  0.7  0.6^{\circ}  0.6^{\circ}  0.7  0.7  0.6^{\circ}  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.7  0.$								0'-5"	+			8
$\frac{1}{126} \frac{6}{6} \frac{1}{126} \frac{1}{2} \frac{1}{2} \frac{1}{12} $								0'-7"	0'-10"	0'-7"	-	×
$\frac{392}{126} = 0 440 \frac{4! \cdot 4''}{127} \frac{118}{1-0''} \frac{3\cdot 4''}{127} \frac{1}{1-0''} \frac{1}{1-0''$		Ť			1	~ ~ ~	- · ·	<u>,</u>			1	
$\frac{392}{126} = 0 440 \frac{4! \cdot 4''}{127} \frac{118}{1-0''} \frac{3\cdot 4''}{127} \frac{1}{1-0''} \frac{1}{1-0''$	01E	6	162	30' - 0''	STR		1		1			Deck Longitudinal Reinforcing
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	50E					3'-4"	1'-0"					
A A B B C C C C C C C C C C C C C	51E	6	470	3' - 6"	127	1'-6"	0'-6"	1'-6"				Parapet Top Reinforcing
HOOK HOOK TYPE 126 TYPE 129 TYPE 127 TYPE 138 TYPE 138 TYPE 143 BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS REINFORCING STEEL SCHEDULE III SHEET NUMBER: S-54	-	<u>B</u>		2 <u>E 100</u>		<u> </u>						C STANDARD HOOK
	MAIN	E		THE	GOL	D S		<u>TYF</u>	<u>e 127</u> COB	E BOSS	BRIDGE EECO	E REHABILITATION NTEE STREAM OVERPASS G STEEL SCHEDULE III
PROJECT MANAGER: Kristi Van Ooyen, P.E. [CONTRACT:2018.15 90 0	X	/										SHEET NUMBER: S-54







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eel Sc												<u>TYPE 126</u>	<u>TYPE 129</u>	2
00_Reinforcing St	Scale:	Revision	Ву	Date	Designed by	:	HN	ITE	3		HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092	MAINE TURNPIKE	THE GOLD S	
name: 09					CONSULTANT Designed	PROJEC By HJW	T MANAGER Date 07\18	Craig R.M	By	Date 07\18	TEL (207) 774-5155 FAX (207) 228-0909			
File					Drawn	ERB	07\18	In Charge		07\18		MTA PROJECT MAN	AGER: KristiVan Ooyen,P.E.	

)ate:7/27/2018

MARK	SIZE	NO.	LENGTH	TYPE	A	в	c	D	E	F	REMARKS
PIER 1											
P401E1	4	72	3' - 6"	126	0'-6"	2'-4"	0'-8"				Pier Stem Horizontal Ties
P501E1	5	6	14' - 11"	100	4'-8"	2'-4"	0'-5.5"				Pier Footing Horizontal Stirrups
P510E1	5	84	2' - 8.5"	118	1'-6"	1'-2.5"					Pedestal Longitudinal (D+G)
P511E1	5	48	3' - 10.5"	118	2'-8"	1'-2.5"					Pedestal Transverse (D+G)
P512E1	5	12	5' - 10"	129	1'-6"	2'-2"					Pedestal Stirrup
P601E1	6	20	11' - 0"	129	2'-4"	4'-4"					Pier Stem Horizontal End Stirrups
P602E1	6	6	5' - 9"	STR							Pier Cap Horizontal Dowel
P603E1	6	4	7' - 3"	STR							Pier Hammerhead Cap Horizontal
P604E1	6	2	10' - 8''	STR							Pier Hammerhead Cap Horizontal
P605E1	6	2	13' - 3"	STR							Pier Hammerhead Cap Horizontal
P606E1	6	2 30	13' - 6"	STR	01.01	41.41					Pier Hammerhead Cap Horizontal
P607E1 P608E1	6 6	30	10' - 10'' 10' - 7''	129 129	2'-8" 2'-8"	4'-1" 3'-11.5"					Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
P609E1	6	2	10' - 4"	129	2'-8"	3'-10"					Pier Hammerhead Vertical Stirrups
P610E1	6	2	10' - 0''	129	2'-8"	3'-8"					Pier Hammerhead Vertical Stirrups
P611E1	6	2	9' - 9''	129	2'-8"	3'-6.5"					Pier Hammerhead Vertical Stirrups
P612E1	6	2	9' - 6''	129	2'-8"	3'-5"					Pier Hammerhead Vertical Stirrups
P613E1	6	2	9' - 4''	129	2'-8"	3'-4"					Pier Hammerhead Vertical Stirrups
P614E1	6	2	9' - 0''	129	2'-8"	3'-2"					Pier Hammerhead Vertical Stirrups
P615E1	6	2	8' - 9"	129	2'-8"	3'-0.5"			L		Pier Hammerhead Vertical Stirrups
P616E1	6	2	8' - 7"	129	2'-8"	2'-11.5"			───┤		Pier Hammerhead Vertical Stirrups
P617E1 P618E1	6 6	2	8' - 4'' 8' - 0''	129 129	2'-8" 2'-8"	2'-10" 2'-8"			+		Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
P618E1	6	2	7' - 8"	129	2-8	2-8			+ +		Pier Hammerhead Vertical Stirrups
P620E1	6	4	10' - 9''	129	1'-0"	6'-3"	2'-6"	1'-6"	0'-7.5"		Pier Hammerhead End Bars
P621E1	6	28	3' - 8"	119	1'-10"	1'-3"	1'-2.5"	-			Pier Hammerhead Vertical
P622E1	6	14	6' - 4''	129	2'-8"	1'-10"					Pier Hammerhead Vertical Stirrups
P623E1	6	8	7' - 8''	119	6'-5"	1'-3"	1'-2.5"				Pier Hammerhead Horizontal
P701E1	7	18	6' - 11"	STR							Pier Footing Transverse Dowel
P701E1	7	18	6' - 5''	STR							Pier Footing Transverse
P703E1	7	20	7' - 8"	STR							Pier Footing Longitudinal
P704E1	7	6	12' - 4"	118	11'-2"	1'-2"					Pier Footing Vertical Hook
P705E1	7	6	13' - 4"	118	12'-2"	1'-2"					Pier Footing Vertical Hook
P706E1	7	40	6' - 11"	STR							Pier Stem Horizontal Dowel
P707E1	7	40	7' - 7"	STR							Pier Stem Horizontal
P708E1 P709E1	7	6 6	14' - 3" 13' - 3"	STR STR							Pier Stem Vertical Pier Stem Vertical
P7109E1	7	4	17' - 0"	STR							Pier Stem Vertical
P711E1	7	2	16' - 10''	STR							Pier Stem Vertical
P1001E1	10	4	14' - 10"	111	13'-0"	1'-10"	5.5"				Pier Hammerhead Horizontal
P1002E1	10	4	13' - 8.5"	111	11'-10.5"	1'-10"	5.5"				Pier Hammerhead Horizontal
PIER 2											
P401E2	4	92	3' - 6"	126	0'-6"	2'-4"	0'-8"				Pier Stem Horizontal Ties
D501E2	5	6	14' - 11"	100	4'-8"	2' 4"	0' 5 5"				Diar Facting Harizantal Stimung
P501E2 P510E2	5	84	2' - 8.5"	100	4'-8"	2'-4" 1'-2.5"	0'-5.5"		+		Pier Footing Horizontal Stirrups Pedestal Longitudinal (D+G)
P511E2	5	48	3' - 10.5"	118	2'-8"	1'-2.5"			+ +		Pedestal Transverse (D+G)
P512E2	5	12	5' - 10''	129	1'-6"	2'-2"					Pedestal Stirrup
											•
P601E2	6	25	11' - 0''	129	2'-4"	4'-4"					Pier Stem Horizontal End Stimups
P602E2	6	6	5' - 9"	STR							Pier Cap Horizontal Dowel
P603E2	6	4	7' - 3"	STR							Pier Hammerhead Cap Horizontal
P604E2 P605E2	6 6	2	10' - 8" 13' - 3"	STR					+		Pier Hammerhead Cap Horizontal Pier Hammerhead Cap Horizontal
P605E2 P606E2	6	2	13' - 3" 13' - 6"	STR STR					+		Pier Hammerhead Cap Horizontal Pier Hammerhead Cap Horizontal
P607E2	6	30	10' - 10"	129	2'-8"	4'-1"					Pier Hammerhead Vertical Stirrups
P608E2	6	2	10' - 7"	129	2'-8"	3'-11.5"					Pier Hammerhead Vertical Stirrups
P609E2	6	2	10' - 4''	129	2'-8"	3'-10"					Pier Hammerhead Vertical Stirrups
P610E2	6	2	10' - 0''	129	2'-8"	3'-8"					Pier Hammerhead Vertical Stirrups
P611E2	6	2	9' - 9''	129	2'-8"	3'-6.5"					Pier Hammerhead Vertical Stirrups
P612E2	6	2	9' - 6"	129	2'-8"	3'-5"			↓ Ţ		Pier Hammerhead Vertical Stirrups
P613E2	6	2	9' - 4"	129	2'-8"	3'-4"					Pier Hammerhead Vertical Stirrups
P614E2 P615E2	6 6	2	9' - 0'' 8' - 9''	129 129	2'-8" 2'-8"	3'-2" 3'-0.5"			+		Pier Hammerhead Vertical Stirrups Pier Hammerhead Vertical Stirrups
P615E2	6	2	8 - 9	129	2'-8"	2'-11.5"			+ +		Pier Hammerhead Vertical Stirrups
P617E2	6	2	8' - 4"	129	2-8	2'-11.5					Pier Hammerhead Vertical Stirrups
P618E2	6	2	8' - 0''	129	2'-8"	2'-8"					Pier Hammerhead Vertical Stirrups
P619E2	6	2	7' - 8"	129	2'-8"	2'-6"					Pier Hammerhead Vertical Stirrups
P620E2	6	4	10' - 9"	143	1'-0"	6'-3"	2'-6"	1'-6"	0'-7.5"		Pier Hammerhead End Bars

:e:7/27,

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St	Scal	e:			Designed by:						
Reinforcing.							HN	TB			74
Re	No.	Revision	Bу	Date							34
091_											
					CONSULTANT F	ROJE	CT MANAGER:	Craig R. Morin,	P.E.		
Filename:						By	Date		By	Date	
Bug					Designed	HJW	07\18	Checked	KEB	07\18	
Ē					Drawn	ERB	07\18	In Charge of	RAL	07\18	

HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155 FAX (207) 228-0909



## THE GOLD STAR **MEMORIAL HIGHWAY**

MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

COBBOSSEECONTEE NB BRIDGE WIN# 2018.15

TYPE

STR

STR

STR

118

STR

STR

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STR

STR STR

111

126

100 118

118

129

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129

129

129

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129

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143

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STR STR STR STR

118

118

11'-2"

12'-2"

13'-0"

0'-6"

4'-8"

1'-6"

2'-8"

1'-6"

2'-4"

2'-8"

2'-8"

2'-8"

2'-8"

2'-8"

2'-8"

2'-8"

2'-8"

2'-8"

2'-8"

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2'-8"

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1'-0"

11'-2" 1'-2"

12'-2" 1'-2"

111 13'-0" 1'-10" 5.5" 111 11'-10.5" 1'-10" 5.5"

111 11'-10.5" 1'-10" 5.5"

1'-2"

1'-2"

1'-10" 5.5"

2'-4" 0'-8"

0'-5.5"

2'-4"

1'-2.5"

1'-2.5"

2'-2"

4'-4"

4'-1"

3'-11.5"

3'-10"

3'-8" 3'-6.5"

3'-5"

3'-4"

3'-2"

3'-0.5"

2'-11.5"

2'-10"

2'-8"

2'-6"

6'-3"

2'-6"

LENGTH

MARK

PIER 2 P701E2

P702E2

P704E2

P705E2

P706E2

P708E2

P709E2

P710E2

P711E2

PIER 3 P401E3

P501E3

P510E3

P511E3

P512E3

P601E3

P603E3

P605E3

P606E3

P607E3

P608E3

P609E3

P610E3

P611E3

P612E3

P613E3

P614E3

P618E3

P619E3

P620E3

P703E3

P704E3

P706E3

P707E3

SIZE NO.

P703E2 7 38 8' - 8"

P707E2 7 50 7' - 7"

P1001E2 10 4 14' - 10"

P1002E2 10 4 13' - 8.5"

5 6 5 84

5 12

5

P602E3 6 6 5' - 9"

P604E3 6 2 10' - 8"

6 4

6 30

6 2

6 2 6 2

P615E3 6 2 8' - 9"

 P616E3
 6
 2
 8'
 7''

 P617E3
 6
 2
 8'
 4''

6 4

 P701E3
 7
 18
 6' 11"

 P702E3
 7
 18
 6' 5"

P705E3 7 6 13' - 4"

 PT08E3
 7
 6
 24'
 5"

 P709E3
 7
 6
 24'
 5"

 P709E3
 7
 6
 23'
 5"

 P710E3
 7
 4
 27'
 3"

 P711E3
 7
 2
 27'
 1"

 P1001E3
 10
 4
 14'
 10''

 P1002E3
 10
 4
 13'
 8.5''

6 2

6 2 9'-6" 
 6
 2
 9'
 4''

 6
 2
 9'
 0''

6 2 8' - 0"

7 20 7' - 8"

7 62 6' - 11"

7 6 12' - 4"

7 62 7' - 7"

6 2 7' - 8"

48

6 31 11' - 0"

 6
 2
 13' - 3"

 6
 2
 13' - 6"

7 6

7

7 20 6' - 11"

50 6' - 11'

18' - 5"

14' - 11'

7' - 3'

10' - 10'

10' - 7"

10' - 4"

10' - 0"

9' - 9"

10' - 9"

2' - 8.5"

3' - 10.5" 5' - 10"

7 20 6' - 5"

 7
 6
 12' - 4''

 7
 6
 13' - 4''

7 6 17' - 5"

 7
 4
 21' - 3"

 7
 2
 21' - 1"

4 116 3' - 6"

D         F         REMARKS           Pier Footing Transverse Dowel         Pier Footing Transverse Dowel           Pier Footing Vertical Hook         Pier Footing Vertical Hook           Pier Footing Vertical Hook         Pier Footing Vertical Hook           Pier Footing Vertical Hook         Pier Stem Vertical           Pier Stem Vertical         Pier Stem Vertical           Pier Stem Nertical         Pier Stem Vertical           Pier Stem Vertical         Pier Stem Vertical				
Pier Footing Transverse         Pier Footing Vertical Hook         Pier Stem Vertical Hook         Pier Stem Horizontal Dowel         Pier Stem Horizontal         Pier Stem Vertical         Pier Stem Horizontal         Pier Stem Horizontal Ties         Pier Footing Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Longitudinal (D+G)         Pedestal Transverse (D+G)         Pedestal Stirrups         Pier Tammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stirrups	D	E	F	REMARKS
Pier Footing Transverse         Pier Footing Vertical Hook         Pier Stem Vertical Hook         Pier Stem Horizontal Dowel         Pier Stem Horizontal         Pier Stem Vertical         Pier Stem Horizontal         Pier Stem Horizontal Ties         Pier Footing Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Longitudinal (D+G)         Pedestal Transverse (D+G)         Pedestal Stirrups         Pier Tammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stirrups				Pier Footing Transverse Dowel
Pier Footing Vertical Hook       Pier Footing Vertical Hook       Pier Stem Horizontal       Pier Stem Horizontal       Pier Stem Vertical       Pier Fortig Horizontal       Pier Stem Horizontal       Pier Stem Horizontal       Pier Stem Horizontal Stirups       Pedestal Longitudinal (D+G)       Pedestal Stirups       Pier Cap Horizontal Ed Stirups       Pier Cap Horizontal Dowel       Pier Cap Horizontal Dowel       Pier Hammerhead Cap Horizontal       Pier Hammerhead Cap Horizontal       Pier Hammerhead Cap Horizontal       Pier Hammerhead Vertical Stirups				
Pier Footing Vertical Hook         Pier Stem Horizontal         Pier Stem Horizontal         Pier Stem Vertical         Pier Hammerhead Horizontal         Pier Hammerhead Horizontal         Pier Footing Horizontal Ties         Pier Footing Horizontal Stimups         Pedestal Longitudina (Dr-G)         Pedestal Transverse (Dr-G)         Per Stem Horizontal End Stimups         Pier Cap Horizontal Dowel         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stimups         Pier Hammerhead Vertical Stimups <tr< td=""><td></td><td></td><td></td><td></td></tr<>				
Pier Footing Vertical Hook         Pier Stem Horizontal Dowel         Pier Stem Vertical         Pier Footing Horizontal         Pier Footing Horizontal Stirups         Pedestal Longitudinal (D+G)         Pedestal Stirup         Pedestal Stirups         Pedestal Stirups         Pedestal Stirups         Pedestal Stirups         Per Stem Horizontal Stirups         Pedestal Cap Horizontal         Pier Stem Horizontal End Stirups         Pier Stem Horizontal Dovel         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stirups				
Pier Stem Horizontal         Pier Stem Vertical         Pier Hammerhead Horizontal         Pier Footing Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Stirrups         Pedestal Stirrups         Pier Cop Horizontal End Stirrups         Pier Cap Horizontal End Stirrups         Pier Cap Horizontal Dowel         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stirrups         Pier Hammerhead Vertical Stirups				
Per Stem Vertical         Per Hammerhead Horizontal         Per Stem Vertical         Per Hammerhead Cap Horizontal         Per Hammerhead Cap Horizontal         Per Hammerhead Cap Horizontal         Per Hammerhead Vertical Stimps         Per Hamm				Pier Stem Horizontal Dowel
Pier Stem Vertical         Pier Stem Vertical         Pier Stem Vertical         Pier Hammerhead Horizontal         Pier Hammerhead Horizontal         Pier Stem Horizontal Ties         Pier Stem Horizontal Ties         Pier Stem Horizontal Stimps         Pedestal Congitudinal (D+G)         Pedestal Transverse (D+G)         Pedestal Transverse (D+G)         Pier Stem Horizontal End Stimps         Pier Cap Horizontal End Stimps         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stimps				Pier Stem Horizontal
Pier Stem Vertical         Pier Stem Vertical         Pier Hammerhead Horizontal         Pier Hammerhead Horizontal         Pier Stem Horizontal Ties         Pier Stem Horizontal Stimps         Peter Stem Horizontal Dowel         Pier Stem Horizontal End Stimps         Pier Gap Horizontal Dowel         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stimps				Pier Stem Vertical
Pier Stem Vertical         Pier Hammerhead Horizontal         Pier Hammerhead Horizontal         Pier Stem Horizontal Ties         Pier Stem Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Stirrup         Pedestal Stirrup         Pier Cap Horizontal End Stirrups         Pier Cap Horizontal Dowel         Pier Cap Horizontal Dowel         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stirrups         Pier Footing Tran				Pier Stem Vertical
Pier Hammerhead Horizontal         Pier Hammerhead Horizontal         Pier Stem Horizontal Ties         Pier Footing Horizontal Ties         Pier Stem Horizontal Ties         Pier Stem Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Stirrups         Pedestal Stirrups         Pier Cap Horizontal End Stirrups         Pier Cap Horizontal Dowel         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Cap Horizontal         Pier Hammerhead Vertical Stirrups         Pier				Pier Stem Vertical
Pier Hammethead Horizontal         Pier Stem Horizontal Ties         Pier Stem Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Stirrup         Pedestal Stirrup         Pier Stem Horizontal End Stirrups         Pier Cap Horizontal Dowel         Pier Cap Horizontal Dowel         Pier Hammethead Cap Horizontal         Pier Hammethead Vertical Stirrups         Pier Footing Transverse         Pier Footi				Pier Stem Vertical
Pier Hammethead Horizontal         Pier Stem Horizontal Ties         Pier Stem Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Stirrup         Pedestal Stirrup         Pier Stem Horizontal End Stirrups         Pier Cap Horizontal Dowel         Pier Cap Horizontal Dowel         Pier Hammethead Cap Horizontal         Pier Hammethead Vertical Stirrups         Pier Footing Transverse         Pier Footi				
Pier Stem Horizontal Ties         Pier Footing Horizontal Stimups         Pedestal Longitudinal (D+G)         Pedestal Transverse (D+G)         Pedestal Stimup         Pier Cap Horizontal Stimups         Pier Cap Horizontal Dowel         Pier Tammerhead Cap Horizontal         Pier Hammerhead Vertical Stimps         Pier Footing Longitudinal         Pier Footing Longitudinal         Pier Footing Congitudinal				
Pier Footing Horizontal Stirrups         Pedestal Longitudinal (D+G)         Pedestal Transverse (D+G)         Pedestal Stirrup         Pier Stem Horizontal End Stirrups         Pier Cap Horizontal Dowel         Pier Tammerhead Cap Horizontal         Pier Hammerhead Vertical Stirrups         Pier Footing Transverse         Pier Footing Transverse Dowel         Pier Footing Transverse Dowel				Pier Hammerhead Horizontal
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				Pier Hammerhead Horizontal

BRIDGE REHABILITATION COBBOSSEECONTEE STREAM OVERPASS

REINFORCING STEEL SCHEDULE IV

CONTRACT:2018.15

SHEET NUMBER: S-55

#### GENERAL NOTES

I. ALL DETAILS SHALL BE IN CONFORMANCE WITH MAINE DEPARTMENT OF TRANSPORTATION (MAINEDOT) STANDARD DETAILS HIGHWAY AND BRIDGES 2014 LATEST REVISIONS AND MAINEDOT BEST MANAGEMENT PRACTICES FOR EROSION AND SEDIMENT CONTROL LATEST REVISION UNLESS OTHERWISE INCLUDED IN THESE PLANS.

2. ALL TEMPORARY AND PERMANENT EROSION CONTROL DEVICES SHALL BE INSTALLED IN ACCORDANCE WITH THE MAINE DEPARTMENT OF TRANSPORTATION BEST MANAGEMENT PRACTICES.

3. THERE ARE NO PERMANENT OR TEMPORARY EASEMENTS ASSOCIATED WITH THIS PROJECT. ALL WORK SHALL BE COMPLETED WITHIN THE EXISTING RIGHT OF WAY.

4. THE CONTRACTOR SHALL SUBMIT THE PROPOSED STAGING AREA(S) TO THE RESIDENT PRIOR TO STARTING WORK.

5. COPIES OF AS-BUILT PLANS ARE POSTED ON THE MAINE TURNPIKE AUTHORITY WEBSITE AT WWW.MAINETURNPIKE.COM/PROJECT-AND-PLANNING/CONSTRUCTION-CONTRACTS.ASPX.THE COMPLETENESS AND ACCURACY OF THESE PLANS IS NOT GUARANTEED.

6. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT TOOLS, VEHICLES, EQUIPMENT, MATERIALS AND PERSONNEL FROM MARRING THE PAVEMENT SURFACE ON THE BRIDGE DECK AND APPROACHES WHILE THE WORK IS BEING COMPLETED.

7. THE CONTRACTOR SHALL SET, MAINTAIN AND REMOVE NAVIGATIONAL AIDS IN THE RIVER. BOAT TRAFFIC SHALL BE MAINTAINED DURING CONSTRUCTION. NO LONG TERM STOPPAGES ARE PERMITTED. THE NAVIGATIONAL AIDS, FURNISHED BY THE STATE OF MAINE TO THE AUTHORITY, SHALL BE POSITIONED TO ADVISE BOATERS OF THE CONSTRUCTION HAZARDS AHEAD. ADDITIONAL NAVIGATIONAL AIDS MAY BE NEEDED DEPENDING ON CONTRACTOR ACTIVITIES. PAYMENT FOR OBTAINING, SETTING, MAINTAINING AND REMOVING THE NAVIGATIONAL AIDS SHALL BE INCIDENTAL TO THE CONTRACT.

8. BOOM SUPPORTED FLOATING SILT FENCE SHALL ONLY BE USED BETWEEN NO MORE THAN TWO PIERS AT ONE TIME ON THE ANDROSCOGGIN RIVER.

9. CONTRACTOR SHALL PROVIDE RESIDENT ACCESS TO ALL AREAS UNDER CONSTRUCTION FOR INSPECTION PURPOSES. THIS SHALL BE INCIDENTAL TO PAY ITEM 524, TEMPORARY ACCESS PLATFORMS FOR PIER INSPECTION AND REPAIR.

#### <u>MATERIALS</u>

#### CONCRETE

SUBSTRUCTURE CONCRETE REPAIRS SHALL BE PER SPECIAL PROVISION 5/8.

REINFORCING STEEL

AASHTO M3I, GRADE 60. ALL REINFORCING SHALL BE EPOXY-COATED.

STRUCTURAL STEEL PLATES AND BARS SHALL CONFORM TO ASTM A36

HIGH STRENGTH BOLTS SHALL BE AASHTO MI64 (ASTM A325) HOT DIP GALVANIZED IN STANDARD HOLES.

ABUT. - ABUTMENT ADDL. - ADDITIONAL ALT. - ALTERNATE APPROX. - APPROXIMATELY BOT. - BOTTOM BRG. - BEARING CL. - CLEAR CONC. - CONCRETE CONSTR. - CONSTRUCTION DEMO, - DEMOLITION DIA. - DIAMETER EA. - EACH EB - EASTBOUND E.F. - EACH FACE EL. - ELEVATION EQ. - EQUAL EXIST. - EXISTING EXP. - EXPANSION F.F. - FAR FACE JT. - JOINT MAX. - MAXIMUM MAINEDOT - MAINE DEPARTMENT OF TRANSPORTATION MIN. - MINIMUM MTA - MAINE TURNPIKE AUTHORITY NB - NORTHBOUND N.F. - NEAR FACE N.T.S. - NOT TO SCALE PED. - PEDESTAL PGL - PROFILE GRADE LINE R - PLATE PROP. - PROPOSED P.S.I. - POUNDS per SQUARE INCH RDWY. - ROADWAY SHLDR. - SHOULDER SB - SOUTHBOUND SP. - SPACES STA. - STATION T.&B. - TOP & BOTTOM TPKE. - TURNPIKE TYP. - TYPICAL U.O.N. - UNLESS OTHERWISE NOTED VERT. - VERTICAL

WB - WESTBOUND W.P. - WORKING POINT

LIST OF ABBREVIATIONS

ITEM NO.	ITEM DESCRIPTION	REFERENCE QUANTITY	UNIT	ANDRO. QUANTITY
201.31	Removal of Debris		LS	1
504.885	Post-Tensioning Sleeve Repair		EA	13
514.06	Curing Box for Concrete Cylinders		EA	1
515.202	Clear Protective Coating for Concrete Surfaces		SY	3,250
515.23	Anti-Graffiti Coating		SY	900
518.40	Epoxy Injection Crack Repair		LF	310
518.401	Epoxy Injection Crack Repair - Below Waterline		LF	470
518.51	Repair of Upward Facing Surfaces - Below Reinforcing Steel < 8 inches		SF	43
518.60	Repair of Vertical Surfaces < 8 inches		SF	2,950
518.601	Repair of Vertical Surfaces < 8 inches - Below Waterline		SF	18
518.70	Repair of Overhead Surfaces < 8 inches		SF	91
523.56	Cleaning and Painting Bearing		EA	42
523.561	Repair Bearing		EA	1
523.562	Repair Bearing Keeper Strap		EA	10
524.301	Temporary Structural Support - Androscoggin Jacking		LS	1
524.60	Temporary Access Platforms for Pier Inspection and Repair		LS	1
526.306	Temporary Concrete Barrier, Type I - Supplied by Authority	5210 LF	LS	0.5
527.341	Work Zone Crash Cushions - TL-3		Unit	2
627.73	Temporary 6 Inch Pavement Marking Tape		LF	9,700
627.731	Temporary 6 Inch Black Pavement Marking Tape		LF	2,800
629.05	Hand Labor, Straight Time		HR	20
631.10	Air Compressor (including operator)		HR	10
631.11	Air Tool (including operator)		HR	10
631.172	Truck - Large (including operator)		HR	20
631.36	Foreman		HR	10
652.30	Flashing Arrow		EA	2
652.33	Drum		EA	86
652.34	Cone		EA	50
652.35	Construction Signs		SF	830
652.361	Maintenance of Traffic Control Devices		LS	0.1
652.38	Flaggers		HR	10
652.41	Portable-Changeable Message Sign		EA	2
652.45	Truck Mounted Attenuator		CD	20
652.451	Automated Trailer Mounted Speed Limit Sign		CD	20
652.46	Temporary Portable Rumble Strip		Unit	20
656.50	Baled Hay, in place		EA	
656.632	30 inch Temporary Silt Fence		LF	
656.64	Boom Supported Floating Silt Fence		LF	302

#### SCOPE OF WORK

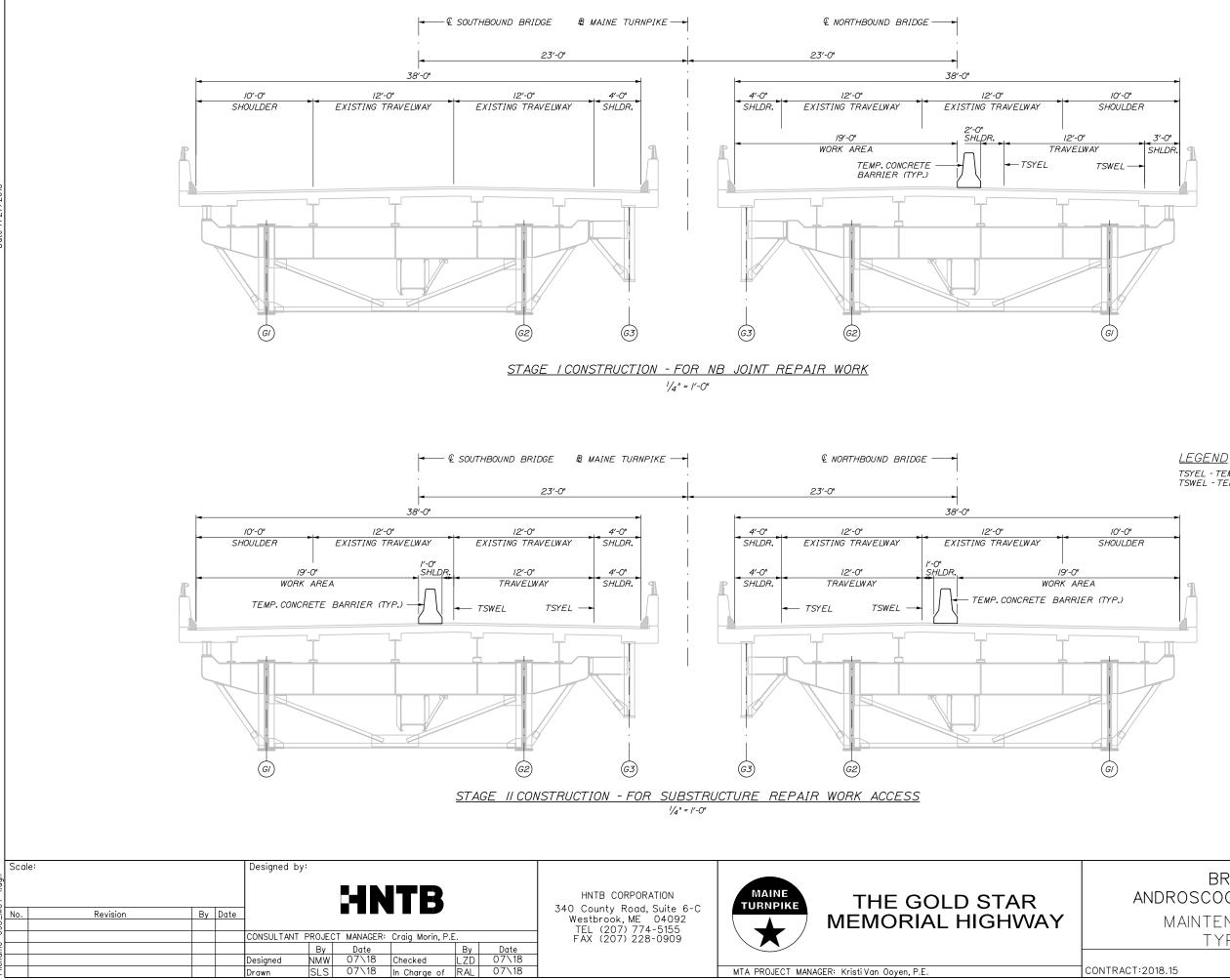
THE PROPOSED REPAIR PROJECT INCLUDES BUT IS NOT LIMITED TO THE FOLLOWING WORK:

- I. SUBSTRUCTURE REPAIRS
- 2. BEARING REPAIRS
- 3. POST-TENSIONING SLEEVE REPAIRS
- 4. ASSOCIATED MOT ACTIVITIES TO COMPLETE WORK

lNotes a	Scale:			Designed by	,							
092_Genera	No.	Revision	By Date				ITB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155		THE GOLD STAR MEMORIAL HIGHWAY
0:0				CONSULTANT	PROJEC	T MANAGER:	Craig Morin, P	.E.		FAX (207) 228-0909		
me					By	Date		By	Date			
enc				Designed	NMW	07\18	Checked	BRG	07\18			
File				Drawn	PEB	07\18	In Charge of	RAL	07\18	]	MTA PROJECT MAN	AGER: Kristi Van Ooyen, P.E.

BRIDGE REPAIRS ANDROSCOGGIN RIVER OVERPASS GENERAL NOTES AND ESTIMATED QUANTITIES SHEET NUMBER: AR-01

CONTRACT:2018.15



Drawn

MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

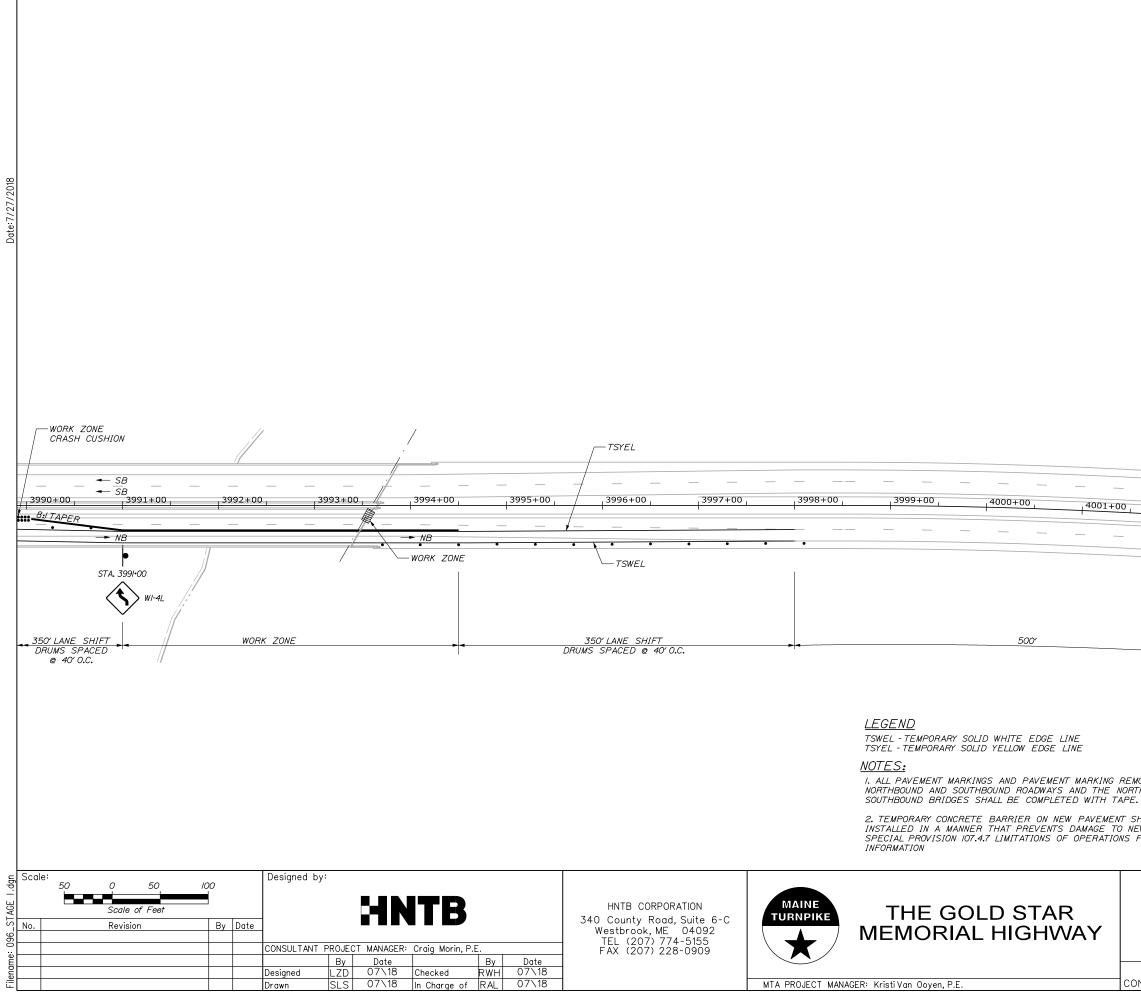
## MAINTENANCE OF TRAFFIC TYPICAL SECTION



TSYEL - TEMPORARY SOLID YELLOW EDGE LINE TSWEL - TEMPORARY SOLID WHITE EDGE LINE

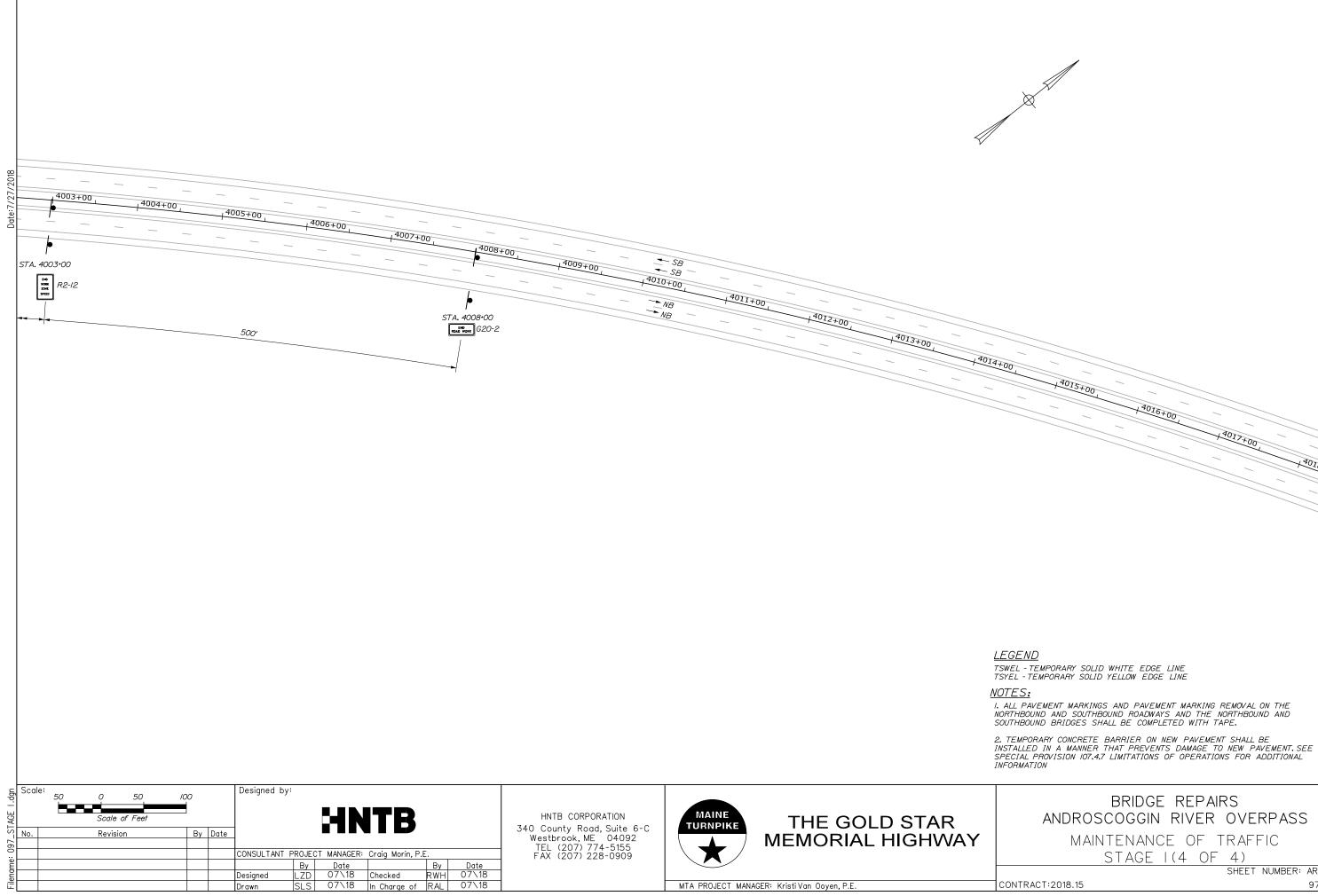
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Sedie: 50 0 50 00       Designed by:         Sedie: 50 0 50 00       50 0 50 00       Designed by:         Marker Mark Bould Designed by:       HNIB CORPORATION       Suite 5-0         Marker Mark Bould Designed by:       HNIB CORPORATION       Suite 5-0         Scole: 6 free:       Designed by:       HNIB CORPORATION       Suite 5-0         Marker		
Sedie: 50 0 50 00       Designed by:         Sedie: 50 0 50 00       50 0 50 00       Designed by:         Marker Mark Bould Designed by:       HNIB CORPORATION       Suite 5-0         Marker Mark Bould Designed by:       HNIB CORPORATION       Suite 5-0         Scole: 6 free:       Designed by:       HNIB CORPORATION       Suite 5-0         Marker		NORTHBOUND 840' LANE DROP
Scale of Feet       HNTB CORPORATION         Scale of Feet       HNTB CORPORATION         No.       Revision       By Date         Maine       THE GOLD STAR         MAINE       MAINE         Loc       By Date       By Date         By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date       By Date	TSWEL - TEMPORARY SO TSYEL - TEMPORARY SO <u>NOTES:</u> I. ALL PAVEMENT MARK. NORTHBOUND AND SOUT SOUTHBOUND BRIDGES 2. TEMPORARY CONCRET INSTALLED IN A MANNE	INGS AND PAVEMENT MARKING REMOVAL ON THE THBOUND ROADWAYS AND THE NORTHBOUND AND SHALL BE COMPLETED WITH TAPE. TE BARRIER ON NEW PAVEMENT SHALL BE ER THAT PREVENTS DAMAGE TO NEW PAVEMENT.SEE
Drawn         SLS         07\18         In Charge of RAL         07\18         MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.         CONTRACT:2018.15         94 OF 115		

Date:7/27/2018				
		0 3982+00 3983+00 3984+00 • • • • • • • • • • • • • • • • • • •	B B B B B B B B B B B B B B	$\begin{array}{c} & & & & & \\ & & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & &$
GE I.dgn	Scale: 50 0 50 100 Scale of Feet No Revision By Date	HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155 FAX (207) 228-0909	NORTHBOUND AND SOUTHBOU SOUTHBOUND BRIDGES SHAL 2. TEMPORARY CONCRETE B INSTALLED IN A MANNER TI	WHITE EDGE LINE AND PAVEMENT MARKING REMOVAL ON THE IND ROADWAYS AND THE NORTHBOUND AND IL BE COMPLETED WITH TAPE. WARRIER ON NEW PAVEMENT SHALL BE HAT PREVENTS DAMAGE TO NEW PAVEMENT. SEE LIMITATIONS OF OPERATIONS FOR ADDITIONAL BRIDGE REPAIRS ANDROSCOGGIN RIVER OVERPASS



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0, 4002+00, 4003+00, 4004+00,
STA. 4003+00
500 ⁻
REMOVAL ON THE IORTHBOUND AND APE. T SHALL BE D NEW PAVEMENT. SEE NS FOR ADDITIONAL
BRIDGE REPAIRS ANDROSCOGGIN RIVER OVERPASS MAINTENANCE OF TRAFFIC STAGE I (3 OF 4) SHEET NUMBER: AR-05
CONTRACT:2018 15 96 OF 115



MTA	PROJECT	MANAGER:	Kristi Van	Ooven	ΡF

CONTRACT:2018.15

4015+00

4016+00

97 OF 115

STAGE I(4 OF 4)

BRIDGE REPAIRS

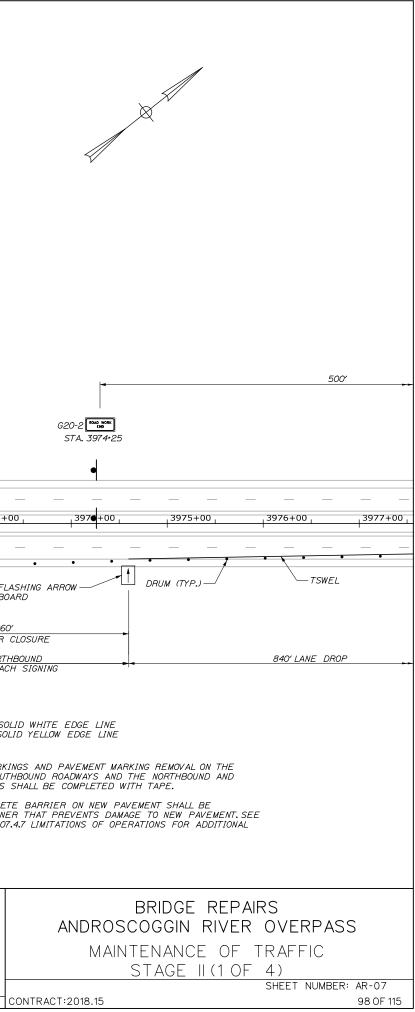
ANDROSCOGGIN RIVER OVERPASS

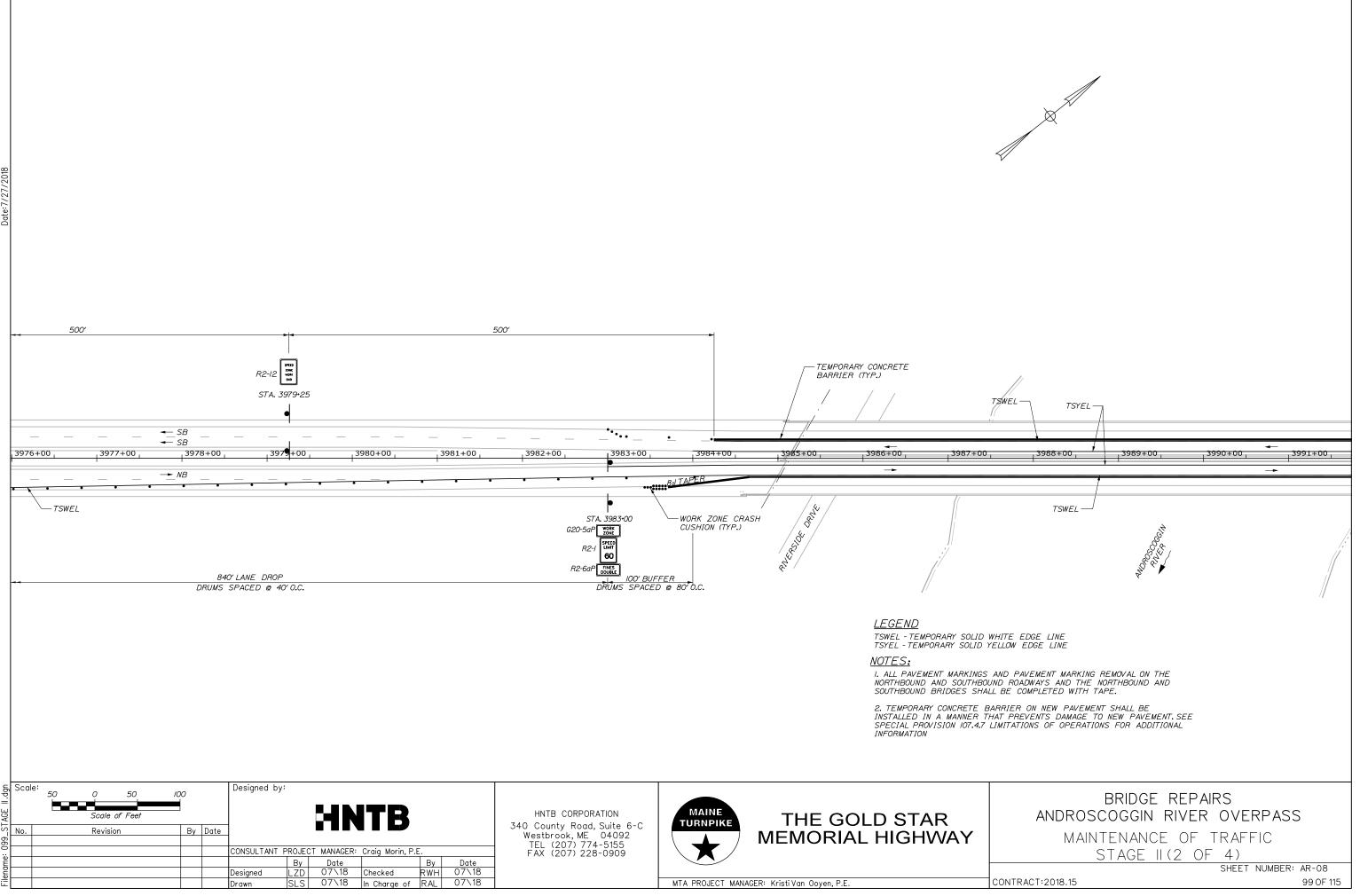
MAINTENANCE OF TRAFFIC

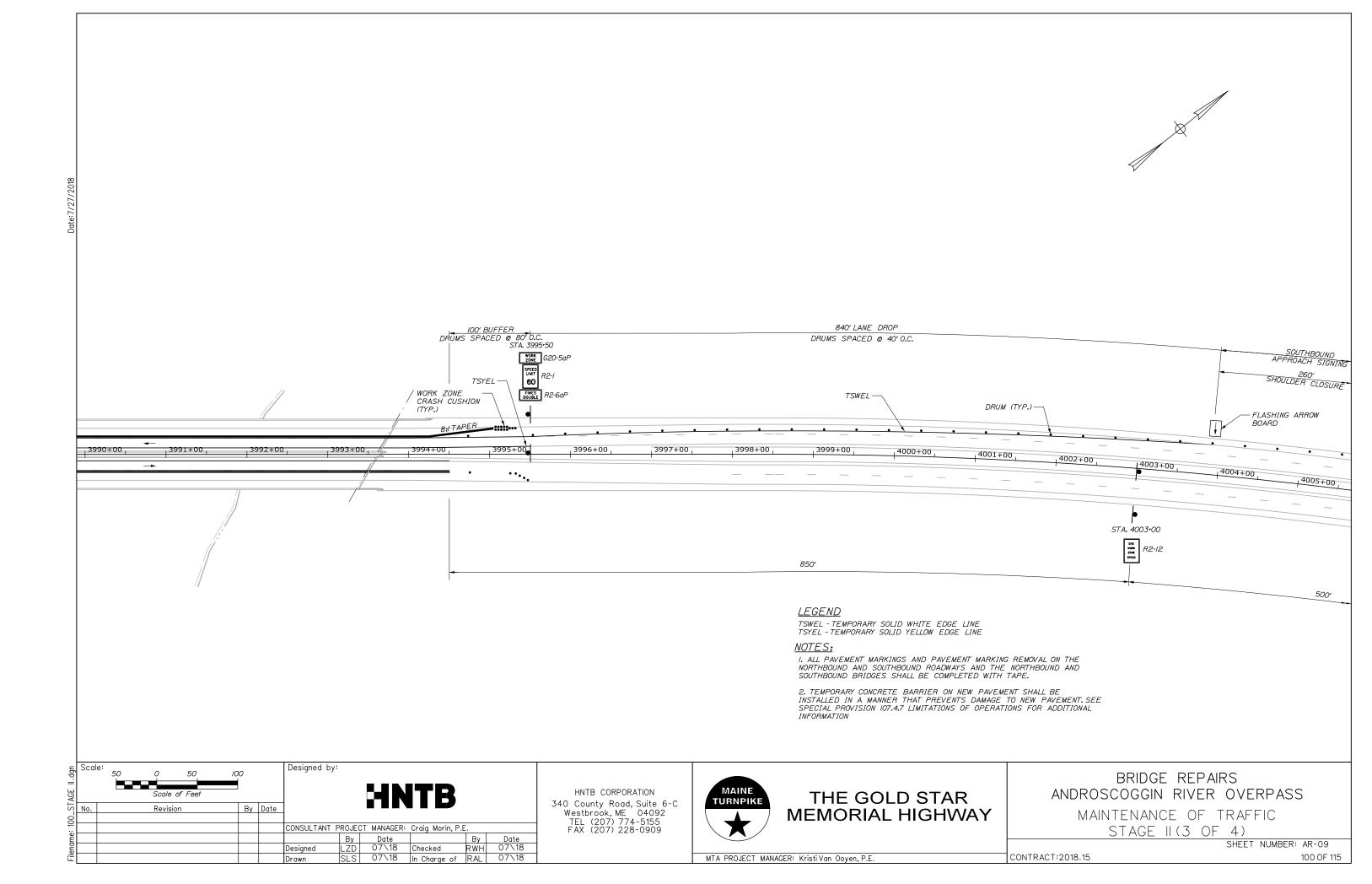
SHEET NUMBER: AR-06

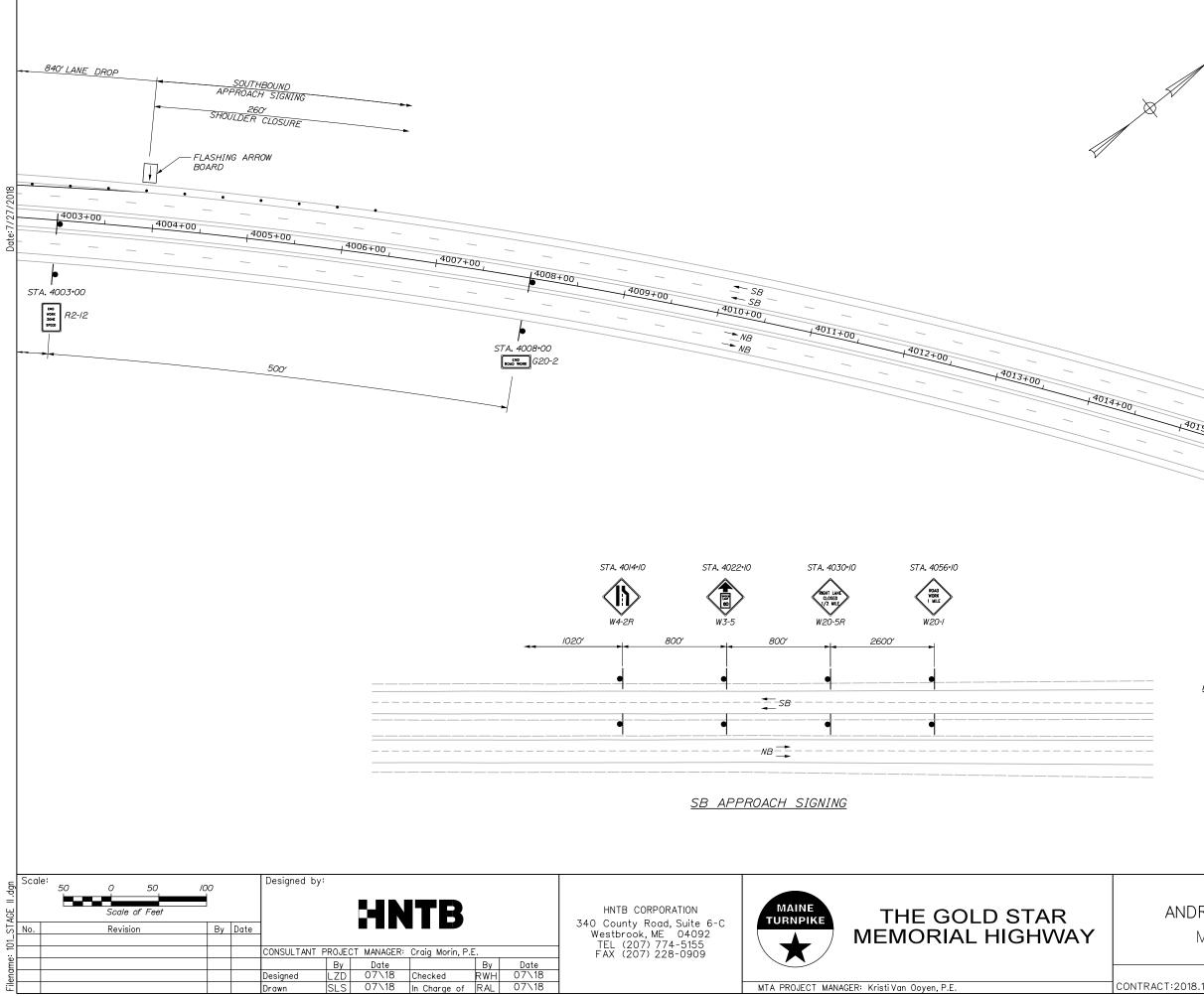
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Date:7/27/2018				STA. 392	-I W20-5R		  /020′		
	3962+00 ,; 3963+0 NB №	0,3964	+00 1 3965	5+00 , 3966-	+003967+0	$ \underbrace{ SB}_{\bullet} SB \\ \underbrace{ SB}_{\bullet} SB \\ \underbrace{ SB}_{\bullet} \\ \underbrace{ 3968+00}_{\bullet} \\ \underbrace{ 3969+0}_{\bullet} \\  3969+$			 3972+00 , 3973+  FI BU FI BU FI BU SHOULDER NORT APPROAU
Filename: 098_STAGE II.dgn	e: 50 0 50 Scale of Feet Revision	/00 By Date			orin, P.E.	HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155 FAX (207) 228-0909		THE GOLI MEMORIAL I	LEGEND TSWEL - TEMPORARY S TSYEL - TEMPORARY S NOTES: I. ALL PAVEMENT MARI NORTHBOUND AND SOL SOUTHBOUND BRIDGES 2. TEMPORARY CONCRE INSTALLED IN A MANN SPECIAL PROVISION IC INFORMATION
Filenar			Designed LZI	) 07\18 Checked		-	MTA PROJECT MAN/	AGER: Kristi Van Ooyen, P.E.	









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<u>LEGEND</u>

TSWEL - TEMPORARY SOLID WHITE EDGE LINE TSYEL - TEMPORARY SOLID YELLOW EDGE LINE

4016+00

### NOTES:

4015+00

I. ALL PAVEMENT MARKINGS AND PAVEMENT MARKING REMOVAL ON THE NORTHBOUND AND SOUTHBOUND ROADWAYS AND THE NORTHBOUND AND SOUTHBOUND BRIDGES SHALL BE COMPLETED WITH TAPE.

4017+00

4018+00

2. TEMPORARY CONCRETE BARRIER ON NEW PAVEMENT SHALL BE INSTALLED IN A MANNER THAT PREVENTS DAMAGE TO NEW PAVEMENT. SEE SPECIAL PROVISION 107.4.7 LIMITATIONS OF OPERATIONS FOR ADDITIONAL INFORMATION

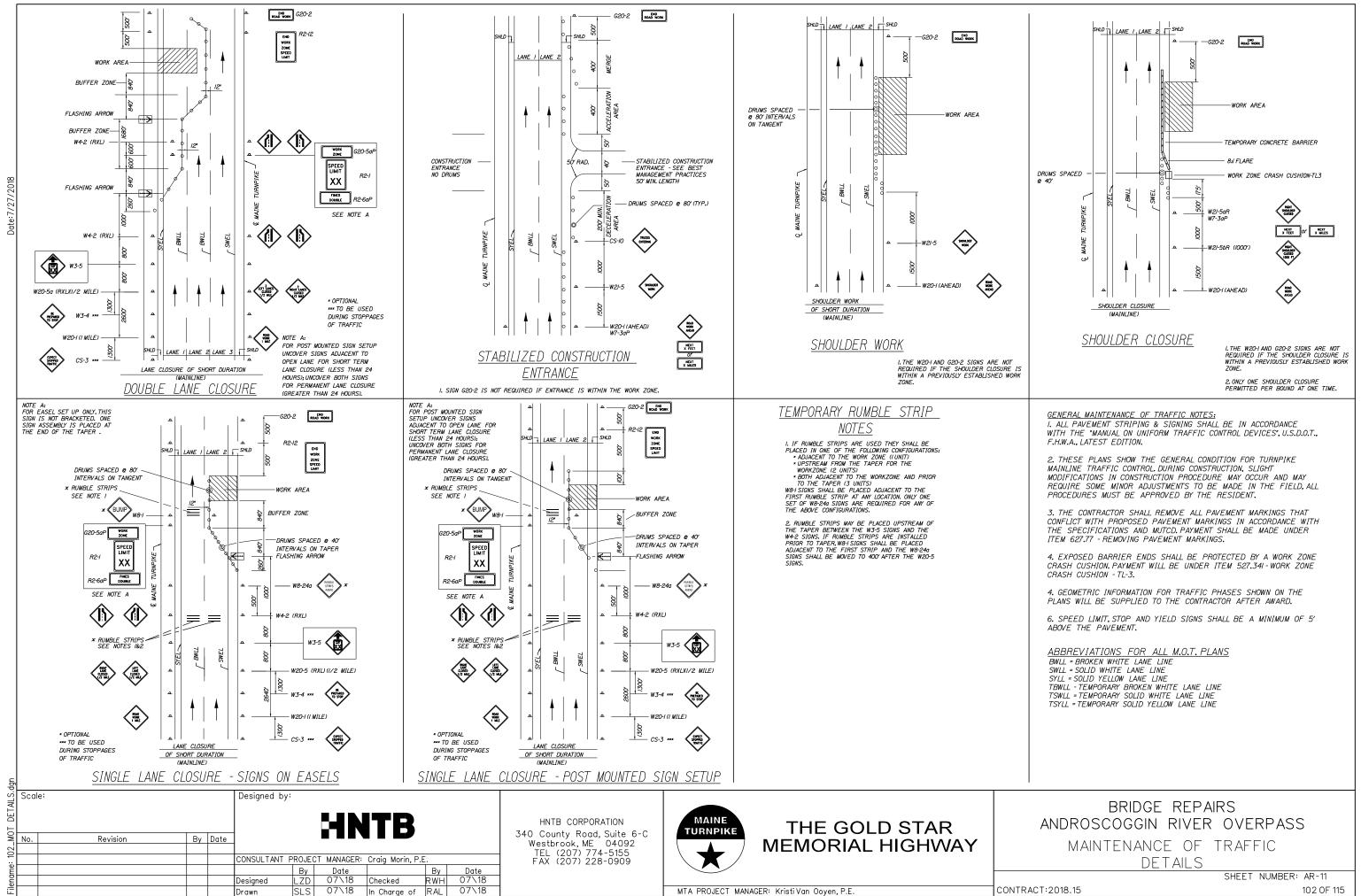
4. W2O-I "ROAD WORK I MILE" SIGN NEEDED ON THE EXIT 80 SOUTHBOUND ON RAMP.

## BRIDGE REPAIRS ANDROSCOGGIN RIVER OVERPASS MAINTENANCE OF TRAFFIC STAGE II (4 OF 4)

CONTRACT:2018.15

MTA PROJECT MANAGER: KristiVan Ooyen, P.E.

SHEET NUMBER: AR-10



CATION	IZE OF	- SIGN	TEXT			(INCHES)	NUMBER OF	COL		BORDER	AREA IN SQUARE	IDENTIFI- CATION	SIZE C	F SIGN	TEXT			ENSIONS		NUMBER OF	С
	VIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE.MKR.	SIGNS REQUIRED	BACK- GROUND	LEGEND BORDER	RADIUS	FEET	NUMBER	WIDTH	HEIGHT		LETT HEIG	ER V GHT S	'ERTICAL SPACING	ARROW RTE.MKR.	SIGNS REQUIRED	BACK- GROUNL
G20-2	<i>48</i> "	24"	END ROAD WORK	CONFORM STANDAR		EDITION - Y SIGNS -	4	ORANGE	BLACK		8.00 (32)	W8-/	<i>48</i> *	<i>48</i> "	BUMP	CONF	FORM T	MENSIONS 10 "2004 L HIGHWAY	EDITION -	4	ORANGE
G20-5aP	48"	24"	WORK ZONE				4				8.00 (32)							SUPPLEM			
R2-I (60)	<i>48</i> "	60"	Speed Limit XX				4	WHITE	BLACK		20.00 (80)	W8-24a	<i>48</i> "	<i>48</i> "	RUMBLE STRIPS AHEAD					4	
R2-6aP	48"	24"	FINES DOUBLED				4				8.00 (32)				<u> </u>						
R2-12	<i>48</i> "	60"	END WORK ZONE SPEED				4	<b>•</b>			20.00 (80)	W20-I (  MILE)	<i>48</i> *	<i>48</i> "	ROAD WORK XXX					5	
WI-4L	<i>48</i> *	48"					/	ORANGE	BLACK		16.00 (16)	W20-5L (1/2 MILE	) <i>48</i> *	48"	LEFT LANE CLOSED XXX					2	
WI-4R	48"	<i>48</i> "	$\langle \rangle$				I				16.00 (16)	W20-5R (1/2 MILE	) <i>48</i> "	48"	RIGHT LANE CLOSED					4	
W3-5 (60)	48"	<i>48</i> "	SPEED LIMIT XX				4				16.00 (64)					•	,	<b>.</b>	<b>  ▼</b>		<u> </u>
W4-2L	48*	48"					2				16.00 (32)										
W4-2R	<i>48</i> "	<i>48</i> "					4				16.00 (64)										

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COLOR			BORDER	AREA IN SQUARE
CK- DUND	LEGI BORI	END DER	RADIUS	FEET
NGE	BLA	ACK		16.00 (64)
				16.00 (64)
				16.00 (80)
				16.00 (32)
		ľ		16.00 (64)

BRIDGE REPAIRS ANDROSCOGGIN RIVER OVERPASS MAINTENANCE OF TRAFFIC SIGN SUMMARY

CONTRACT:2018.15

SHEET NUMBER: AR-12

#### ABUTMENT NOTES:

I. ABUTMENT REPAIR WORK SHALL INCLUDE: ABUTMENT SURFACE REPAIRS, EPOXY INJECTION CRACK REPAIR AND SEALING EXISTING CONSTRUCTION JOINTS. ALL ABUTMENT REPAIR AREAS ARE ABOVE THE ORDINARY WATER ELEVATION.

2. WHERE ABUTMENT SURFACE REPAIRS ARE SPECIFIED THE WORK SHALL INCLUDE REMOVAL OF UNSOUND CONCRETE AND PLACEMENT AND CURING OF REPAIR MATERIALS. REPAIRS SHALL BE MEASURED FOR PAYMENT UNDER PAY ITEM 5/8.60, "REPAIR OF VERTICAL SURFACES <8 INCHES" and 5/8.51 REPAIR OF UPWARD FACING SURFACES - BELOW REINFORCING STEEL < 8 INCHES.

3. WHERE EPOXY INJECTION CRACK REPAIR IS SPECIFIED THE WORK SHALL INCLUDE PRESSURE INJECTION OF CRACKS 1/8" OR MORE IN WIDTH. WORK SHALL BE PAID UNDER PAY ITEM 518.40, "EPOXY INJECTION CRACK REPAIR".

4. EXISTING ABUTMENT CONSTRUCTION JOINTS SHALL BE CLEANED AND SEALED WITH BACKER ROD AND A HIGH-PERFORMANCE POLYURETHANE SEALANT FOLLOWING COMPLETION OF THE ABUTMENT REPAIRS. THIS WORK WILL NOT BE PAID FOR DIRECTLY BUT SHALL BE INCIDENTAL TO THE RELATED CONTRACT ITEMS.

5. FOR APPROXIMATE LOCATIONS OF ABUTMENT REPAIRS, SEE SHEETS S-02 TO S-03. ACTUAL REPAIR AREAS WILL BE DETERMINED BY THE RESIDENT DURING CONSTRUCTION.

6. ALL EXCAVATION REQUIRED TO COMPLETE THE ABUTMENT REPAIRS WILL NOT BE MEASURED FOR PAYMENT SEPARATELY, BUT SHALL BE INCIDENTAL TO PAY ITEM 518.60, "REPAIR OF VERTICAL SURFACES <8 INCHES".

7. THE ESTIMATED REPAIR QUANTITIES ARE BASED ON AN INSPECTION COMPLETED IN MAY 2016.

#### PIER NOTES:

I. PIER REPAIR WORK SHALL INCLUDE: PROVIDING ACCESS FOR PIER INSPECTION, PIER SURFACE REPAIRS AND EPOXY INJECTION CRACK REPAIR.

2. WHERE PIER SURFACE REPAIRS ARE SPECIFIED THE WORK SHALL INCLUDE REMOVAL OF UNSOUND CONCRETE AND PLACEMENT AND CURING OF REPAIR MATERIALS. REPAIRS SHALL BE MEASURED FOR PAYMENT UNDER PAY ITEM 518.60, "REPAIR OF VERTICAL SURFACES <8 INCHES", PAY ITEM 518.60, "REPAIR OF VERTICAL SURFACES <8 INCHES", PAY ITEM 518.60, "REPAIR OF VERTICAL SURFACES <8 INCHES", PAY ITEM 518.70, "REPAIR OF OVERHEAD SURFACES <8 INCHES".

3. WHERE EPOXY INJECTION CRACK REPAIR IS SPECIFIED THE WORK SHALL INCLUDE PRESSURE INJECTION OF CRACKS 1/8" OR MORE IN WIDTH. WORK SHALL BE PAID UNDER PAY ITEM 518.40, "EPOXY INJECTION CRACK REPAIR" AND 518.401, "EPOXY INJECTION CRACK REPAIR - BELOW WATERLINE".

4. THE CONTRACTOR SHALL PROVIDE AN ACCEPTABLE MEANS OF ACCESS AT ALL PIER LOCATIONS THAT WILL ALLOW THE RESIDENT TO SAFELY PERFORM A DETAILED CONCRETE INSPECTION OF ALL PIER SURFACES LOCATED ABOVE THE WATERLINE. PROVIDING ACCESS AT PIER LOCATIONS SHALL BE COMPLETED IN ACCORDANCE WITH SPECIAL PROVISION 524, "TEMPORARY ACCESS PLATFORMS FOR PIER INSPECTION AND REPAIR".

5. THE QUANTITIES OF PIER REPAIR NOTED ON THESE DRAWINGS ARE BASED ON BOTH AN UNDERWATER INSPECTION AND A VISUAL INSPECTION. BELOW WATER REPAIRS ARE BASED ON THE ORDINARY WATER LEVEL AT THE TIME OF INSPECTION. IF DIFFERENCES ARE EXPERIENCED THE RESIDENT SHOULD BE NOTIFIED AND QUANTITIES CAN BE ADJUSTED ACCORDINGLY.

6. FOR APPROXIMATE LOCATIONS OF REPAIR SEE SHEETS S-04 TO S-08. ACTUAL REPAIR AREAS WILL BE DETERMINED BY THE RESIDENT DURING CONSTRUCTION.

#### ABUTMENT AND PIER CONCRETE REPAIR PROCEDURE:

I. PRIOR TO THE START OF CONCRETE REPAIRS THE RESIDENT AND THE CONTRACTOR SHALL SOUND ALL ABUTMENT AND PIER SURFACES AND AGREE ON THE REPAIR LIMITS.

2. PERFORM " DEEP SAW CUTS ALONG LIMITS OF REMOVAL.

3. CHIP CONCRETE TO THE DEPTH SPECIFIED IN SPECIAL PROVISION 518. IF THE REMOVAL LIMITS CHANGE DURING THE DEMOLITION PROCESS THE CONTRACTOR SHALL NOTIFY THE RESIDENT. THE RESIDENT AND CONTRACTOR SHALL AGREE ON THE REVISED PAY LIMITS PRIOR TO THE CONTRACTOR CONTINUING THE REMOVALS.

4. PREPARE AND PATCH REPAIR AREAS. SEE SPECIFICATIONS FOR SURFACE PREPARATION, MATERIALS, PLACEMENT AND CURING REQUIREMENTS.

5. PERFORM GENERAL FINISHING.

#### EPOXY INJECTION CRACK REPAIR:

I. ALL CRACK REPAIRS SHALL BE COMPLETED IN ACCORDANCE WITH SPECIAL PROVISION 518.

2. THE SELECTED REPAIR MATERIAL FOR UNDERWATER CRACK REPAIR SHALL BE APPROVED BY THE MANUFACTURER FOR PLACEMENT UNDERWATER.

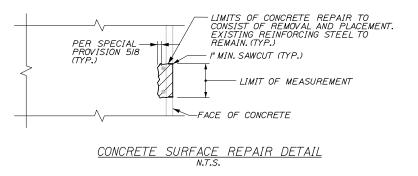
3. PREPARE CONCRETE SURFACE AND CRACK AREA BY CLEANING SUBSTRATE WITH WIRE BRUSH TO REMOVE LAITANCE AND CONTAMINANTS. BLAST CLEAN THE CRACK AND CONCRETE SURFACE; DO NOT USE WATER.

4. SET INJECTION PORTS ALONG THE LENGTH OF THE CRACK USING A HIGH-MODULUS, LOW VISCOSITY EPOXY SUCH AS SIKADUR 35, SIKADUR 55, OR APPROVED EQUAL ONCE SET SEAL PORTS AND CRACK WITH THE SAME EPOXY ADHESIVE. EPOXY SEAL AROUND PORTS SHALL BE ALLOWED TO CURE PRIOR TO CRACK INJECTION.

### GENERAL FINISHING:

I. CONTRACTOR SHALL REMOVE TECTYL COATING AND GRAFFITI WHERE PRESENT PRIOR TO APPLYING PROTECTIVE CONCRETE COATING.WORK SHALL BE INCIDENTAL TO THE SPECIFIED ITEM IN SPECIFICATION 518.

2. ALL EXPOSED SUBSTRUCTURE SURFACES SHALL BE COATED WITH A PROTECTIVE COATING SUITABLE FOR CONCRETE SURFACES AFTER PATCHING IS COMPLETE AND PATCH MATERIALS HAVE CURED IN ACCORDANCE WITH SPECIAL PROVISION 515. ALL EXPOSED SURFACES FACING ROADWAY ON ABUTMENT I AND PIER I SHALL BE COATED WITH A PIGMENTED ANTI-GRAFFITI COATING.



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ostructure	Scale:		Designed by: HNTB	HNTB CORPORATION	TURNPIKE THE GOLD STAR
04_Sut	No. Revision B	By Date		340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155	
ĕ[			CONSULTANT PROJECT MANAGER: Craig Morin, P.E.	FAX (207) 228-0909	
Ĕ			By Date By Date		
Suc			Designed NMW 07\18 Checked BRG 07\18		
Ē.			Drawn PEB 07\18 In Charge of RAL 07\18		MTA PROJECT MANAGER: Kristi Van Ooyen, P.E.

CONTRACT:2018.15

104 OF 115

BRIDGE REPAIRS ANDROSCOGGIN RIVER OVERPASS SUBSTRUCTURE REPAIRS CONCRETE REPAIR NOTES AND DETAILS SHEET NUMBER: AR-13

REPAIR OF VERTICAL SURFACES < 8 INCHES 29 S.F.* REPAIR OF UPWARD FACING SURFACES 5 S.F.*

- BELOW REINFORCING STEEL < 8 INCHES

EPOXY INJECTION CRACK REPAIR

* INCLUDES 5 S.F. ADDITIONAL REPAIR QUANTITY AS A CONTINGENCY.

5 L.F.**

** ASSUMES 5 L.F. OF ADDITIONAL CRACK REPAIR QUANTITY AS A CONTINGENCY.

### <u>LEGEND</u>

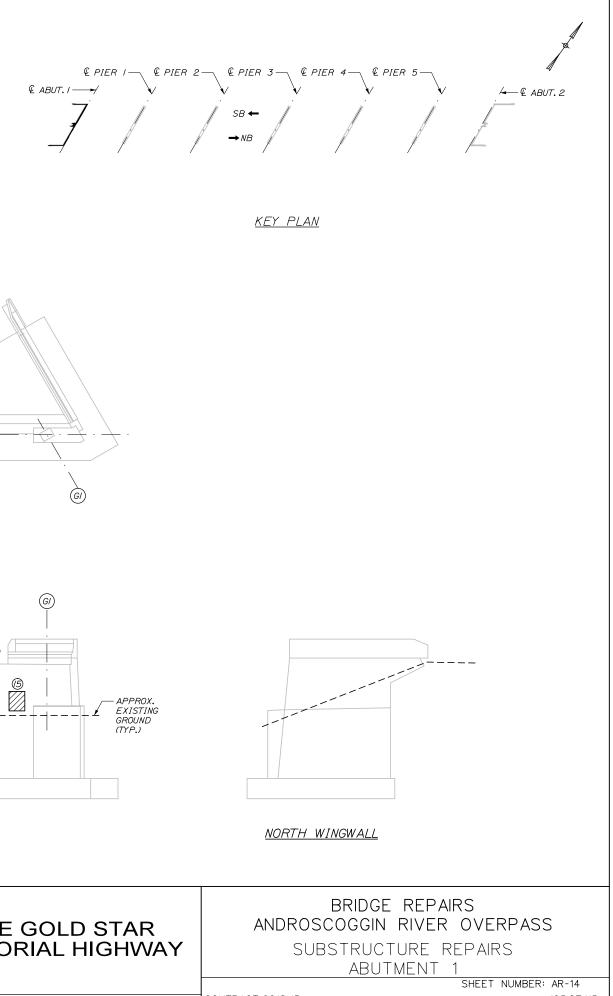
- LIMIT OF SURFACE PATCH REPAIR
- (#) SQUARE FOOT AREA OF REPAIR #

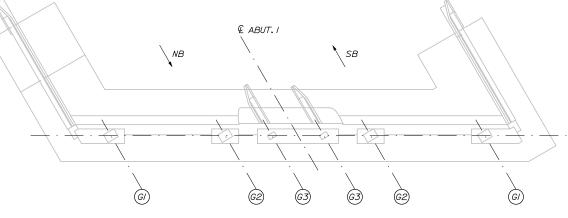
LINEAR FOOT LENGTH OF CRACK

----- EPOXY INJECTION CRACK REPAIR

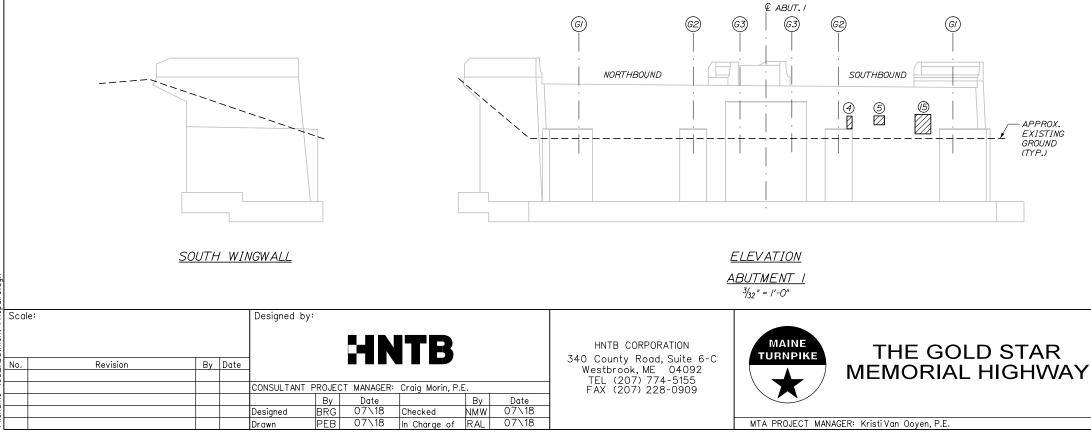
NOTES:

I. FOR CONCRETE REPAIR DETAIL AND PROCEDURAL NOTES SEE SHEET S-01.





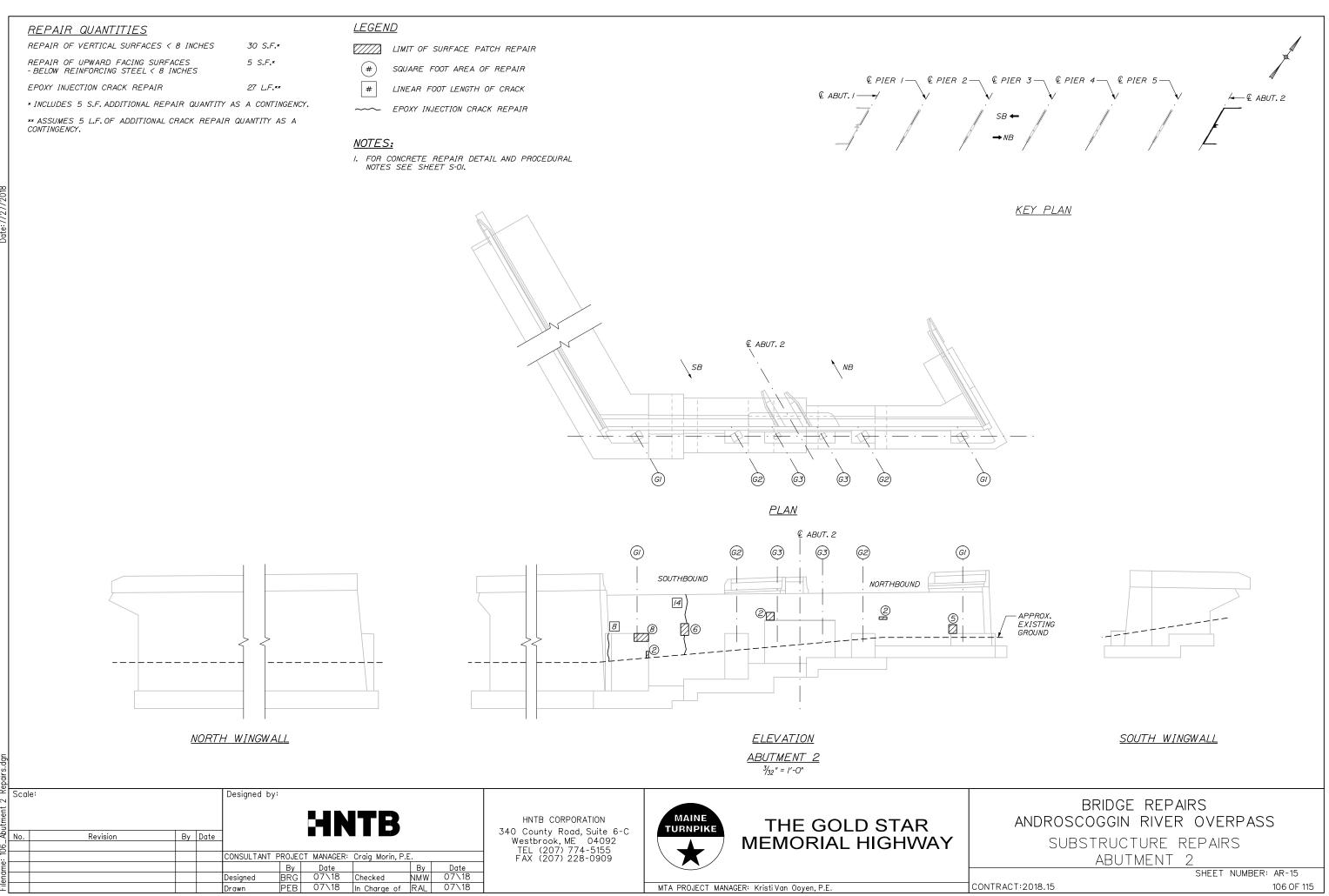
<u>PLAN</u>



CONTRACT:2018.15

#### REPAIR QUANTITIES REPAIR OF VERTICAL SURFACES < 8 INCHES 30 S.F.* LIMIT OF SURFACE PATCH REPAIR REPAIR OF UPWARD FACING SURFACES 5 S.F.* (#) SQUARE FOOT AREA OF REPAIR - BELOW REINFORCING STEEL < 8 INCHES # EPOXY INJECTION CRACK REPAIR 27 L.F.** LINEAR FOOT LENGTH OF CRACK € ABUT. I — → * INCLUDES 5 S.F. ADDITIONAL REPAIR QUANTITY AS A CONTINGENCY. ----- EPOXY INJECTION CRACK REPAIR ** ASSUMES 5 L.F. OF ADDITIONAL CRACK REPAIR QUANTITY AS A

Drawn



CONTRACT:2018.15

REPAIR OF VERTICAL SURFACES < 8 INCHES 438 S.F.* REPAIR OF UPWARD FACING SURFACES 5 S.F.** - BELOW REINFORCING STEEL < 8 INCHES

REPAIR OF OVERHEAD SURFACES < 8 INCHES 2| S.F.** EPOXY INJECTION CRACK REPAIR 23 L.F.***

* INCLUDES 50 S.F. ADDITIONAL REPAIR QUANTITY AS A CONTINGENCY.

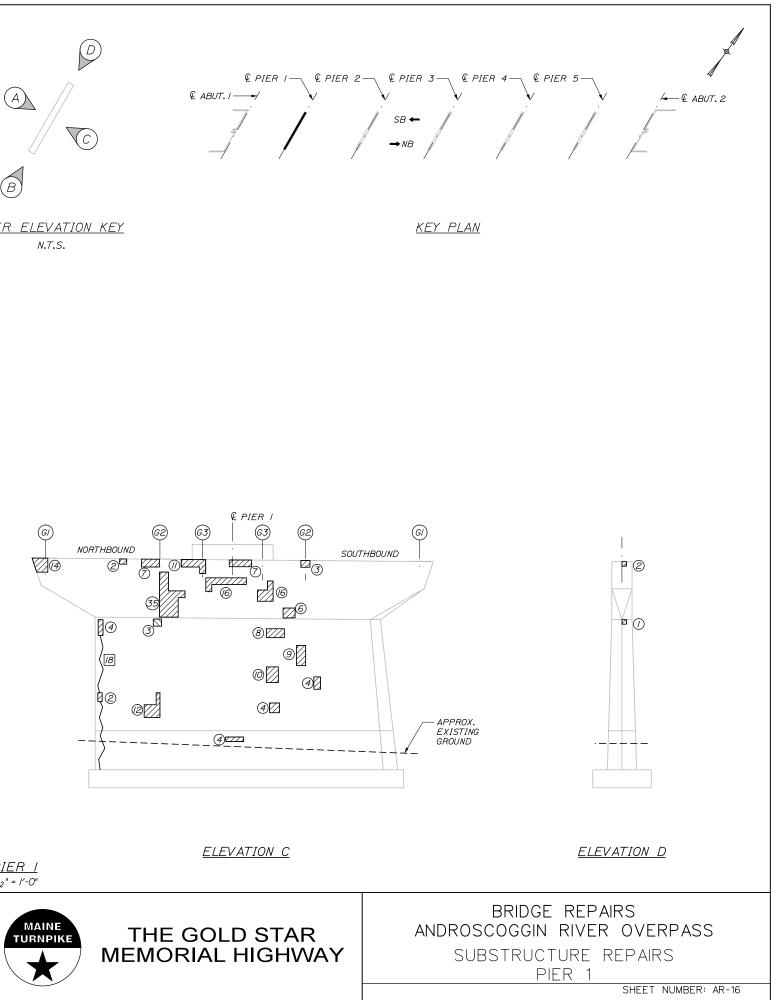
** INCLUDES 5 S.F. ADDITIONAL REPAIR QUANTITY AS A CONTINGENCY.

*** ASSUMES 5 L.F. OF ADDITIONAL CRACK REPAIR QUANTITY AS A CONTINGENCY.

- <u>LEGEND</u>
- LIMIT OF SURFACE PATCH REPAIR
- (#) SQUARE FOOT AREA OF REPAIR
- # LINEAR FOOT LENGTH OF CRACK
- ----- EPOXY INJECTION CRACK REPAIR

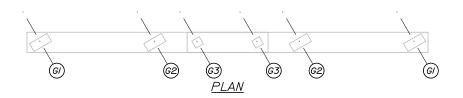
### NOTES:

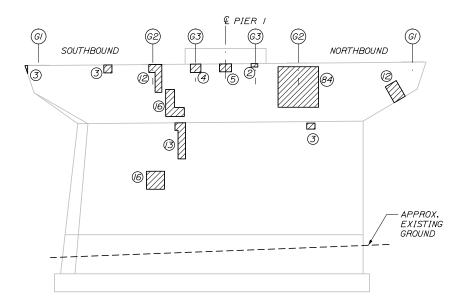
I. FOR CONCRETE REPAIR DETAIL AND PROCEDURAL NOTES SEE SHEET S-01.

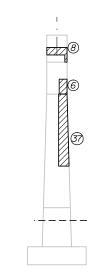


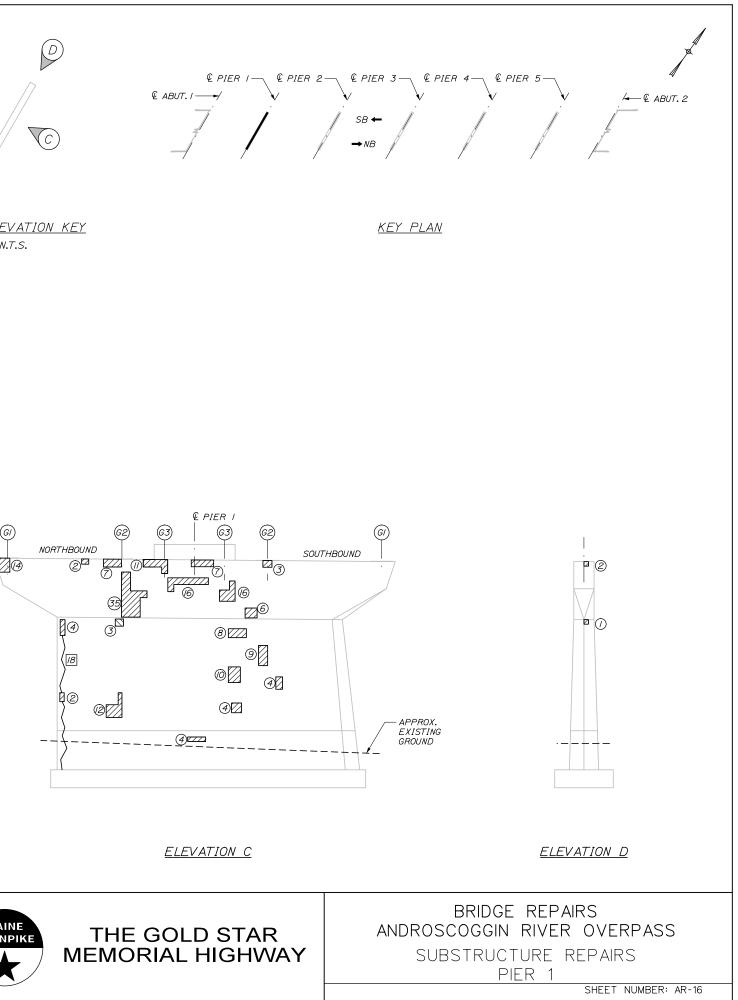
PIER ELEVATION KEY N.T.S.

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107_Pier 1 Repairs.	Scale:	Revision	By	Date	Designed by:			TB			HNTB CORPORATION 340 County Road, Suite 6-C Westbrook, ME 04092 TEL (207) 774-5155		THE GOLD STAR MEMORIAL HIGHWAY	
le:					CONSULTANT PF			Craig Morin, P		Data	FAX (207) 228-0909			
enarr					~	BRG 07		Checked	By NMW	Date 07\18				
Ē					Drawn F	PEB 07	∖18  ı	In Charge of	RAL	07\18		MTA PROJECT MAN	IAGER: Kristi Van Ooyen, P.E.	

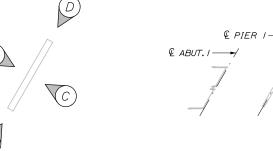
CONTRACT:2018.15

<u>ABOVE WATER</u>	
REPAIR OF VERTICAL SURFACES < 8 INCHES	580 S.F.*
REPAIR OF UPWARD FACING SURFACES -BELOW REINFORCING STEEL < 8 INCHES	5 S.F.**
REPAIR OF OVERHEAD SURFACES < 8 INCHES	13 S.F.**
EPOXY INJECTION CRACK REPAIR	41 L.F.***
<u>BELOW_WATER</u> REPAIR OF VERTICAL SURFACES < 8 INCHES - BELOW WATERLINE	7 S.F.**
EPOXY INJECTION CRACK REPAIR -	

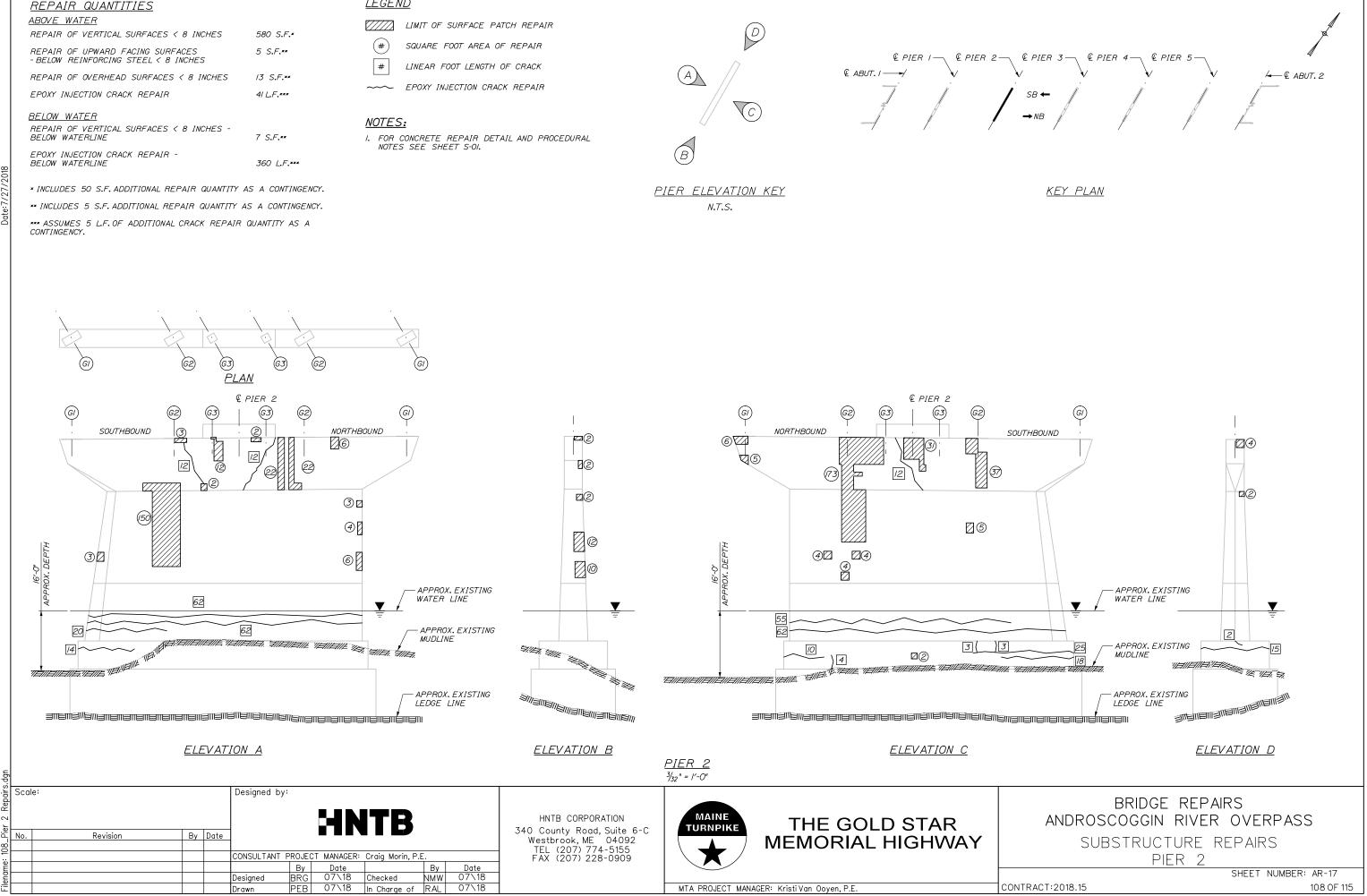


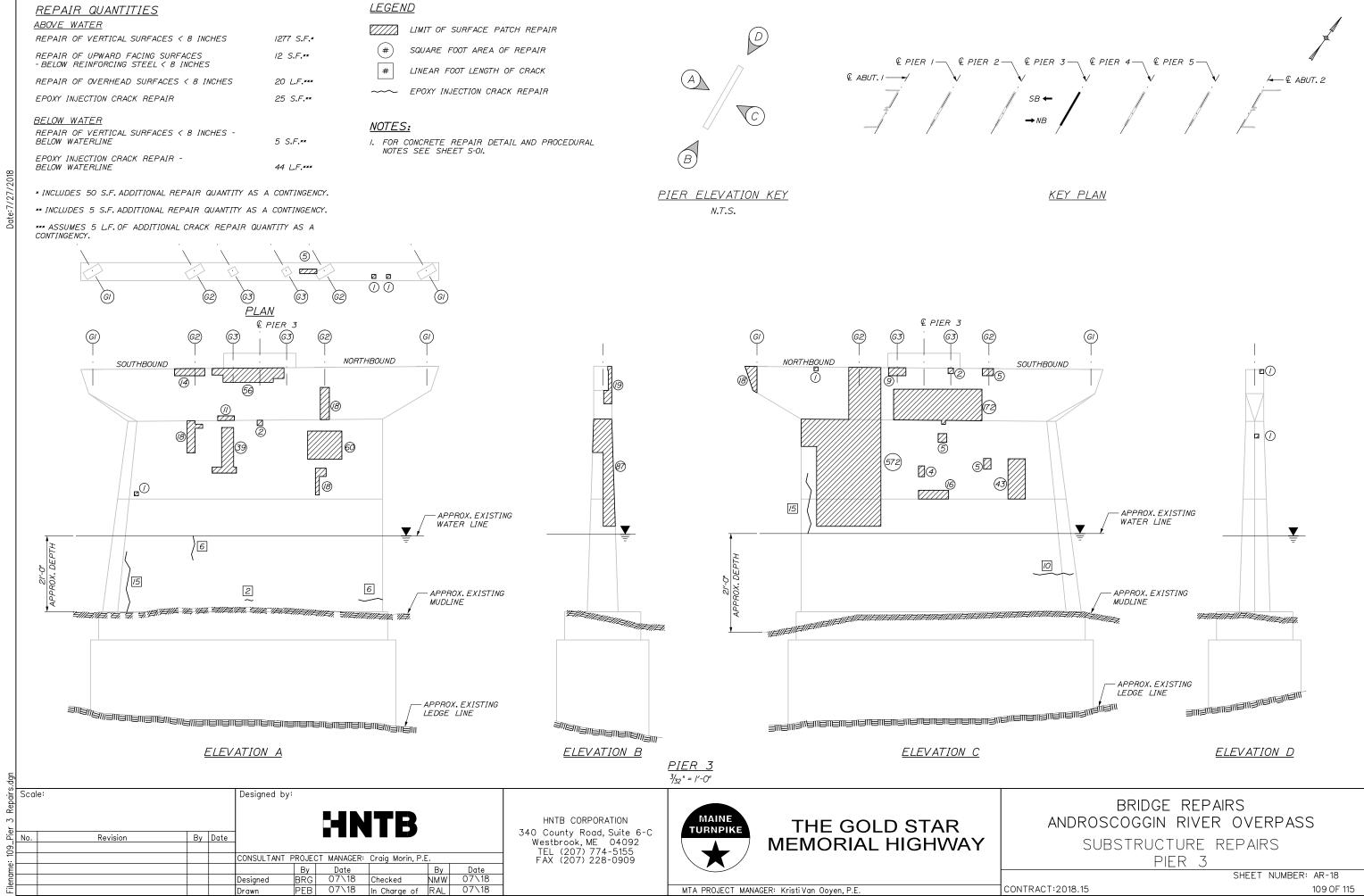
- EPOXY INJECTION CRACK REPAIR

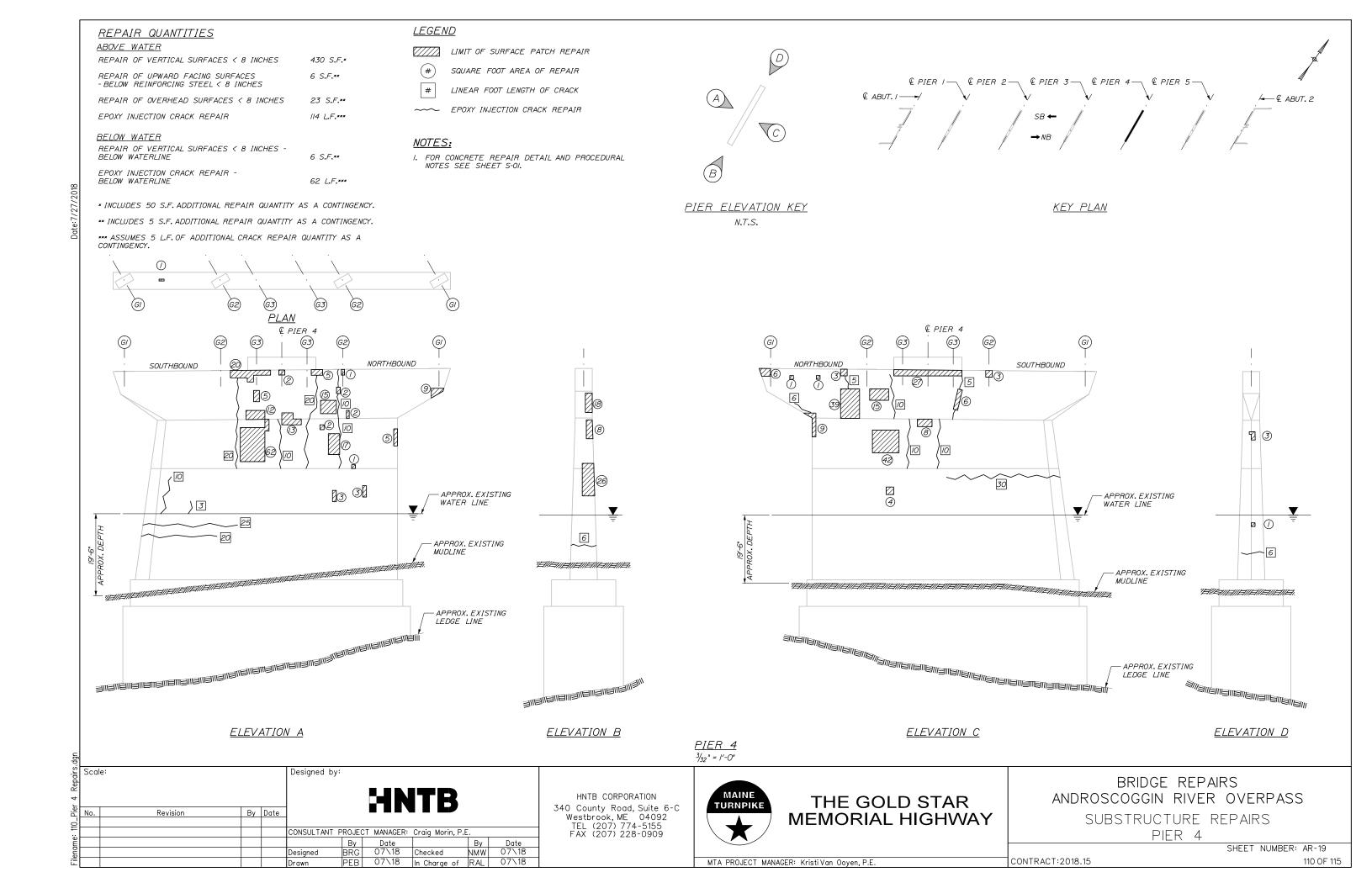
NOTES SEE SHEET S-OI.



N.T.S.







REPAIR OF VERTICAL SURFACES < 8 INCHES 163 S.F.* REPAIR OF UPWARD FACING SURFACES 5 S.F.**

- BELOW REINFORCING STEEL < 8 INCHES REPAIR OF OVERHEAD SURFACES < 8"

EPOXY INJECTION CRACK REPAIR

* INCLUDES 15 S.F. ADDITIONAL REPAIR QUANTITY AS A CONTINGENCY.

14 S.F.**

69 L.F.***

** INCLUDES 5 S.F. ADDITIONAL REPAIR QUANTITY AS A CONTINGENCY.

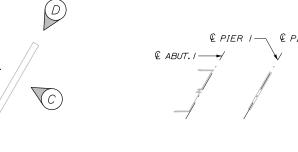
*** ASSUMES 5 L.F. OF ADDITIONAL CRACK REPAIR QUANTITY AS A CONTINGENCY.



- LIMIT OF SURFACE PATCH REPAIR
- (#) SQUARE FOOT AREA OF REPAIR
  - # LINEAR FOOT LENGTH OF CRACK
- EPOXY INJECTION CRACK REPAIR  $\sim\sim$

### NOTES:

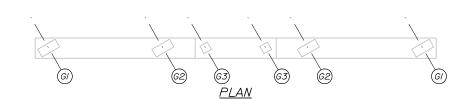
I. FOR CONCRETE REPAIR DETAIL AND PROCEDURAL NOTES SEE SHEET S-OI.

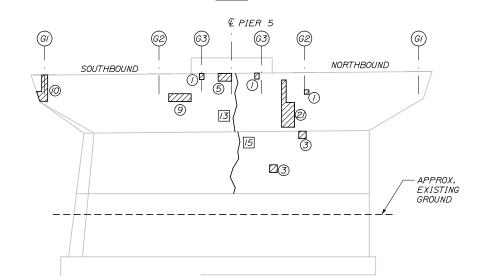


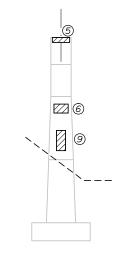


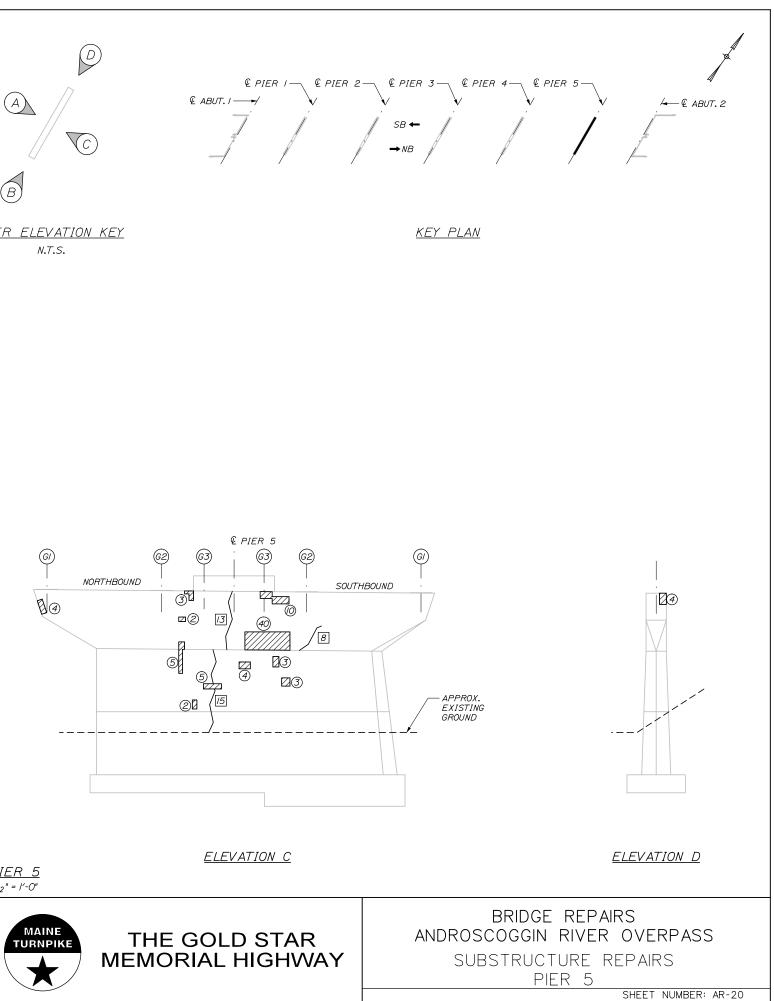
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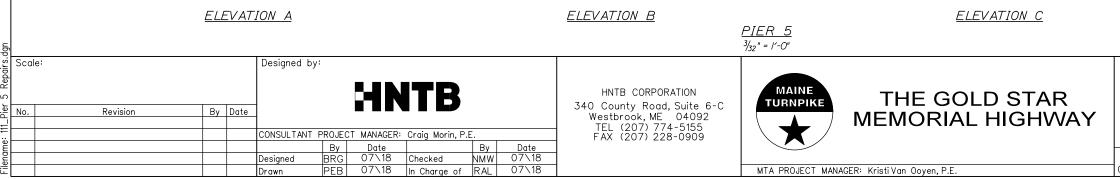
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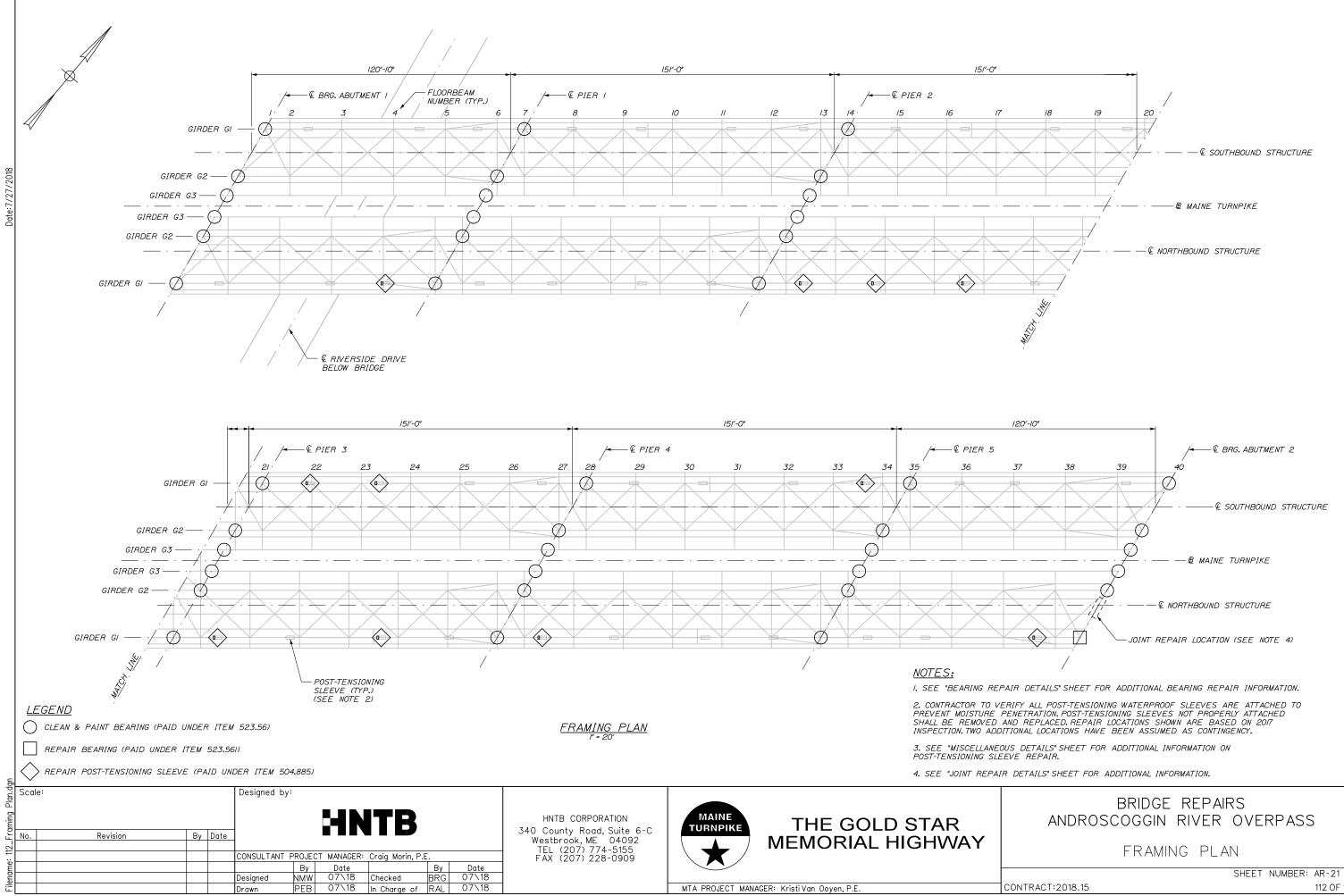




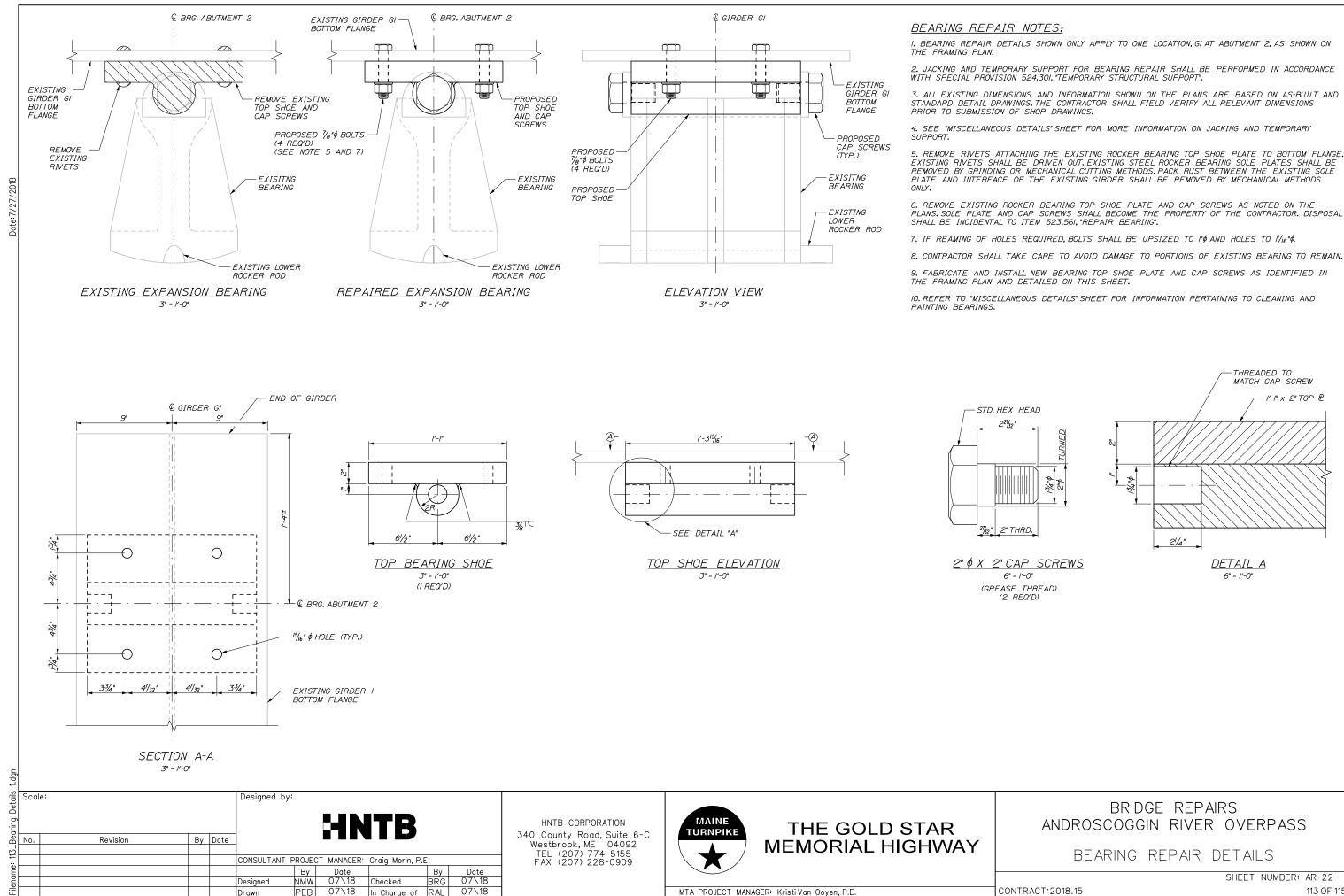


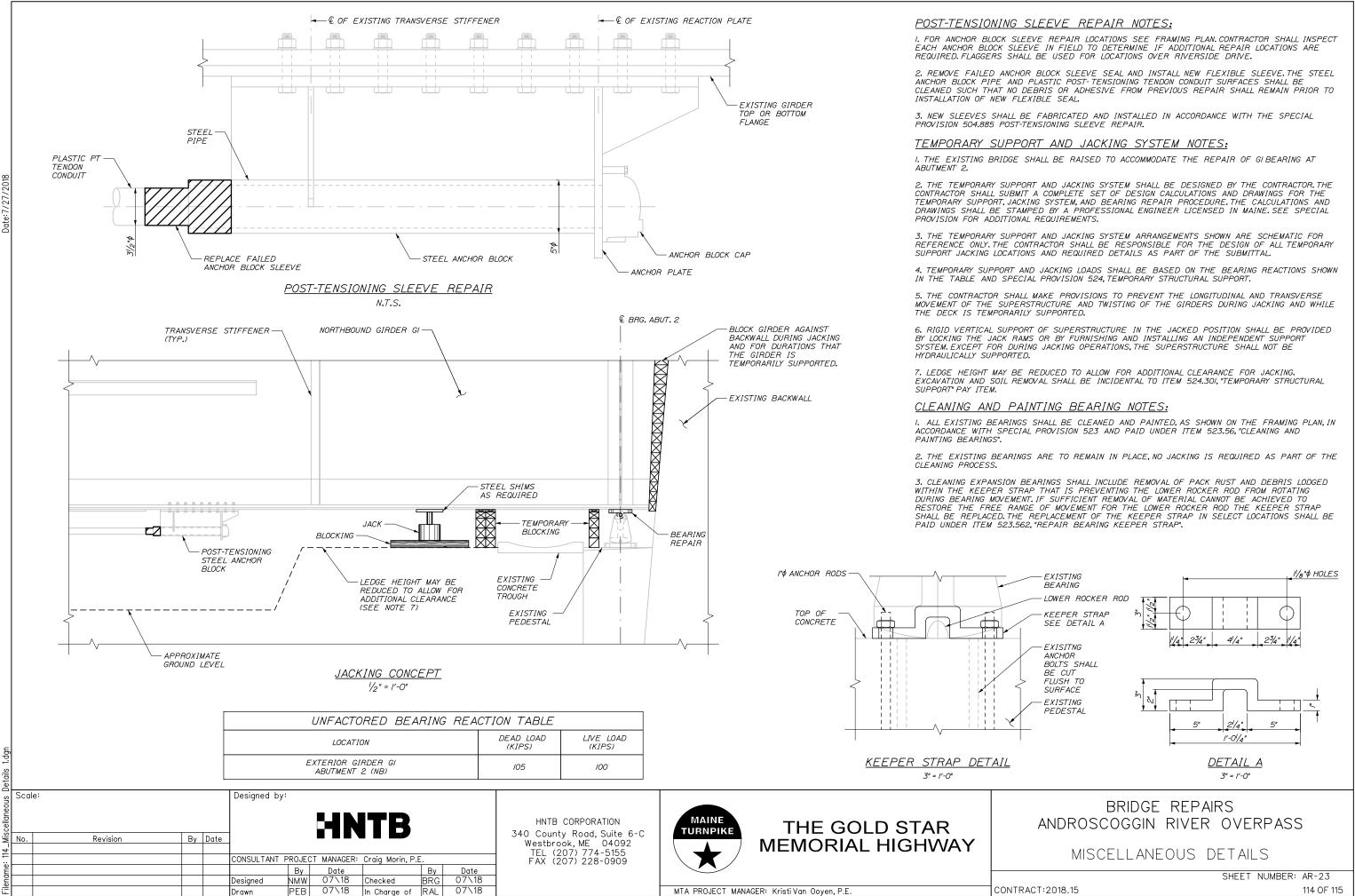


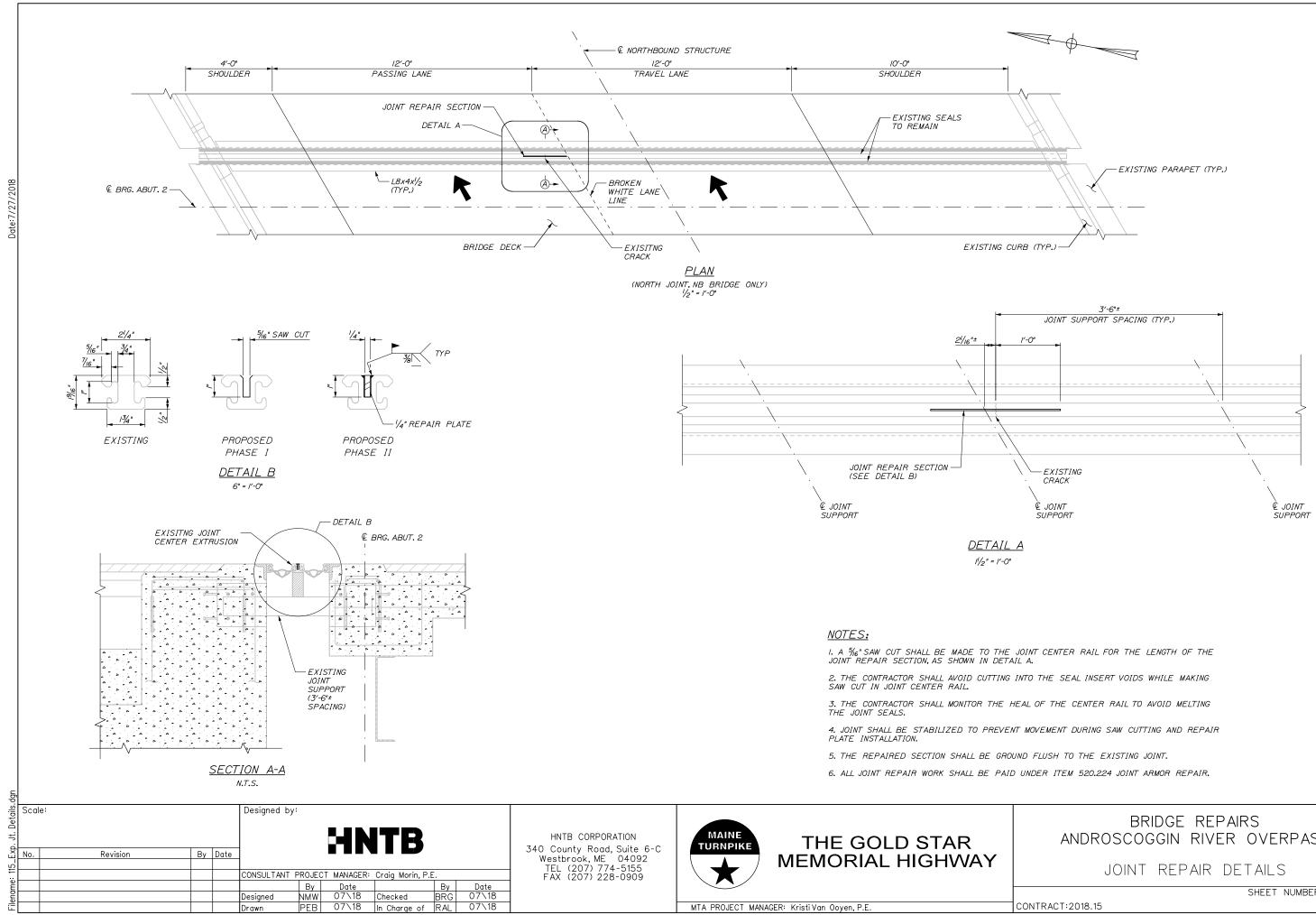
CONTRACT:2018.15



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SHEET NUMBER: AR-24 115 OF 115

ANDROSCOGGIN RIVER OVERPASS